General Notes

- 1. Specifications, codes, and standards
 - a. ASCE 37-14 Design Loads on Structures during Construction
 - b. ASCE 7-16 Minimum Design Loads and Associated Criteria for Buildings and Other Structures
 - c. ANSI/AWC NDS National Design Specification for Wood Construction ASD/LRFD
- d. Cal/OSHA Title Regulations Sub-chapter 4, Cl. 1717

2. Reference Documents

- a. NDS 2018
- b. Structural Drawings (Plan Check Response 2 Drawing Set, Dated 2020-05-08)
- c. MTC Structural Screw Design Guide (US), 2020
- d. ASCE 37-14 Design Loads on Structures during Construction
- e. ASCE SEI 7-16 Minimum Loads and Associated Criteria for Buildings and Other Structures

3. Design Criteria

- a. Site Location: 1265 Borregas Ave, Sunnyvale, CA
- b. Risk Category: II (ASCE 7-16)

4. Design Loads

Design conforms to the more stringent of ASCE 37-14 and Cal/OSHA - Title Regulations Sub-chapter 4, Cl. 1717 design loads.

- a. Dead Loads
- i. CLT wall & floor panels = 26pcf
- ii. Glulam = 30pcf
- b. Live Loads (ASCE 37-14, Table 4-4)
- i. Very Light Duty = 20pstii. Light Duty = 25psf
- ii. Medium Duty = 50psf
- * If these loads are to be locally exceeded due to material staging or other construction sequencing requirements, shoring design is to be provided by others.
- c. Wind Load
- i. Design as per Cal/OSHA Title Regulations Sub-chapter 4, Cl. 1717 design loads The lateral loads to be resisted at each floor in both directions were taken as the maximum of 100lbf/ft of floor edge or 2% of the total dead load.
- ii. Work from structures shall be discontinued when adverse weather such as high winds makes the work unsafe
- iii. The temporary lateral support of the structure designed by ASPECT is governed by the stability requirements of ASCE 37-14 Cl. 4.4. The temporary lateral support of the structure is not designed to sustain wind loads when the façade is installed. Ensure the structural system is complete before installing the façade (ie. concrete diaphragms are cast and EOR confirms that façade system can be installed).

d. Seismic Loads

i. Not considered per Cal/OSHA - Title Regulations Sub-chapter 4, Cl. 1717 design loads

- e. Horizontal Construction Loads (max. structural effects of i to iv.) (ASCE 37-14 Cl. 4.4)
- i. For wheeled vehicles transporting materials, 20% for a single vehicle or 10% for two or more vehicles of fully loaded vehicle weight. Said force shall be applied in any direction of possible travel, at the running surface,
- ii. For equipment reactions as described in ASCE 37-14 Section 4.6, the calculated or rated horizontal load, whichever is greater.
- iii. 50lb/person applied at the level of the platform in any direction
- iv. 2% of total vertical load.

5. Materials

- a. Glulam
- i. Single Span = 24f-V4 DF/DF
- ii. Multi Span = 24f-V8 DF/DF iii. Compression Members – L2D DF/DF
- b. CLT min V2M1.1 or V2M2.1
- c. Sawn Timber SPF No.1/No.2
- d. ASTM A325 Through bolts
- e. ASTM F1554, Grade 36 Anchor Bolts
- f. Wood Screws according to details

6. Construction Notes

- a. All concrete substructures to have minimum F'c=3000psi prior to starting erection of wood elements
- b. Erection details for all structural steel items are by others
- c. All anchor bolts have been surveyed and are within acceptable tolerance d. All base plates to be grouted prior to starting erection of wood elements
- e. Plumbing bracing details by others.
- f. All column and panel bases are to be attached and temporary braces installed prior to releasing columns and panels from crane.

7. Scope Limitation and Scope of Field Review

- a. Specification of crane type, boom configuration, hook block and rigging, counterweights, outrigger arrangement and associated loads, bearing pads, and all aspects related to the capacity or suitability of the crane are by others
- b. Aspect Structural Engineers provides field reviews for the structure as shown on this drawing set. These reviews are not a "continuous" review but as a general review to ascertain that the structure has been erected in substantial conformity to the erection drawings and supporting documents.
- c. The field reviews by Aspect Structural Engineers LP is specifically for the review of the completed structure and does not imply conformance with all the regulations of Cal/OSHA. Aspect Structural Engineers LP will not be responsible for construction safety other than structural stability during erection.
- d. Erection review by Aspect Structural Engineers LP is not carried out for the contractor's benefit, nor does it make Aspect guarantors of the contractor's work. It remains the contractor's responsibility to build the structure in conformance with the erection drawings and supporting documents.

8. Field Review Requirements

a. The erected timber structure must be inspected and approved in writing by a professional engineer from Aspect Structural Engineers LP prior to allowing workers access. Aspect Structural Engineers LP may choose to undertake these reviews by photo, video call, or by engagement of a local professional.

9. Site Conditions

- a. These erection drawings have been detailed by Aspect Structural Engineers from the latest information possible. It is the responsibility of the contractor to check and verify all dimensions, elevations, etc. prior to starting construction.
- b. Aspect Structural Engineers LP shall be immediately notified of any discrepancies or inconsistencies between these drawings and actual site conditions. The Contractor must notify Aspect Structural Engineers LP of any layout modification at least 72 hours prior to field review to allow time to revise drawings where necessary.

DRAWING LIST				
DWG SERIES	DRAWING TITLE			
K000s	General Notes, Temporary Bracing Details,			
	and Lifting Plans			
K100s	Level 2 - installation			
K200s	Level 2 - conversion to medium deck			
K300s	Level 3 - installation			
K400s	Level 3 - conversion to medium deck and			
	Level 4 installation			
K500s	Level 4 - conversion to medium deck			
K600s	Level 5 - installation			
K700s	Level 5 - conversion to medium deck and Roof installation			
K800s	Level 2 - concrete pour and temporary brace removal			
K900s	Level 3 & 4 - concrete pour and temporary brace removal			
K1000s	Level 5 & roof - concrete pour and temporary brace removal			
K1100s	Glulam Beam Camber Approach			

Erection Sequence

Erection Step	Reference Drawing	Procedure	
E1	K100 series	E1.1: Place anchors/embeds in slab	
		E1.2: Install steel frame. Brace steel per installer. E1.3: Install timber columns per sequencing drawings. Brace columns per 15' timber	
		brace detail. E1.4: Block out gap between glulam columns & steel brace frame columns. Wrap truck	
		straps around columns per drawings. E1.5: Install level 2 deck. Install all splines and CLT deck connections per structural	
		drawings and temporary strapping plan on level 2. * Note: Level 2 is now considered a "Very Light Duty" deck. No scissor lifts or material staging are permitted on this deck.	
E2	K200 series	E2.1: Replace level 1 timber temporary bracing with cable bracing per typical detail. * Note: Level 2 deck can now be classified as "Medium Duty". Therefore, scissor lifts can now be driven on this deck and material can be staged.	
E3	K300 series	E3.1: Install 15', 30', and 60' Glulam columns on level 2 per sequencing drawings. Brace columns per 15' Timber bracing & 30' cable bracing typical details. Brace 60' column per 30' cable brace detail.	
		E3.2: Block out gap between glulam columns & steel brace frame columns. Wrap truck straps around columns per drawings.	
		E3.2: Install level 3 deck along with all required strapping per structural drawings. * Note: Level 3 is now considered a "Very Light Duty" deck. No scissor lifts or material staging are permitted on this deck.	
E4	K400 series	E4.1: Replace level 2 timber temporary bracing with cable bracing per typical detail. * Note: Level 2 permanent steel braces are now fully engaged and Level 3 deck can now be classified as "Medium Duty", therefore, scissor lifts can be driven on this deck	
		and material can be staged. E4.2: Install 15' columns on level 3 per sequencing drawings. Brace columns per 15'	
		timber bracing typical detail. E4.3: Install Level 4 deck along with all required strapping per structural drawings and	
		temporary strapping plan. * Note: Level 4 is now a "Very Light Duty" deck, therefore no scissor lifts or material	
		staging are permitted on this deck. E4.4: Once Level 4 deck installed, replace 30' column cable bracing with cable cross	
		bracing per typical detail. Ensure all reinforcing screws in column to column connection, per structural drawing S401, installed prior to installation of temporary cable cross bracing.	
E5	K500 series	E5.1: Block out gap between glulam columns & steel brace frame columns on level 3. Wrap truck straps around columns per drawings.	
		E5.2: Remove timber bracing on level 3. * Note: Level 3 permanent steel braces are now fully engaged and Level 4 deck can	
		now be classified as "Medium Duty", therefore, scissor lifts can now be driven on this deck and material can be staged.	
E6	K600 series	E6.1: Install 15' and 30' Glulam columns on level 4 per sequencing drawings. Brace columns per 15' timber bracing & 30' cable bracing details, respectively. Ensure all reinforcing screws in column to column connection, per structural drawing	
		S401 installed prior to installation of 30' temporary cable cross bracing. E6.2: Install Level 5 deck along with all required strapping per structural drawings. * Note: Level 5 is now considered a "Very Light Duty" deck. No scissor lifts or material staging are permitted on this deck.	
E7	K700 series	E7.1: Block out gap between glulam columns & steel brace frame columns on level 4. Wrap truck straps around columns. Remove timber bracing on level 4.	
		* Note: Level 4 permanent steel braces are now fully engaged and level 5 deck can now be classified as "Medium Duty", therefore, scissor lifts can now be driven on dec	
		 and material can be staged. E7.2: Install 15' columns on level 5 per sequencing drawings. Brace columns per 15' timber bracing typical detail. 	
		E7.3: Install roof deck along with all required strapping per structural drawings. * Note: The roof is now a "Light Duty" deck, therefore no scissor lifts or material staging are permitted on this deck.	
		E7.4: Once roof deck installed, replace 30' column cable bracing with cable cross bracing per typical detail. Ensure all reinforcing screws in column to deck connection, per structural drawing S401, installed prior to installation of	
E8	K800 series	temporary cable cross bracing. E8.1: Block out gap between glulam columns & steel brace frame columns on level 5.	
	1,000 00,100	Wrap truck straps around columns. E8.2: Replace level 5 timber temporary bracing with cable bracing per typical detail.	
		* Note: Scissor lifts or material staging are not permitted on the roof deck at any time. E8.3: Pour concrete topping on level 2 per structural drawings prior to removing temporary cable cross bracing on level 1.	
E9	K900 series	temporary cable cross bracing on level 1. E9.1: Pour concrete topping on level 3 and 4 per structural drawings prior to removing temporary bracing on level 2.	
E10	K1000 series	E10.1: Pour concrete topping on level 5 and roof per structural drawings prior to removing temporary bracing on level 4.	

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his drawing is not to be used for construction purposes until noted and date superformation. All measurements must be checked on site and be eriffied by the Contractor. Do not scale off hard copy drawings or any lectronic/computer files. Written dimensions always have precedent. Hard opy drawings are the official documents for the project and always take

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 Date:
 Revision / Issue:
 1

 2020-09-22 Issued for Coordination

 2020-09-29 Issued for Coordination

 2020-10-06 Issued for Coordination

 2020-10-19 Issued for Construction

 2020-12-18 Issued for Construction
 1

 2021-02-09 Issued for Construction
 2

Plot Date:

2020-10-16

General Notes and Erection Sequence

Scale: Drawn:
Designec

NTS Checked

CLT LIFT GENERAL NOTES

1.0 CENTRE OF GRAVITY

C.O.G MAY VARY FROM PANEL TO PANEL AND SHALL BE CONFIRMED BY THE CONTRACTOR

2.0 FASTENER LOADING:

- 2.1 FASTENER DESIGN IS IN ACCORDANCE WITH NDS 2018
- 2.2 PANEL WEIGHTS ARE BASED ON SHOP DRAWINGS AND ARE TO BE CONFIRMED BY
- MAXIMUM WEIGHT OF PANEL ASSUMED TO BE PER PANEL LIFT SCHEDULE. CONTRACTOR TO NOTIFY ASPECT STRUCTURAL ENGINEERS IMMEDIATELY IF PANEL WEIGHT IS TO BE EXCEEDED.
- TOTAL FACTORED LOAD [W.] FOR LIFTING HAS BEEN DETERMINED AS FOLLOWS (PER MTC RIGGING DESIGN GUIDE, PUBLISHED IN 2020):

- = UNFACTORED WEIGHT OF PANEL = PER PANEL LIFT SCHEDULE
- OPTIONAL SAFETY FACTOR TO ACCOUNT FOR SITE BASED LIFTING HAZARDS = 1.2

 DYNAMIC ACCELERATION FACTOR = 1.4 [BASED ON STATIONARY MOBILE CRANE]

FASTENERS

- ALL FASTENERS SHALL BE ASSY KOMBI SCREWS PER PANEL LIFT SCHEDULE
- FASTENER CAPACITIES HAVE BEEN DETERMINED FOR USE IN SPF CLT PANELS AS PER EQUILIBRIUM CONSULTING INC STRUCTURAL ENGINEERS "PLAN CHECK RESPONSE 2" DRAWING SET, DATED AUGUST 5, 2020
- ASSY KOMBI FASTENER CAPACITY BASED ON ALLOWABLE STRESS DESIGN PRINCIPALS BASED IN ACCORDANCE WITH NDS 2018
- TO ENSURE SAFEY AND PROPER CAPACITY FASTENERS SHALL ONLY BE USED ONCE, AND DISPOSED OF AFTER EACH PANEL LIFT. THIS SHALL BE CARRIED OUT IN CONJUNCTION WITH REGULAR INSPECTIONS OF RIGGING AND ANCHORING DEVICES TO ENSURE STRUCTURAL 3.4 INTEGRITY OF ALL LIFTING COMPONENTS

4.0 LIFTING DEVICES

- THE FOLLOWING LIFTING DEVICES ARE PERMITTED, THE USE OF ALTERNATIVES SHALL NOT BE ACCEPTED WITHOUT THE EXPRESS WRITTEN PERMISSION OF ASPECT STRUCTURAL ENGINEERS
- 4.1.1 CROSBY HR-1000 HEAVY LIFT SWIVEL HOIST RINGS (WLL = 15000lbf)
 4.1.2 DAYTON SUPERIOR T-26 (WLL = 13500lbf)
- 4.1.3 1 1/4" EYEBOLT (WLL = 15000lbf)

RIGGING DETAILS

- RIGGING DESIGN REMAINS THE RESPONSIBILITY OF RIGGING ENGINEER ANY RIGGING 5.1 INFORMATION SHOWN ON THIS DRAWING IS FOR ILLUSTRATIVE PURPOSES ONLY
- THE RIGGING ARRANGEMENT SHALL BE PROVIDED TO ENSURE AN EVEN LOAD DISTRIBUTION ACROSS ALL LIFTING POINTS.
- SLINGS AND ASSOCIATED RIGGING SHALL BE POSITIONED TO PRODUCE AN ANGLE NO LESS THAN 65 DEGREES FROM THE HORIZONTAL AT ALL LIFTING POINTS.

15ft & 30ft GLULAM COLUMN LIFT GENERAL NOTES

1.0 FASTENER LOADING:

- FASTENER DESIGN IS IN ACCORDANCE WITH NDS 2018
- 1.2 COLUMN WEIGHTS ARE BASED ON SHOP DRAWINGS AND ARE TO BE CONFIRMED BY CONTRACTOR
- 1.3 MAXIMUM WEIGHT OF COLUMN ASSUMED TO BE 3,600 LBS. CONTRACTOR TO NOTIFY ASPECT STRUCTURAL ENGINEERS IMMEDIATELY IF COLUMN WEIGHT IS TO BE EXCEEDED.

 1.4 TOTAL FACTORED LOAD [W.] FOR LIFTING HAS BEEN DETERMINED AS FOLLOWS (PER MTC

$P = p \times K_{os} \times K$

- = UNFACTORED WEIGHT OF BEAM = 3,600lbf
- = OPTIONAL SAFETY FACTOR TO ACCOUNT FOR SITE BASED LIFTING HAZARDS = 1.2
- = DYNAMIC ACCELERATION FACTOR = 1.4 [BASED ON STATIONARY MOBILE CRANE]

- FASTENERS
 ALL FASTENERS SHALL BE ASSY KOMBI SCREWS PER GLULAM COLUMN LIFTING PLATE DETAIL.
- FASTENER CAPACITIES HAVE BEEN DETERMINED FOR USE IN D.FIR GLULAM AS PER 22 EQUILIBRIUM CONSULTING INC STRUCTURAL ENGINEERS BP DRAWING SET, DATED FEBRUARY 26, 2020
- ASSY KOMBI FASTENER CAPACITY BASED ON ALLOWABLE STRESS DESIGN PRINCIPALS BASED IN ACCORDANCE WITH NDS 2018 $\,$
- TO ENSURE SAFEY AND PROPER CAPACITY FASTENERS SHALL ONLY BE USED ONCE, AND 2.4 DISPOSED OF AFTER EACH COLUMN LIFT. THIS SHALL BE CARRIED OUT IN CONJUNCTION WITH REGULAR INSPECTIONS OF RIGGING AND ANCHORING DEVICES TO ENSURE STRUCTURAL INTEGRITY OF ALL LIFTING COMPONENTS

LIFTING DEVICES

- THE FOLLOWING LIFTING DEVICES ARE PERMITTED. THE USE OF ALTERNATIVES SHALL NOT BE ACCEPTED WITHOUT THE EXPRESS WRITTEN PERMISSION OF ASPECT STRUCTURAL ENGINEERS
- CROSBY HR-1000 HEAVY LIFT SWIVEL HOIST RINGS (WLL = 15000lbf)
- DAYTON SUPERIOR T-26 (WLL = 13500lbf
- 3.1.3 1 1/4" EYEBOLT (WLL = 15000lbf)

4.0 RIGGING DETAILS

- RIGGING DESIGN REMAINS THE RESPONSIBILITY OF RIGGING ENGINEER ANY RIGGING INFORMATION SHOWN ON THIS DRAWING IS FOR ILLUSTRATIVE PURPOSES ONLY
- 4.2 THE RIGGING ARRANGEMENT SHALL BE PROVIDED TO ENSURE AN EVEN LOAD DISTRIBUTION

T26 Double Swivel Lifting Plate

- Designed for use with 1", 1-1/4" and 1-1/2" diameter coil bolts
- Rotates 360° in horizontal plane and 180° in vertical plane.
- Round bearing plate provides maximum safety when used with any coil bolt lifting sys
- Heavy forged bail is made of high-strength material. Double swivel action allows bail to rotate in direction of applied load
- Safe Working Load is 9,000 lbs. for 1" diameter Safe Working Load is 13.500 lbs. for 1-1/4" and 1-1/2" diameter
- Type Bolt H D Safe Working Load



To Order:

60ft GLULAM COLUMN LIFT GENERAL NOTES

1.0 CENTRE OF GRAVITY

1.1 C.O.G MAY VARY FROM BEAM TO BEAM AND SHALL BE CONFIRMED BY THE CONTRACTOR

2.0 FASTENER LOADING:

- FASTENER DESIGN IS IN ACCORDANCE WITH NDS 2018
- COLUMN WEIGHTS ARE BASED ON SHOP DRAWINGS AND ARE TO BE CONFIRMED BY 2.2
- MAXIMUM WEIGHT OF COLUMN ASSUMED TO BE 5.800LBS, CONTRACTOR TO NOTIFY
- ASPECT STRUCTURAL ENGINEERS IMMEDIATELY IF BEAM WEIGHT IS TO BE EXCEEDED. TOTAL FACTORED LOAD [W.] FOR LIFTING HAS BEEN DETERMINED AS FOLLOWS (PER MTC RIGGING DESIGN GUIDE, PUBLISHED IN 2020):

$P = p \times K_{os} \times K_{v}$

- A TOS A TAY

 = UNFACTORED WEIGHT OF BEAM = 5,800lbf

 = OPTIONAL SAFETY FACTOR TO ACCOUNT FOR SITE BASED LIFTING HAZARDS = 1.2
- = DYNAMIC ACCELERATION FACTOR = 1.4 [BASED ON STATIONARY MOBILE CRANE]

3.0 RIGGING DETAILS

- RIGGING DESIGN REMAINS THE RESPONSIBILITY OF RIGGING ENGINEER ANY RIGGING INFORMATION SHOWN ON THIS DRAWING IS FOR ILLUSTRATIVE PURPOSES ONLY
- THE RIGGING ARRANGEMENT SHALL BE PROVIDED TO ENSURE AN EVEN LOAD DISTRIBUTION
- SLINGS AND ASSOCIATED RIGGING SHALL BE POSITIONED TO PRODUCE AN ANGLE NO LESS THAN 60 DEGREES FROM THE HORIZONTAL AT ALL LIFTING POINTS.

GLULAM BEAM LIFT GENERAL NOTES

1.0 CENTRE OF GRAVITY

C.O.G MAY VARY FROM BEAM TO BEAM AND SHALL BE CONFIRMED BY THE CONTRACTOR.

2.0 FASTENER LOADING:

- 2.1 FASTENER DESIGN IS IN ACCORDANCE WITH NDS 2018
- BEAM WEIGHTS ARE BASED ON SHOP DRAWINGS AND ARE TO BE CONFIRMED BY 2.2
- MAXIMUM WEIGHT OF BEAM ASSUMED TO BE 7 000 LBS. CONTRACTOR TO NOTIFY
- ASPECT STRUCTURAL ENGINEERS IMMEDIATELY IF BEAM WEIGHT IS TO BE EXCEEDED.

 TOTAL FACTORED LOAD [W], FOR LIFTING HAS BEEN DETERMINED AS FOLLOWS (PER MTC RIGGING DESIGN GUIDE, PUBLISHED IN 2020):

= UNFACTORED WEIGHT OF BEAM = 7,000lbf

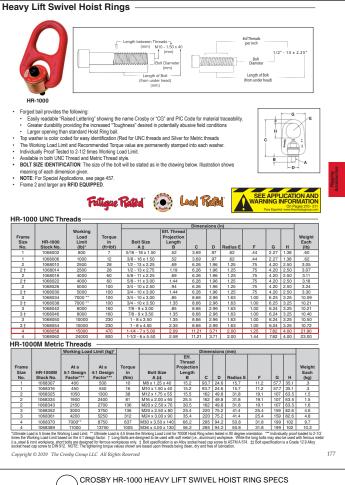
= ONTHAN TORLE WEIGHT OF DEAM = 1,000001

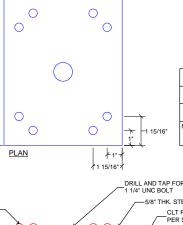
= OPTIONAL SAFETY FACTOR TO ACCOUNT FOR SITE BASED LIFTING HAZARDS = 1.2

= DYNAMIC ACCELERATION FACTOR = 1.4 [BASED ON STATIONARY MOBILE CRANE]

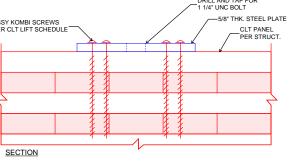
3.0 RIGGING DETAILS

- RIGGING DESIGN REMAINS THE RESPONSIBILITY OF RIGGING ENGINEER ANY RIGGING INFORMATION SHOWN ON THIS DRAWING IS FOR ILLUSTRATIVE PURPOSES ONLY
- THE RIGGING ARRANGEMENT SHALL BE PROVIDED TO ENSURE AN EVEN LOAD DISTRIBUTION
- SLINGS AND ASSOCIATED RIGGING SHALL BE POSITIONED TO PRODUCE AN ANGLE NO LESS THAN 60 DEGREES FROM THE HORIZONTAL AT ALL LIFTING POINTS.





PANEL TYPE	MAX. WEIGHT	FASTENERS	
3-PLY (4 1/8")	4500lbf	8 - 1/2"Øx4" ASSY PT KOMBI SCREWS	
7-PLY (9 5/8") / 9-PLY (12 3/8")	12500lbf	8 - 1/2"Øx6 1/2" ASSY FT KOMBI SCREWS	
ASSY FT I	KOMBI SCREW	2500lbf-15000lbf, USE 8 - 1/2" 2500lbf-15000lbf, USE 8 - 1/2" 2500lbf-15000lbf, USE 8 - 1/2" 2500lbf-15000lbf, USE 8 - 1/2" 2500lbf-15000lbf, USE 8 - 1/2"	NOT FALL



1 CLT PANEL LIFTING PLATE

3.5" MIN. SPACING

3.5" MIN.

EDGE DISTANCE

SECTION A

2 15ft & 30ft GLULAM COLUMN LIFTING PLATE

Scale: NTS

3.5" MIN. SPACING

3.5" MIN. EDGE DISTANCE

5 1/4" MIN. SPACING

 \bigcirc

PER

PLAN

5/8" THK, STEEL PLATE.

3 ROWS OF 2 - 1/2"x4 3/4" ASSI KOMBI SCREWS PER FACE

1/4" THK. STEEL PLATE

GLULAM COLUMN

DRILL AND TAP FOR 1 1/4" -UNC BOLT

71/4



ASPECT

STRUCTURAL ENGINEER

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2020-09-22 Issued for Coordination 2020-09-29 Issued for Coordinatio 2020-10-06 Issued for Coordinatio 2020-10-19 Issued for Construction 2020-12-18 Issued for Construction

Plot Date: 2020-10-19

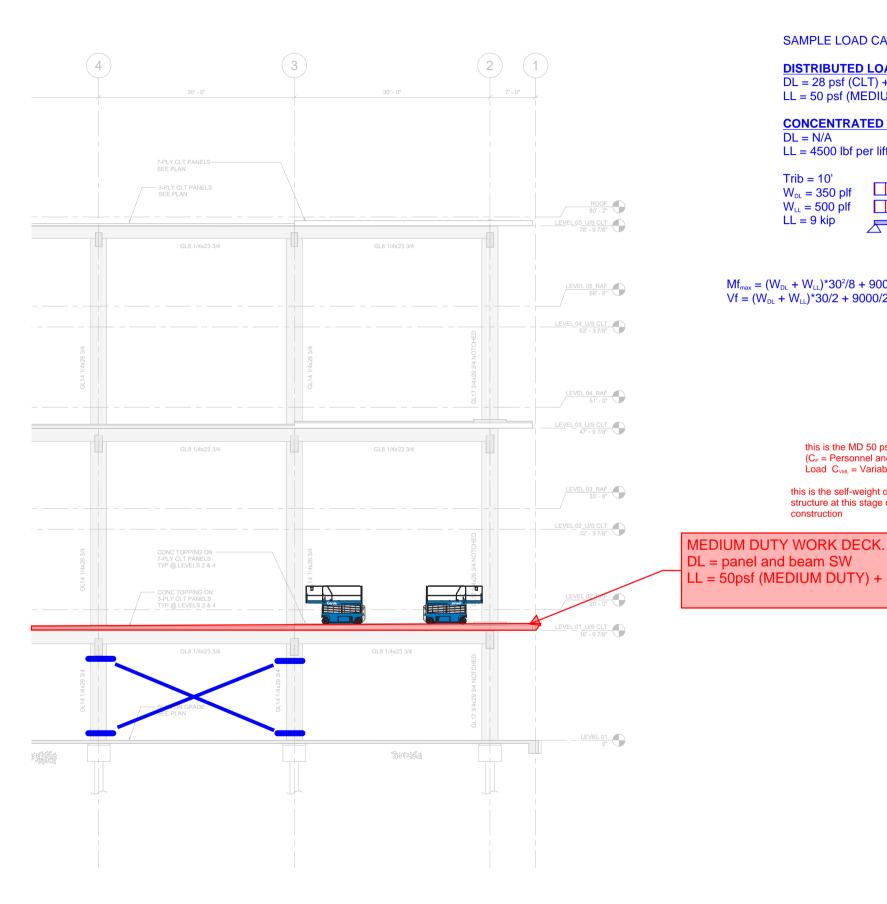
> **Lift Plan General Notes** and Details

NTS

K002

(CLT LIFTING DEVICE)

Kinsol



SAMPLE LOAD CALCULATION FOR STAGED CONSTRUCTION OF RIB IN 7 PLY AREA:

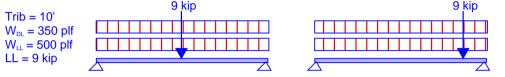
DISTRIBUTED LOADS:

DL = 28 psf (CLT) + 5 psf (RIB) = 33 psf --> **35 psf** (ASSUMED DL) LL = 50 psf (MEDIUM DUTY)

CONCENTRATED LOADS:

DL = N/A

LL = 4500 lbf per lift x 2 = 9000 lbf for side by side lifts (worst case)



 $Mf_{max} = (W_{DL} + W_{LL})*30^2/8 + 9000*30/4 = 163 \text{ kip-ft} \leftarrow$ $Vf = (W_{DL} + W_{LL})*30/2 + 9000/2 = 17.3 \text{ kip}$

this is the MD 50 psf deck load

(C_P = Personnel and Equipment

this is the self-weight of permanent structure at this stage of

construction

LL = 50psf (MEDIUM DUTY) + Scissor Lift

DL = panel and beam SW

Load C_{VML} = Variable Material Load)

 $Vf_{max} = (W_{DL} + W_{LL})*30/2 + 9000 = 21.8 \text{ kip}$

ASD LOAD COMBOS PER ASCE-SEI 7-16

2.3.1 Additive Combinations When using load values provided in this standard for ASD, sufficient additive load combinations shall be considered to obtain the maximum design load effects for members and systems.

The following basic combinations shall be investigated as a minimum:

$$D + C_D + C_{FML} + C_{VML} + L$$
 (2-8)

Governing loads on ribs to LRFD during -current stage of construction

$$D + C_D + C_{FML} + C_{VML} + C_P + C_H + L \qquad (2-9)$$

$$D + C_D + C_{FML} + C_{VML} + 0.6W + C_P + L$$
 (2-10)

$$D + C_D + C_{FML} + C_{VML} + 0.7E + C_P + L$$
 (2-11)

$$0.6D + C_D + (0.6W \text{ or } 0.7E)$$
 (2-12)

ASPECT STRUCTURAL ENGINEERS

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Project No.: 1535

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Date:	Revision / Issue:	No.:
2020-09-22	Issued for Coordination	
2020-09-29	Issued for Coordination	
2020-10-06	Issued for Coordination	
2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	1
2024 02 00	lancard for Countries	

Plot Date:

2020-10-19

CLT Calculation Example (In response to RFI #50 from Katerra)

Scale: NTS

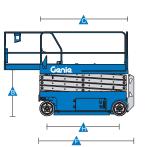


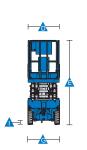


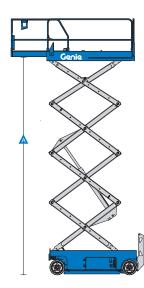
Self-Propelled Scissor Lifts

GS²-2032, GS-2632 & GS-3232

Speci^{*}cations







www.genielift.com

GS-2032 US 26 ft 20 ft 3 ft 5 in 7 ft 5 in 10 ft 5 in deackt	Metric 8.10 m 6.10 m 1.03 m 2.26 m	GS-263 US 32 ft 26 ft 3 ft 10 in	Metric 9.92 m 7.92 m 1.16 m	GS-32 US 38 ft 32 ft	Metric 11.75 m 9.75 m
26 ft 20 ft 3 ft 5 in 7 ft 5 in 10 ft 5 in	8.10 m 6.10 m 1.03 m	32 ft 26 ft	9.92 m 7.92 m	38 ft	11.75 m
20 ft 3 ft 5 in 7 ft 5 in 10 ft 5 in	6.10 m 1.03 m	26 ft	7.92 m		
3 ft 5 in 7 ft 5 in 10 ft 5 in	1.03 m		-)	32 ft	9 75 m
3 ft 5 in 7 ft 5 in 10 ft 5 in		3 ft 10 in	116		
7 ft 5 in 10 ft 5 in		•	I.Io m	4 ft 3 in	1.29 m
10 ft 5 in	2.20 111	7 ft 5 in	2.26 m	7 ft 5 in	2.26 m
	3.18 m	10 ft 5 in	3.18 m	10 ft 5 in	3.18 m
	0.91 m	3 ft	0.91 m	3 ft	0.91 m
2 ft 8 in	0.81 m	2 ft 8 in	0.81 m	2 ft 8 in	0.81 m
3 ft 3 in	0.01 III	ŠŠŠ	ŠŠŠ	ŠŠŠ	ŠŠŠ
	(1.10 m
		·			
6 in	0.15 m	6 IN	0.15 m	6 IN	0.15 m
	}		3	~ ~ ~	~ ~ ~
	(ŠŠŠ
	<i>(</i>	<u> </u>	1		2.43 m
5 ft 9 in	1.75 m	6 ft 3 in	1.91 m 👌	6 ft 8 in	2.03 m
8 ft	2.44 m	8 ft	2.44 m	8 ft	2.43 m
10 ft 11 in	3.33 m	10 ft 11 in	3.33 m	10 ft 11 in	3.33 m
2 ft 8 in	0.81 m	2 ft 8 in	0.81 m	2 ft 8 in	0.81 m
6 ft 1 in	1.85 m	6 ft 1 in	1.85 m	6 ft 1 in	1.85 m
		<u> </u>			8.9 cm
	(,		2.2 cm
•		-			
	}	.	3		
y 2	(2	7	2	
800 lbs	363 kg	500 lbs	227 kg	500 lbs	227 kg
250 lbs	113 kg	250 lbs	113 kg 🔾	250 lbs	113 kg
					6.71 m
					3.5 km/h
	0.8 km/h	,	0.8 km/h	'	0.8 km/h
30%		25%		25%	
ŠŠŠ	ŠŠŠ	ŠŠŠ	ššš {	3°	
ččč	ččč	, Ščč čč	éě 5° }		
	333	·	3 3	zero	
	2 13 m		2 13 m		2.13 m
	2.10111	<u> </u>			2.10111
		·			
<u> </u>					
		<u> </u>			
15 x 5 in	38 x 13 cm	15 x 5 in	38 x 13 cm	15 x 5 in	38 x 13 cr
		•	3		
041/20	}		1	041/50	
					Ah batterie
4.5 gal	17 L	4.5 gai		4.5 yai	17 L
g ⁴	,	- -	3		
4,435 lbs	2,012 kg	4,413 lbs	2,002 kg	5,185 lbs	2,352 kg
•	2,012 kg 771 kg	4,413 lbs 1,773 lb	2,002 kg 804 kg	5,185 lbs 1,700 lb	2,352 kg 771 kg
4,435 lbs		,			2,352 kg 771 kg 781 kPa 13.92 kP
	10 ft 11 in 2 ft 8 in 6 ft 1 in 2 ft 8 in 6 ft 1 in or 3.5 in by6d88 in yy 2 800 lbs 250 lbs full height 2.2 mph 0.5 mph 30% ŠŠŠ zero 7 ft 28 / 24 sec proportional dual front w dual rear wf 15 x 5 in 24 V DC (four 6V 225 4.5 gal	6 ft 8 in 2.03 m 7 ft 2.13 m 5 ft 9 in 1.75 m 8 ft 2.44 m 10 ft 11 in 3.33 m 2 ft 8 in 0.81 m 6 ft 1 in 1.85 m or 3.5 in 8.9 cm ove 6.88 in 2.2 cm 2800 lbs 363 kg 250 lbs 113 kg full height 2.2 mph 3.5 km/h 0.5 mph 0.8 km/h 30% ŠŠŠ ŠŠŠ Žero 7 ft 2.13 m 28 / 24 sec proportional dual front wheel dual rear wheel 15 x 5 in 38 x 13 cm 24 V DC (four 6V 225 Ah batteries) 4.5 gal 17 L	6 ft 8 in	6 ft 8 in	6 fin 0.15 m 6 fin 0.15 m 6 fin 6 ft 8 in 2.03 m ŠŠŠ ŠŠŠ ŠŠŠ 7 ft 2.13 m 7 ft 5 in 2.5 m 8 ft 5 ft 9 in 1.75 m 6 ft 3 in 1.91 m 6 ft 8 in 8 ft 2.44 m 8 ft 2.44 m 8 ft 10 ft 11 in 3.33 m 10 ft 11 in 3.33 m 10 ft 11 in 2 ft 8 in 0.81 m 2 ft 8 in 0.81 m 2 ft 8 in 6 ft 1 in 1.85 m 6 ft 1 in 1.85 m 6 ft 1 in 1 ar 3.5 in 8.9 cm 3.5 in 8.9 cm 3.5 in 1 ar 3.5 in 8.9 cm 3.5 in 8.9 cm 3.5 in 1 ar 3.5 in 8.9 cm 3.5 in 8.9 cm 3.5 in 2 gy 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2

m

Standards Compliance ANSI A92.6, CSA B354.2, CE Compliance, AS 1418.10

- ¹ The metric equivalent of working height adds 2 m to platform height. U.S. adds 6 ft to platform height.

 ² Gradeability applies to driving on slopes, see operator™s manual for details regarding slope ratings.

 ³ Weight will vary depending on options and/or country standards.

 ⁴ Note: Floor loading information is approximate and does not incorporate different option configurations. It should be used only with adequate safety factors.



Self-Propelled Scissor Lifts

GS²-2032, GS-2632 & GS-3232

Features

Standard Features

Measurements

GS-2032

- Ł 26 ft (8.10 m) working height
- Ł Up to 800 lbs (363 kg) lift capacity GS-2632
- Ł 32 ft (9.92 m) working height
- Ł Up to 500 lbs (227 kg) lift capacity GS-3232
- Ł 38 ft (11.75 m) working height

Ł Up to 500 lbs (227 kg) lift capacity Productivity

- Ł 89 x 32 in (2.26 x .81 m) steel platform
- Ł 3 ft (.91 m) extension deck
- Ł Fixed rail with chain entry gate (standard
- Ł Folding rails with half-height swing gate (standard on G§ -2632, GS² -3232) Ł Automatic leveling outriggers
- (GS² -3232 only)
- Ł Dual front wheel drive Ł Universal 27A smart charger
- Ł Rear recessed charger receptacle
- Ł SmartLink proportional lift and drive
- Ł Platform control with battery charge indicator and diagnostic display
- Ł On-board diagnostic system
- Ł AC power to platform
- Ł Lanyard attachment points
- Ł Manual platform lowering valve
- Ł Emergency stop at both platform and ground controls
- Ł Rear wheel multiple disc brakes
- Ł Front wheel hydraulic dynamic braking
- Ł Manual hydraulic brake release
- Ł Swing-out component trays
- Ł Solid non-marking tires
- Ł Pothole guards

Genie United States 6464 185th Ave. NE

Telephone +1 (425) 881-1800

Redmond, WA 98052

Fax +1 (425) 883-3475

- Ł Tilt level sensor with audible alarm
- Ł Dual flashing LED beacons
- Ł Descent alarm Ł Electronic horn
- Ł Hour meter
- Ł Motion alarm

Ł 24 V DC (four 6 V 225 Ah batteries)

Toll Free in USA/Canada +1 (800)-536-1800

Options & Accessories

Productivity Options

- Ł Folding rails with half-height swing gate (GS -2032)
- Ł Platform swing gate, half-height (GS^{*}-2032)
- Ł Air line to platform
- Ł Automotive horn*
- Ł Biodegradable hydraulic fluid
- Ł Platform control guard

Power Options

- Ł Power Inverter (120 V/60 Hz)
- Ł EE rating
- Ł AGM maintenance-free batteries
- Ł Drive cutout while charging



¹ Not available with EE rating



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Project No.: 1535

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Date:	Revision / Issue:	No.
2020-09-22	Issued for Coordination	
2020-09-29	Issued for Coordination	
2020-10-06	Issued for Coordination	
2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	1
2021-02-09	Issued for Construction	2

Plot Date:

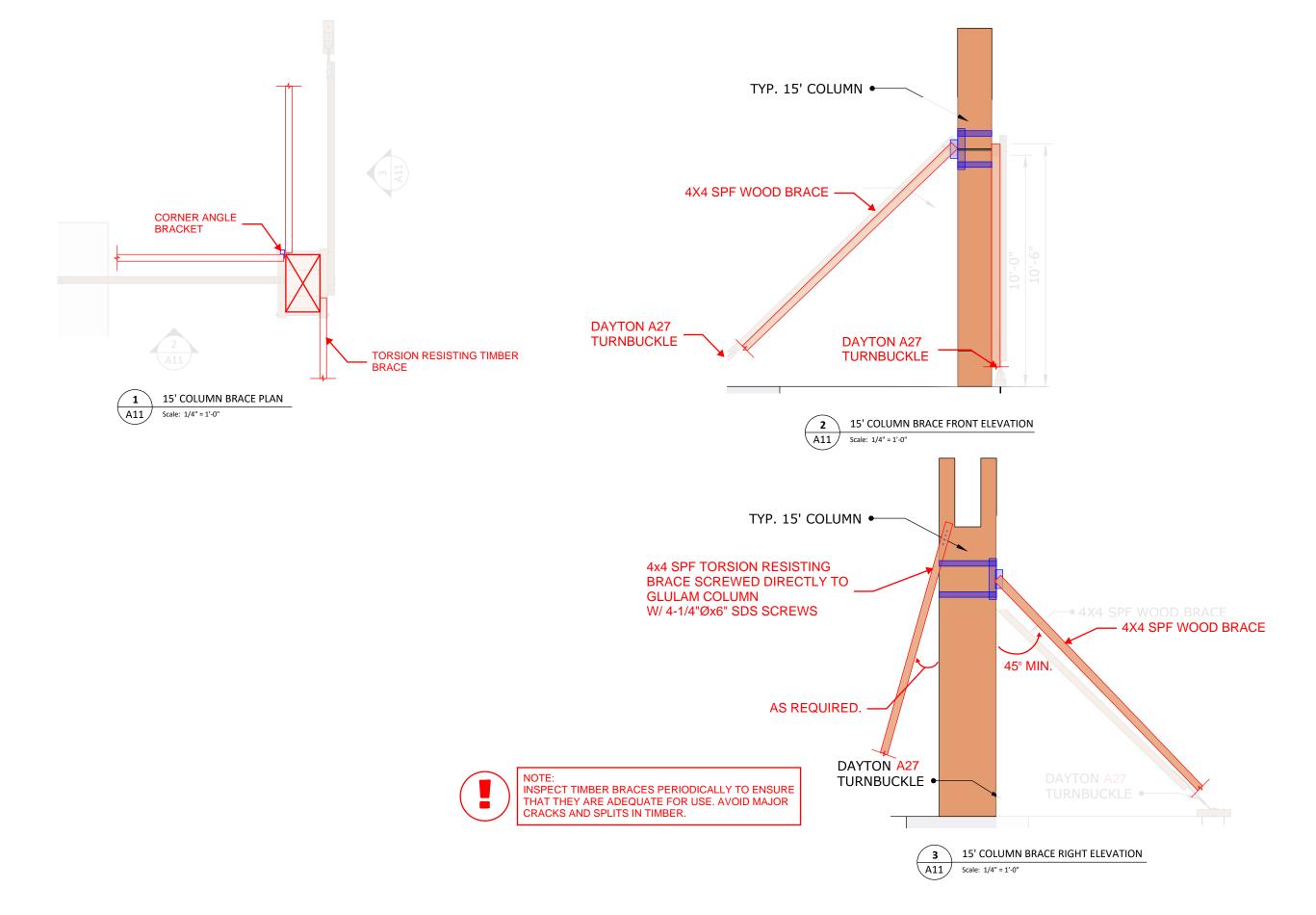
2020-10-19

Genie Self-Propelled Scissor Lift Assumption

K004

Distributed By:

GS32 0210l. Part No. 109378



Kinsol

Timber Systems

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DATE (THIS VERSION) February 9, 2021

REV :	DATE
1	2020 12 18
2	2021-02-09

April 16, 2020

| | CLIENT | < ClientName>

PROJECT NO. <ProjectNumber>

PROJECT
Google FONE

File Name:
google layout

DRAWN BY

DESIGNED BY

DESCRIPTION
15 FOOT BRACE

DETAIL 1

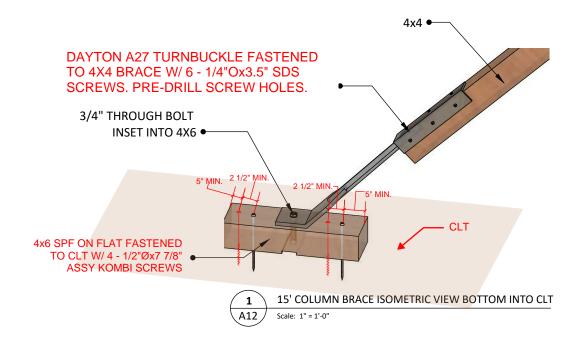
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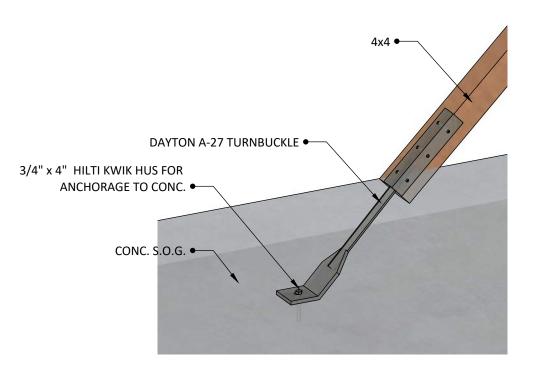


NOTE:

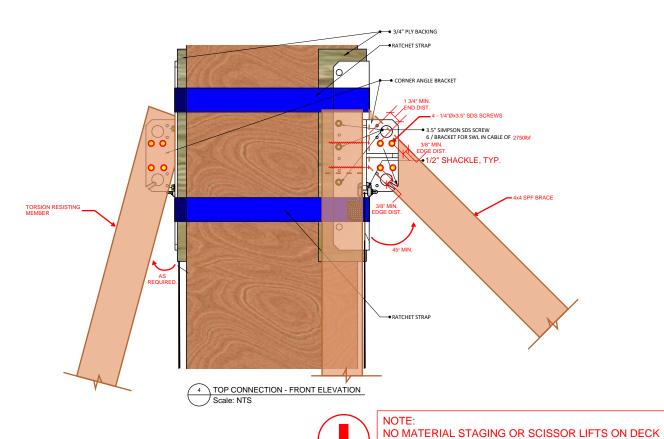
1. IF MOVING BLOCK WITHOUT DAYTON TURNBUCKLE ATTACHED, ADD ADDITIONAL NUT AND WASHER TO SECURE BOLT IN PLACE.

2. DAYTON TO 4X4 SDS SCREWS MUST BE PRE DRILLLED











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File Name:
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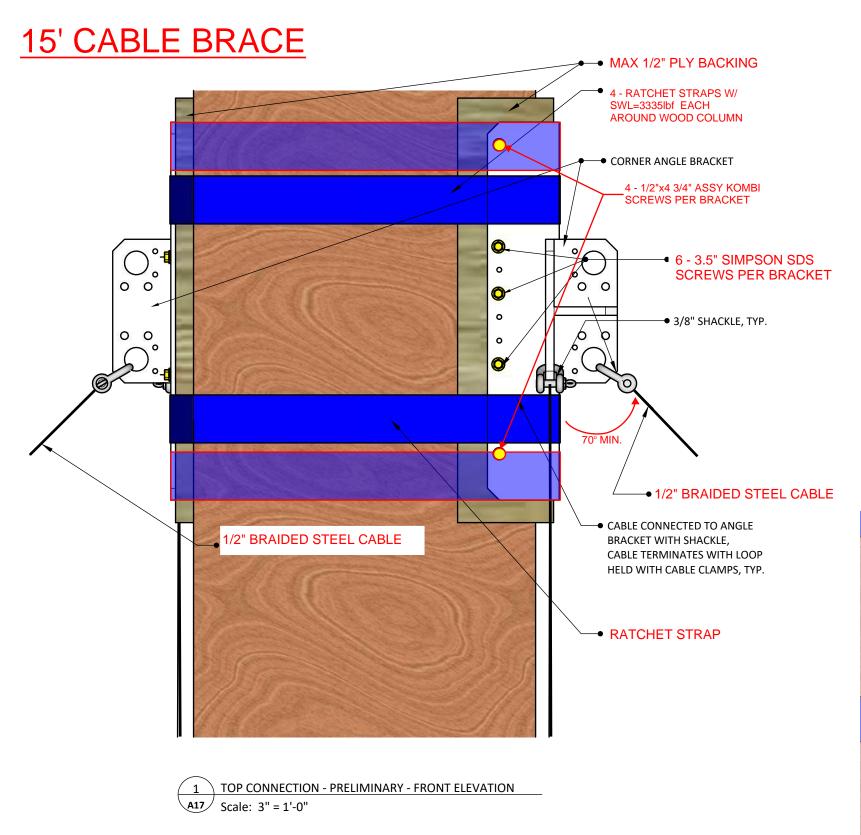
DESCRIPTION
15 FOOT BRACE

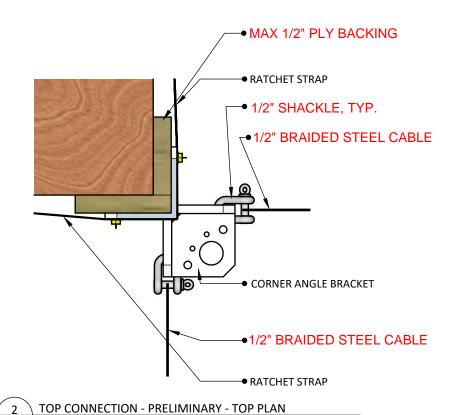
PRIOR TO CABLE BRACING. THIS BRACING IS ONLY

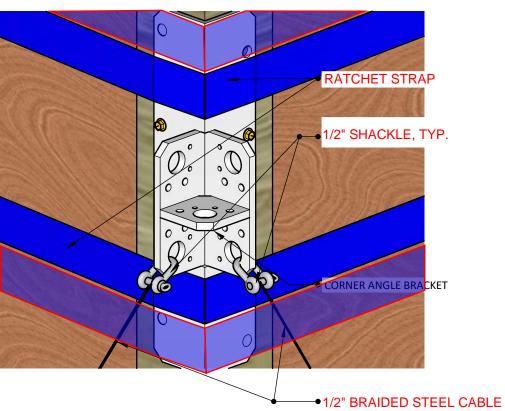
SUITABLE TO SUPPORT A LIGHT DUTY DECK ABOVE.

DETAIL 2









3 TOP CONNECTION - PRELIMINARY - ISO

A17 Scale: 3" = 1'-0"

A17 Scale: 3" = 1'-0"



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	2	2021-02-0	
	DATE CREAT	ED	

April 16, 2020

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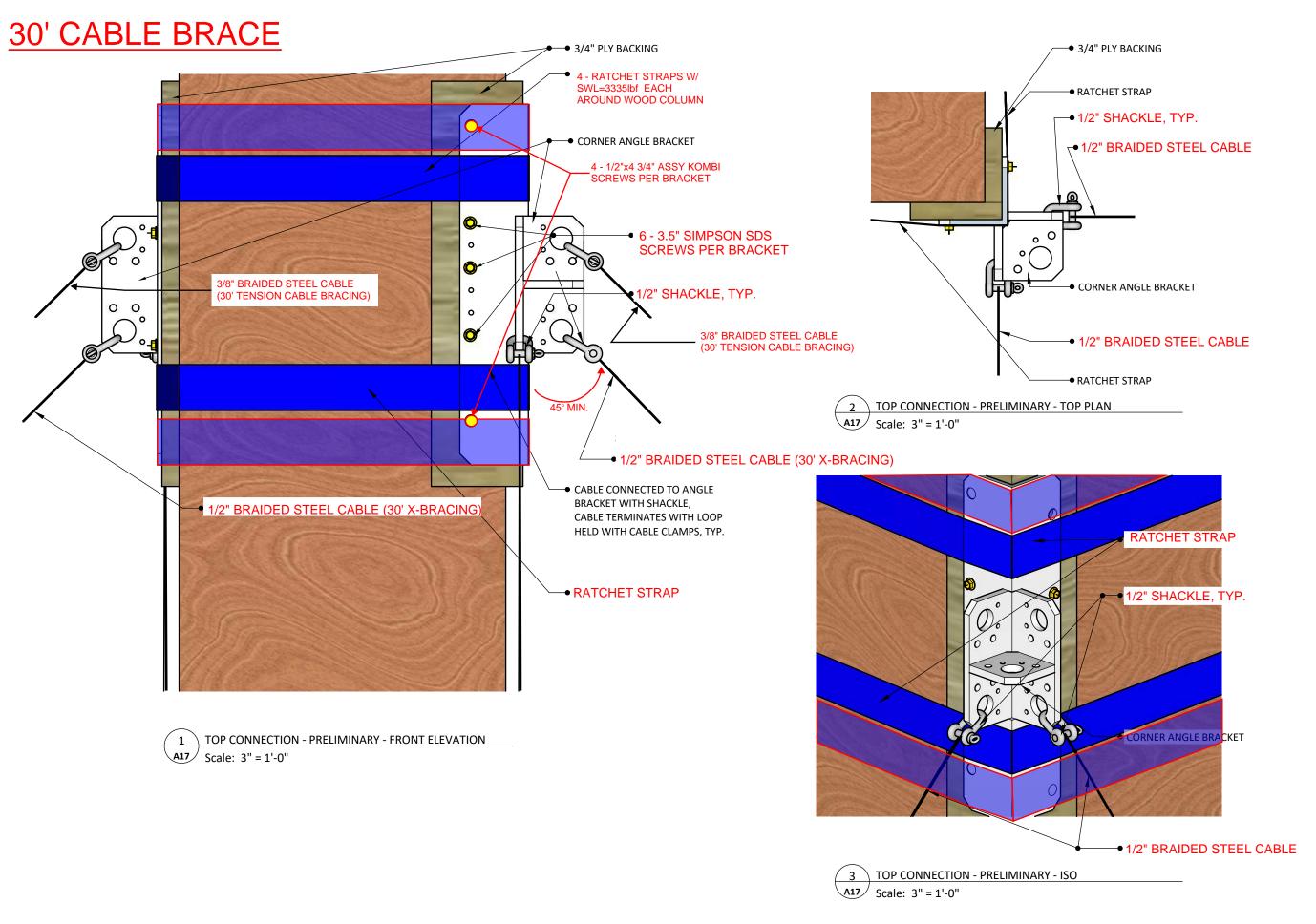
PROJECT
Google FONE

File Name: google layout

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DESCRIPTION
15' CABLE BRACE
TOP DETAIL





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2	2021-02-09
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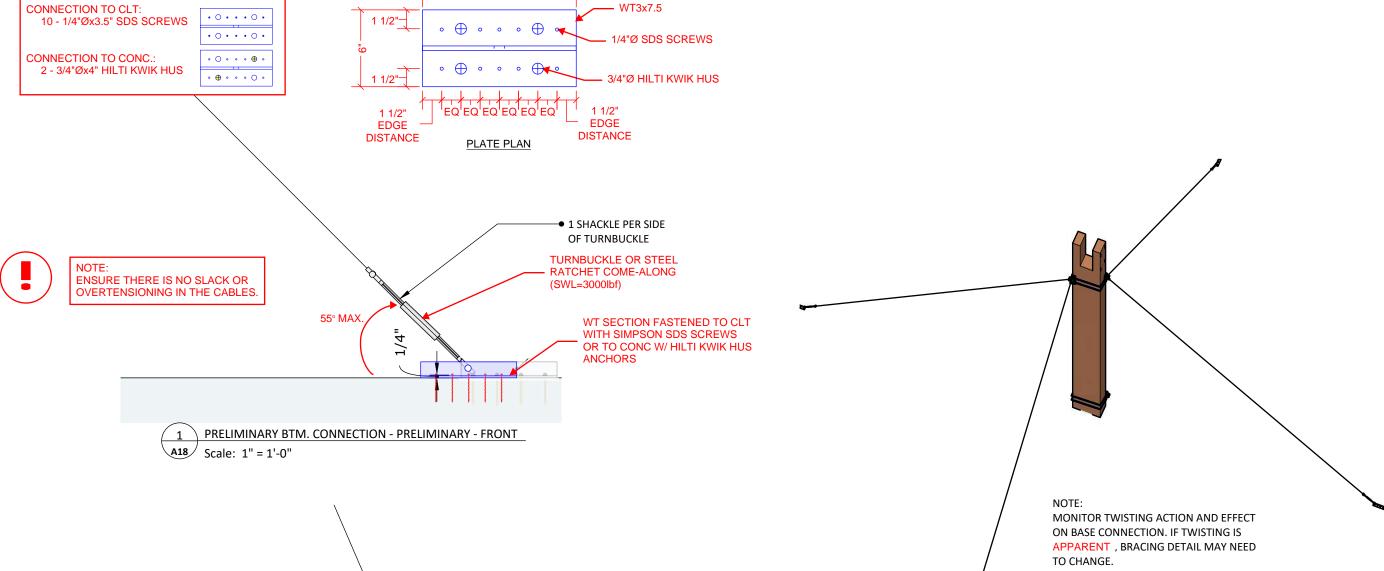
File Name: google layout

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DESIGNED BY

DESCRIPTION
30' CABLE BRACE
TOP DETAIL

30' CABLE BRACE



WT-SECTION FASTENED TO

CONC. W/ HILTI KWIK HUS

CLT W/ SIMPSON SDS SCREWS OR FASTENED TO Kinsol

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GUY LINE CONNECTION - EACH 30' COLUMN - PRELIMINARY

A18 Scale: 1/8" = 1'-0"

DESIGNED BY

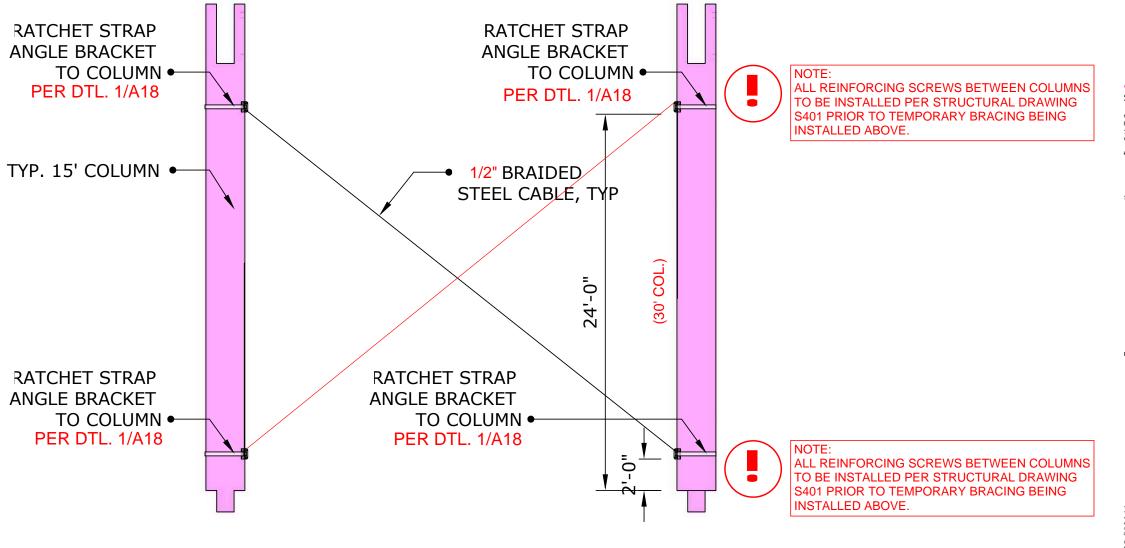
DESCRIPTION 30' TENSION CABLE BRACE BOTTOM

2 PRELIMINARY BTM. CONNECTION - ISOMETRIC A18 Scale: 1" = 1'-0"

TURNBUCKLE •



Kinsol



1 15' and 30' COLUMN CROSS BRACE ELEVATION Scale: NTS

ASPECT

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Seal



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2020-10-06	Issued for Coordination	
2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	1
2024 02 00	Issued for Construction	2

Plot Da

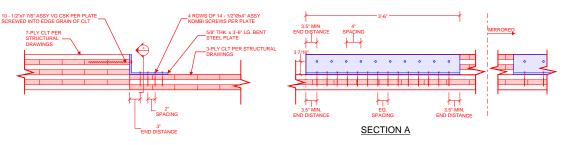
2020-10-19

Drawin

30' Cable X-Brace Detail

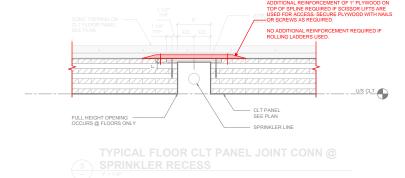
NTS Drawn: N Designed: N Checked: A

Pauling No : Paulisian No :

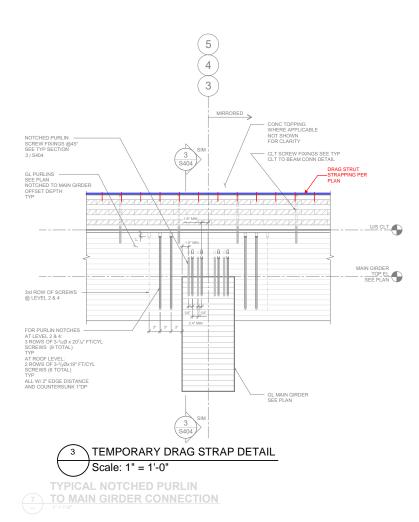


7-PLY TO 3-PLY DRAG CONNECTION Scale: 1" = 1'-0"





TEMPORARY PLYWOOD SPLINE REINFORCEMENT
PRIOR TO CONCRETE POUR
Scale: 1" = 1'-0"



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2020-12-18	Issued for Construction	1
2021-02-00	legued for Construction	2

Plot Date:

2020-10-19

Drawing

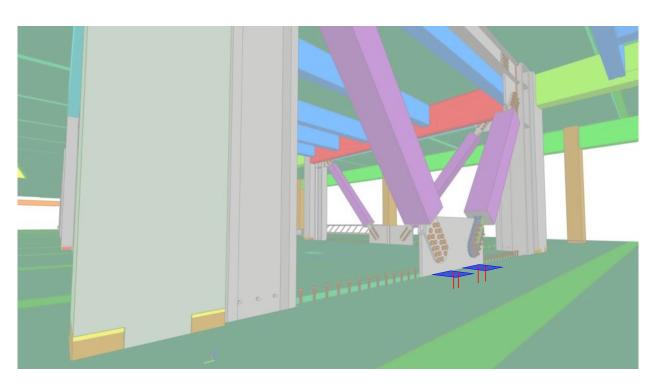
Temporary Diaphragm Details

 Scale:
 Drawn:
 M

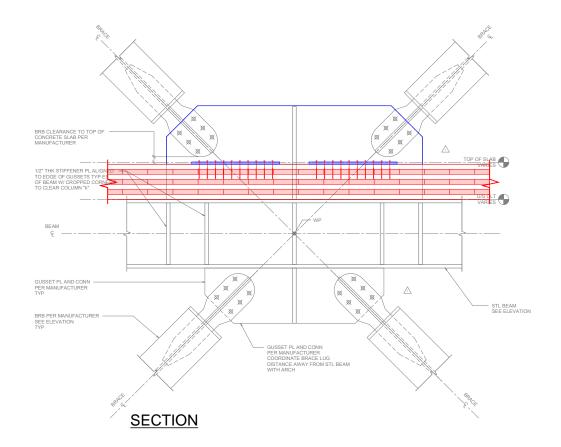
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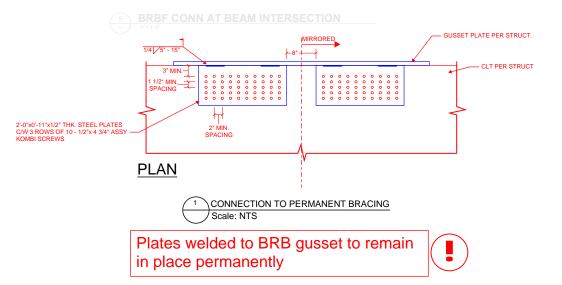
 NTS
 Checked:
 A

rawing No : Buision No :



CONNECTION TO PERMANENT BRACING SCHEMATIC
Scale: NTS





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Sea



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2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	1
2024 02 00	I	

Plot Date

2020-10-19

Drawing

Temporary Diaphragm to Permanent Bracing Detail

 Scale:
 Drawn:
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Weight

3600lbf

Length

32'

Distance from Pivot

47'-180'

A,B



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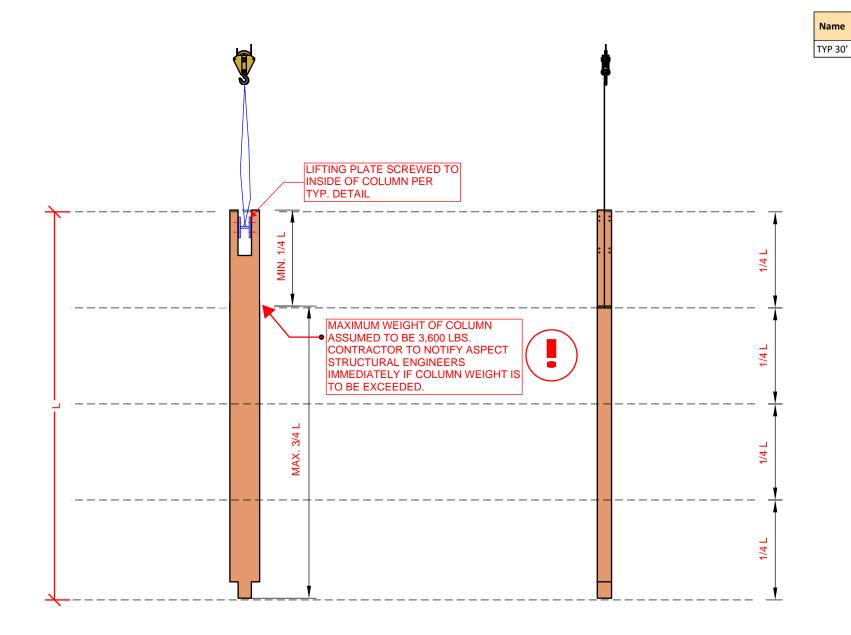
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DESCRIPTION
COLUMN HOIST PLAN





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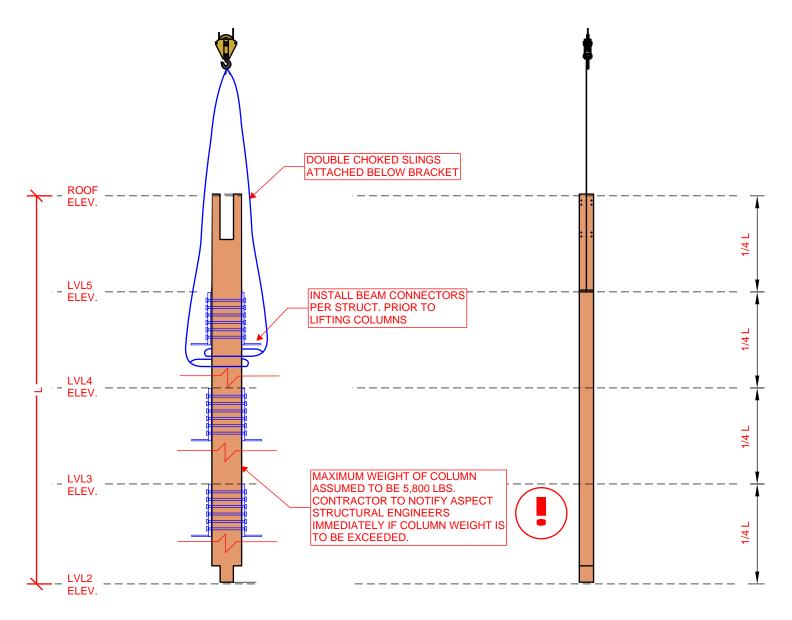
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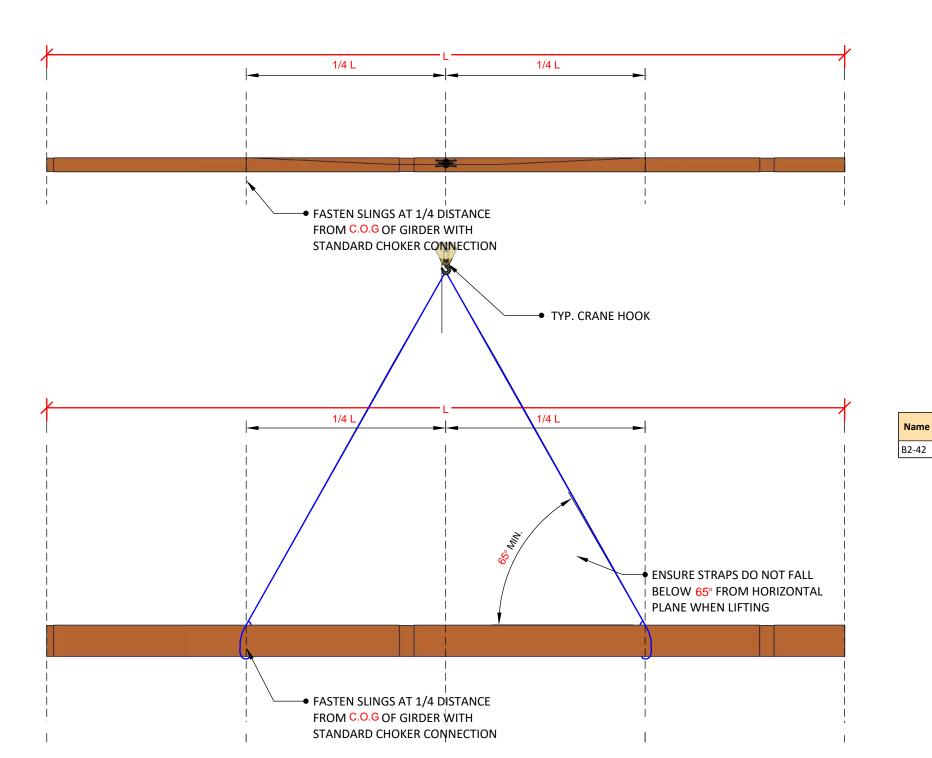
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DESCRIPTION

COLUMN HOIST PLAN (60FT COLUMN)

Name	Crane	Distance from Pivot	Weight	Length
TYP 60'			6800lbf	







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Distance from Pivot Weight

Length

66'-5 7/8"

Crane

149'-0"

REV :	DATE
1	2020 12 18
2	2021-02-09

April 16, 2020

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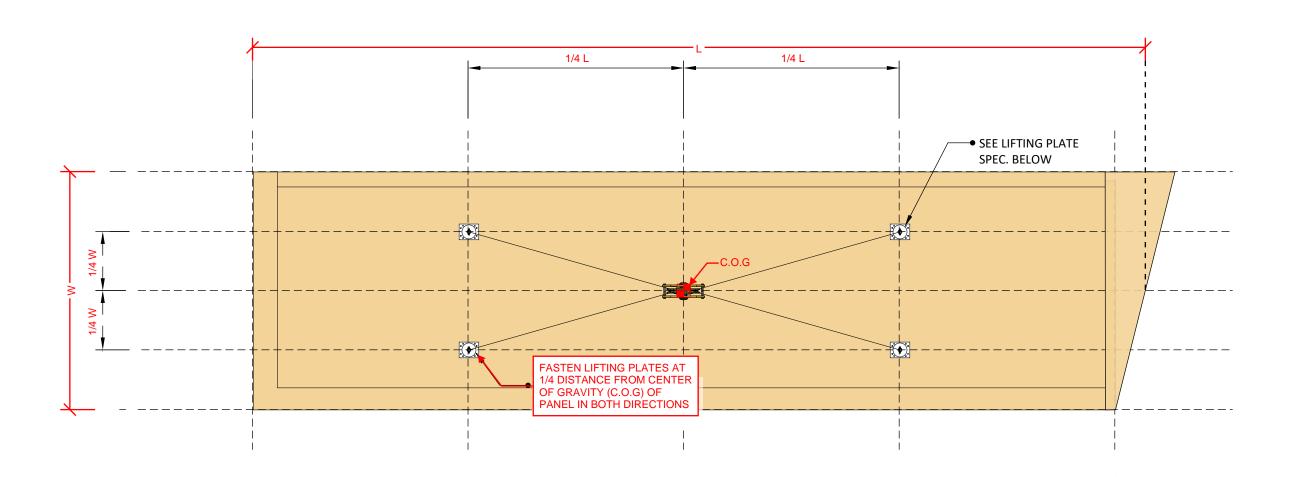
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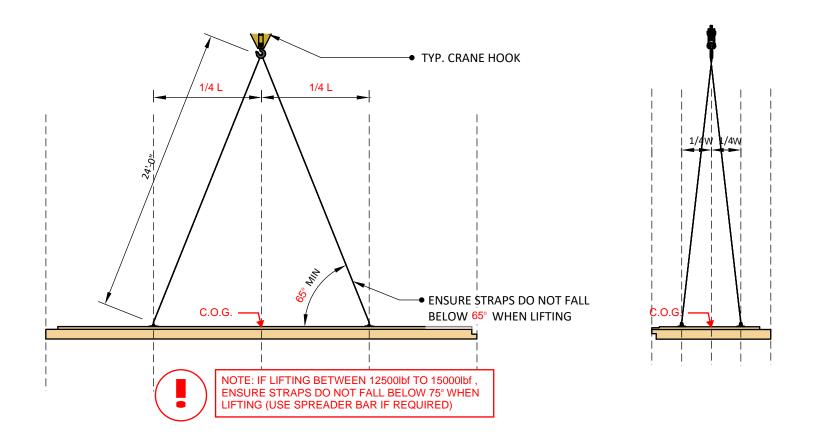
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DESCRIPTION
GIRDER HOIST PLAN







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1	2020 12 1
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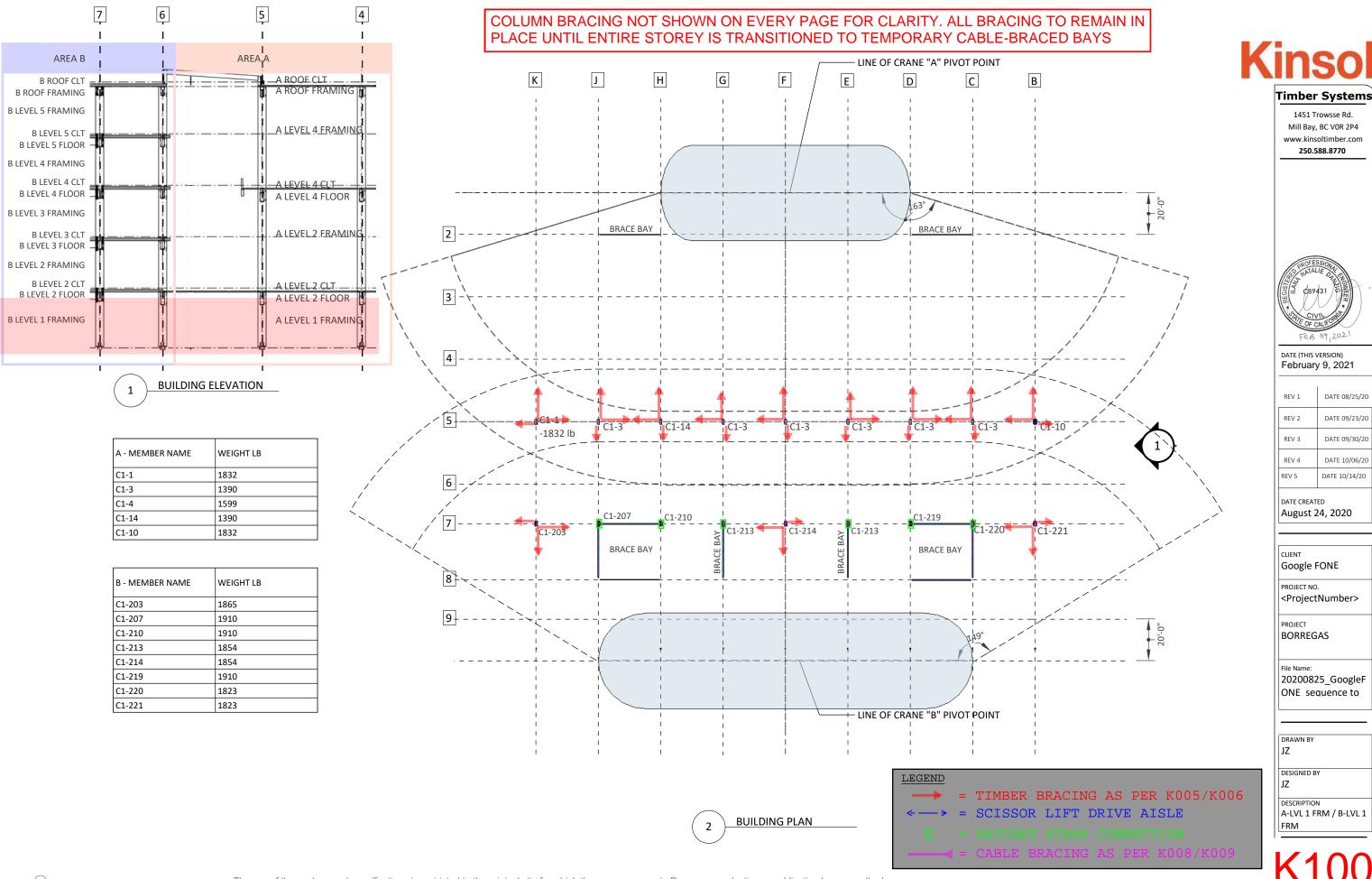
File Name:

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DESCRIPTION

CLT HOIST PLAN



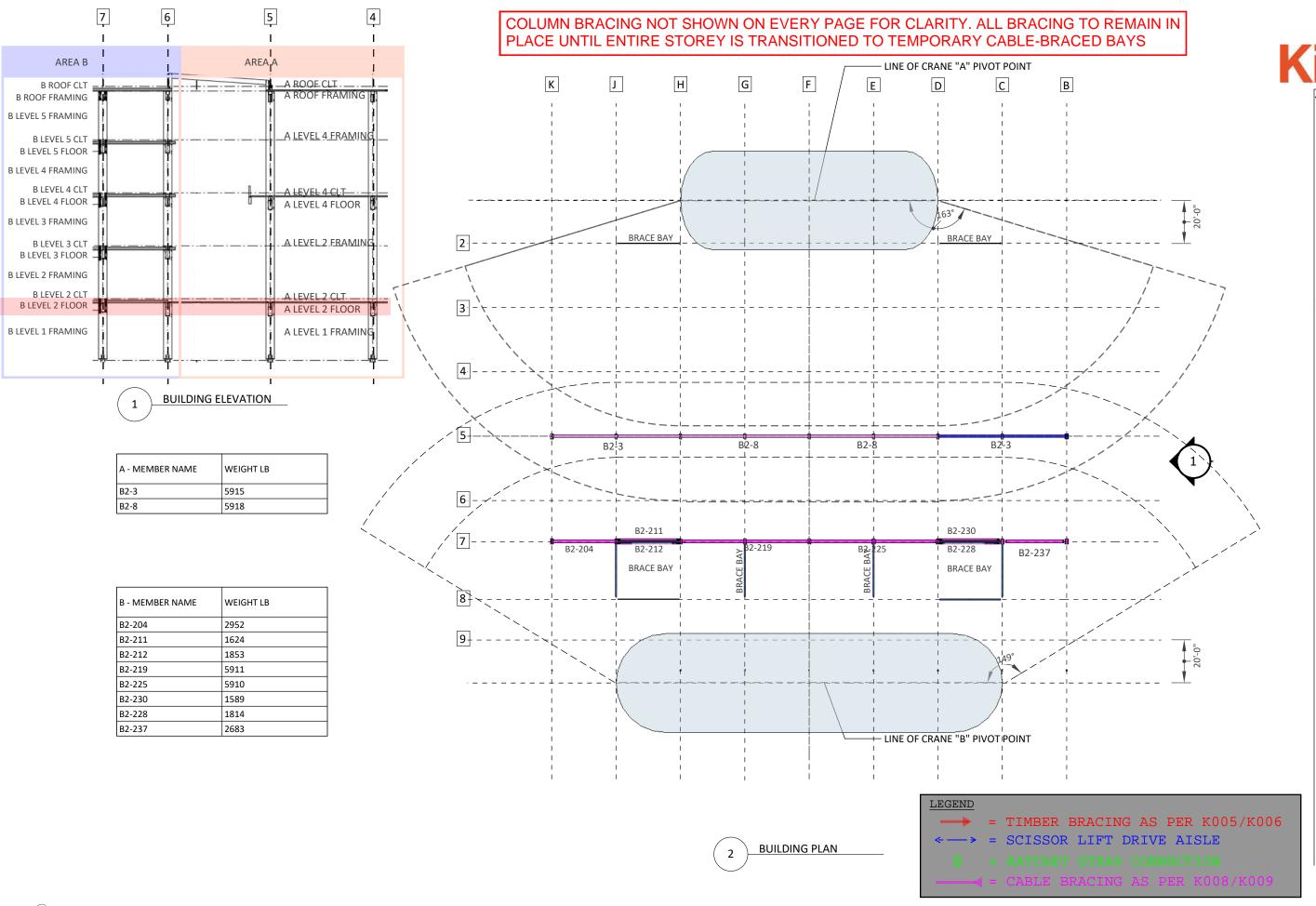
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DATE 08/25/20

DATE 09/23/20

DATE 09/30/20

DATE 10/06/20





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REV 1	DATE 08/25/20
REV 2	DATE 09/23/20
REV 3	DATE 09/30/20
REV 4	DATE 10/06/20
REV 5	DATE 10/14/20

DATE CREATED
August 24, 2020

Google FONE
PROJECT NO.

<ProjectNumber>

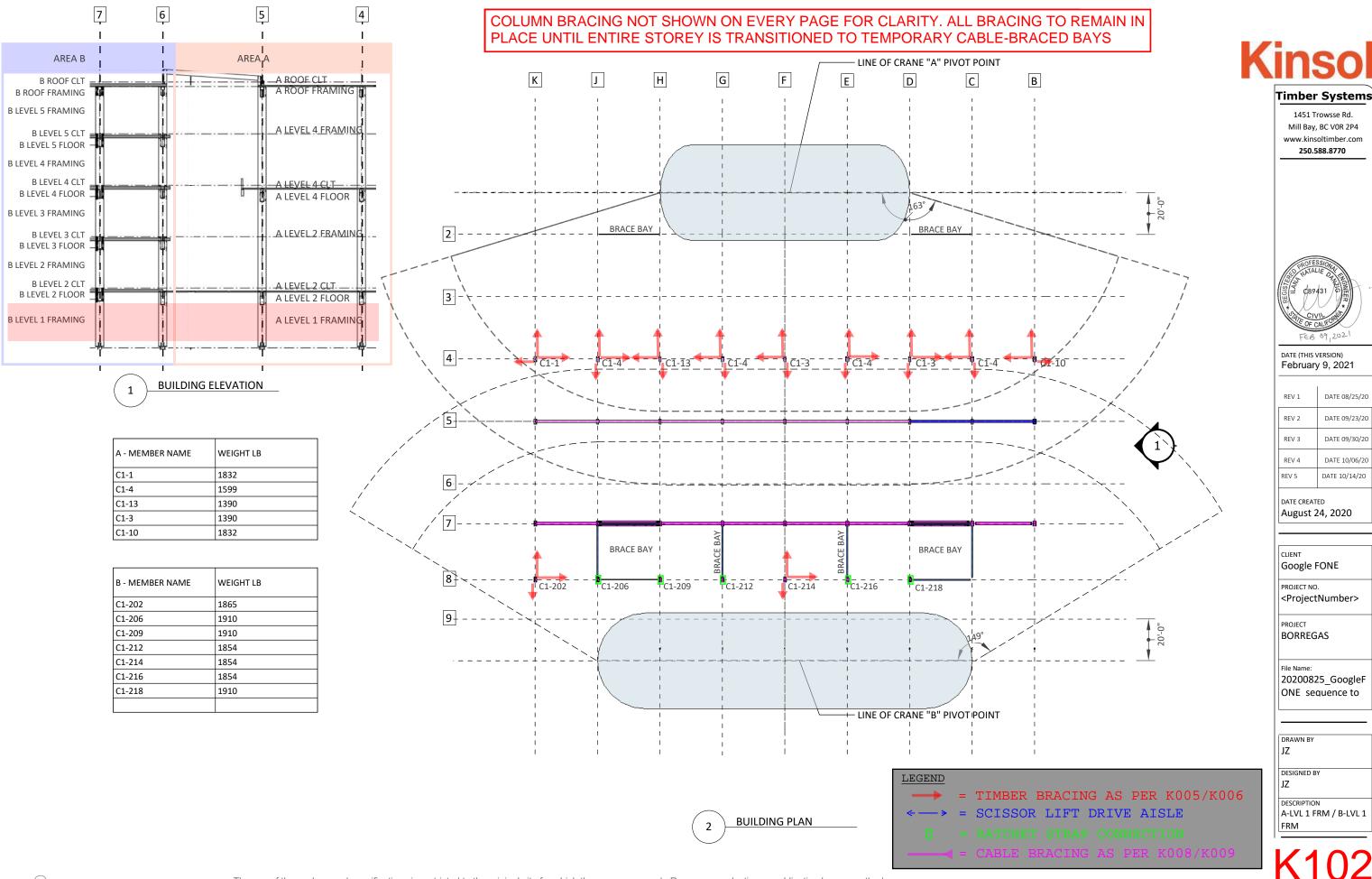
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DESCRIPTION
A-LVL 2 FLR / B-LVL 2
FLR



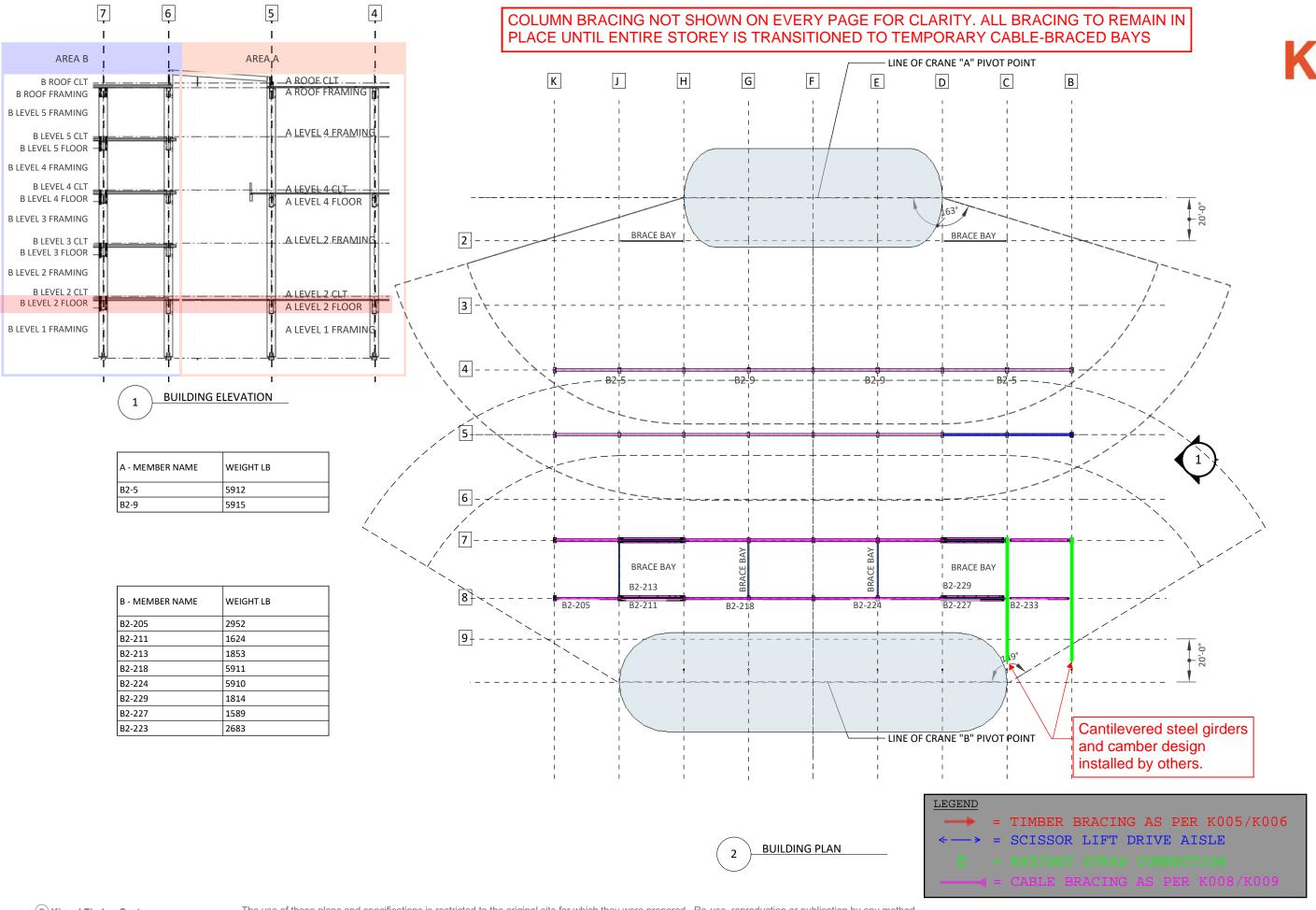
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DATE 09/23/20

DATE 09/30/20

DATE 10/06/20



K103

A-LVL 2 FLR / B-LVL 2

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DATE 08/25/20 DATE 09/23/20

DATE 09/30/20

DATE 10/06/20

DATE 10/14/20

REV 1

REV 2

REV 4

DATE CREATED
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REV 5

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PROJECT

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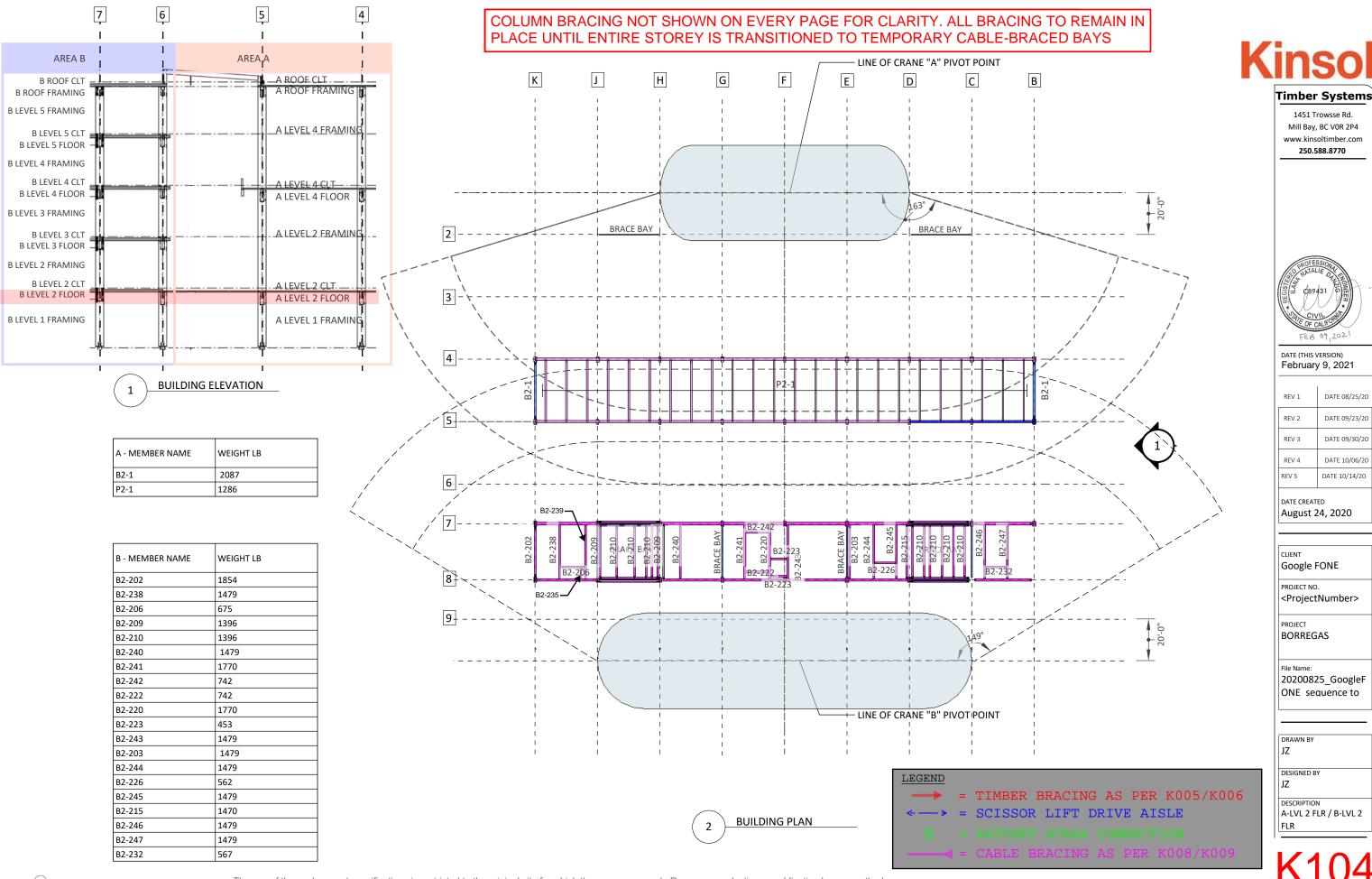
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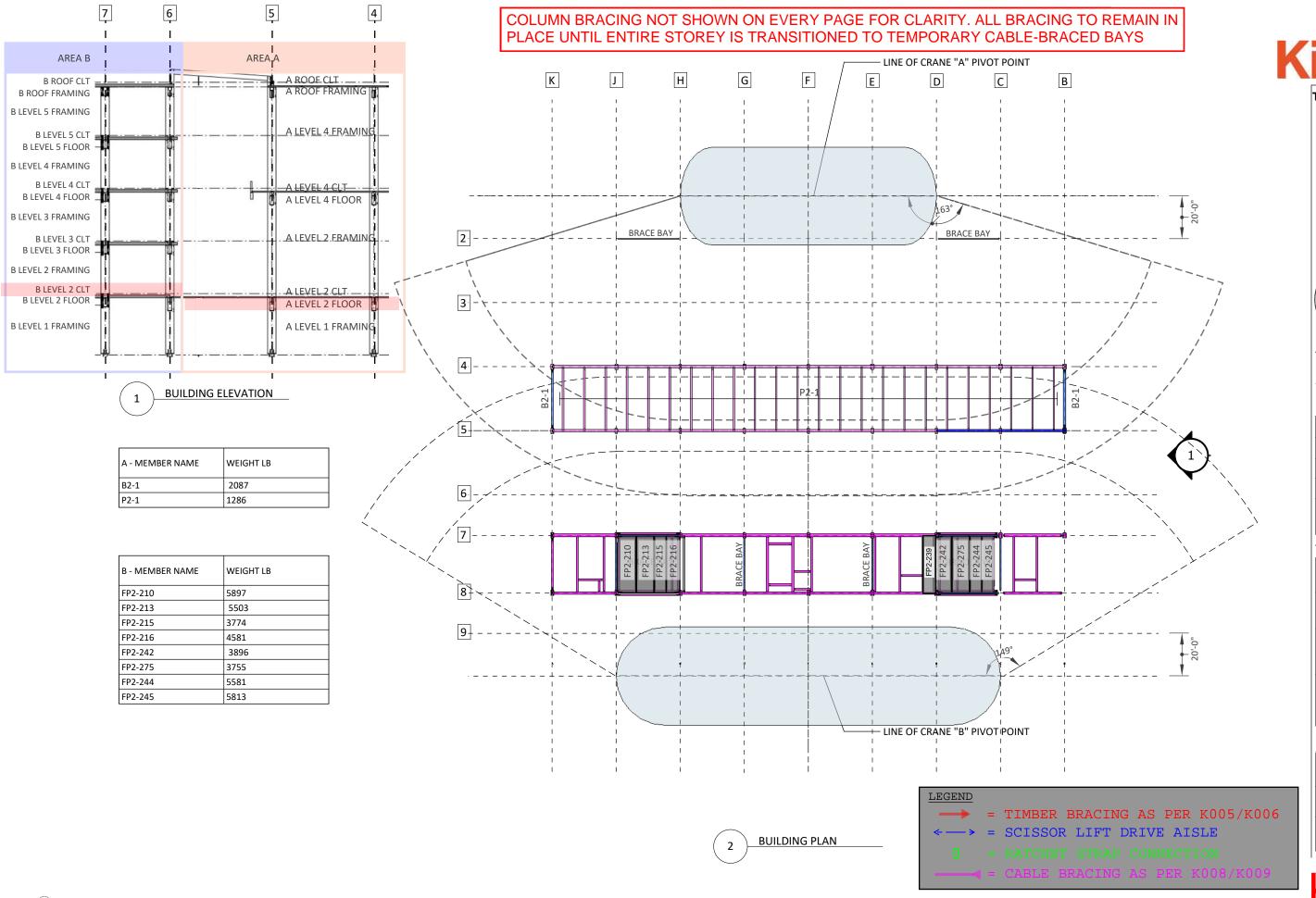
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DATE 08/25/20

DATE 09/23/20

DATE 09/30/20

DATE 10/06/20



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REV 2	DATE 09/23/20	
REV 3	DATE 09/30/20	
REV 4	DATE 10/06/20	
REV 5	DATE 10/14/20	
DATE CREATED		

August 24, 2020

Google FONE

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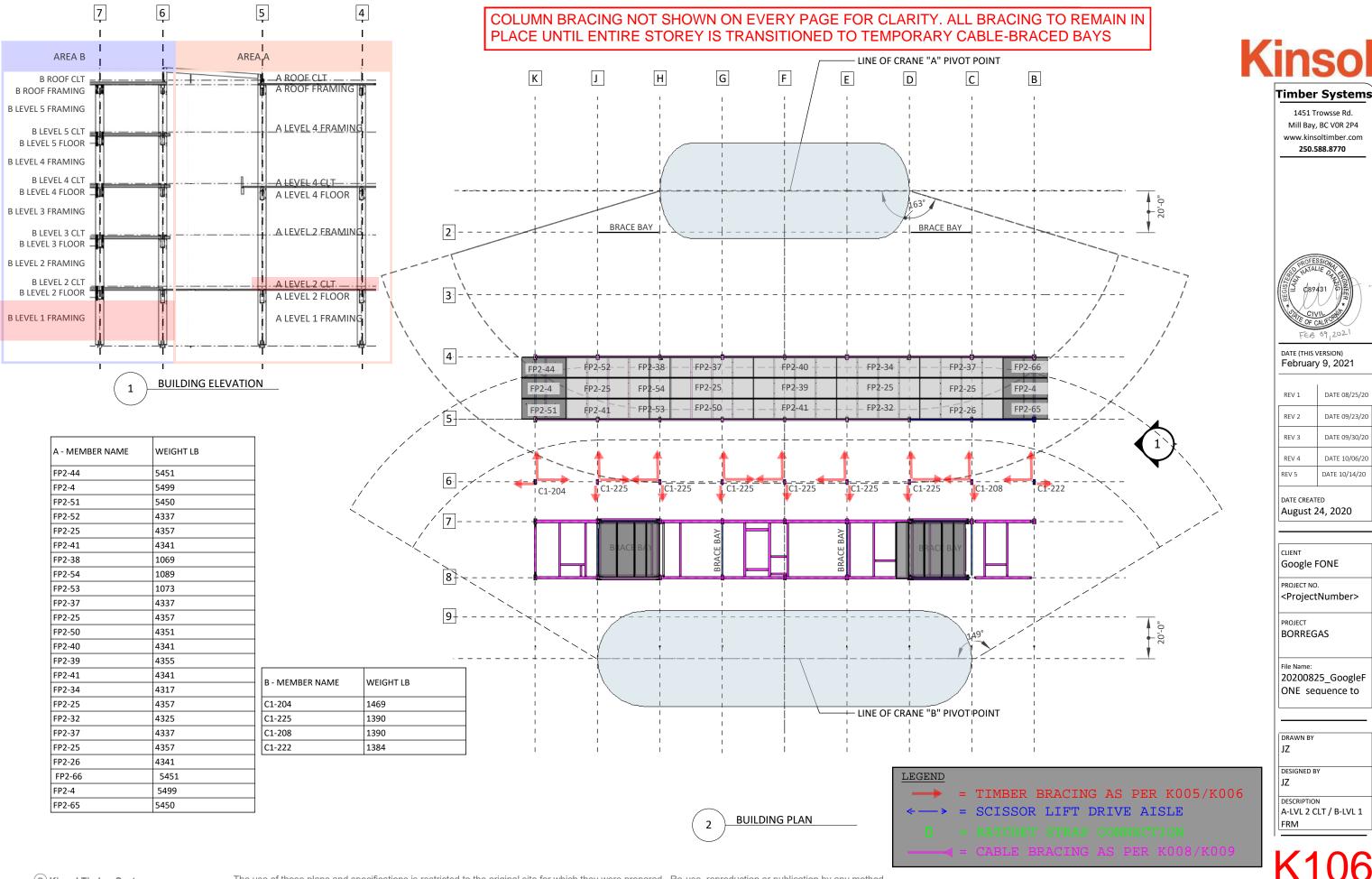
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DESIGNED BY
JZ

DESCRIPTION

B-LVL 2 CLT BRACE
BAY



A-LVL 2 CLT / B-LVL 1

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REV 1

REV 2

REV 3

REV 4

DATE CREATED

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20200825 GoogleF

ONE sequence to

REV 5

CLIENT

PROJECT NO.

PROJECT

File Name:

DRAWN BY

DESIGNED BY

DESCRIPTION

JΖ

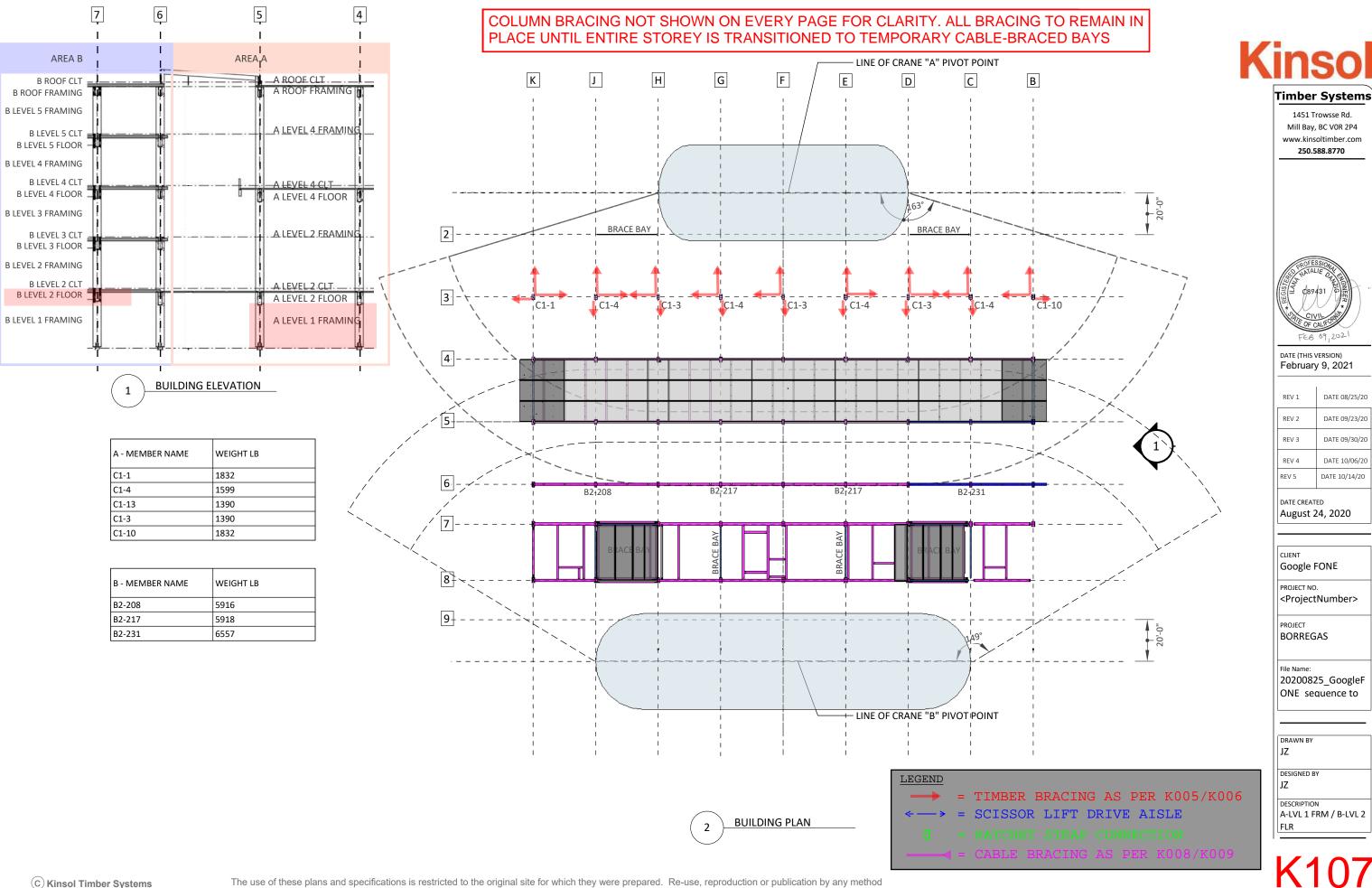
BORREGAS

DATE 08/25/20

DATE 09/23/20

DATE 09/30/20

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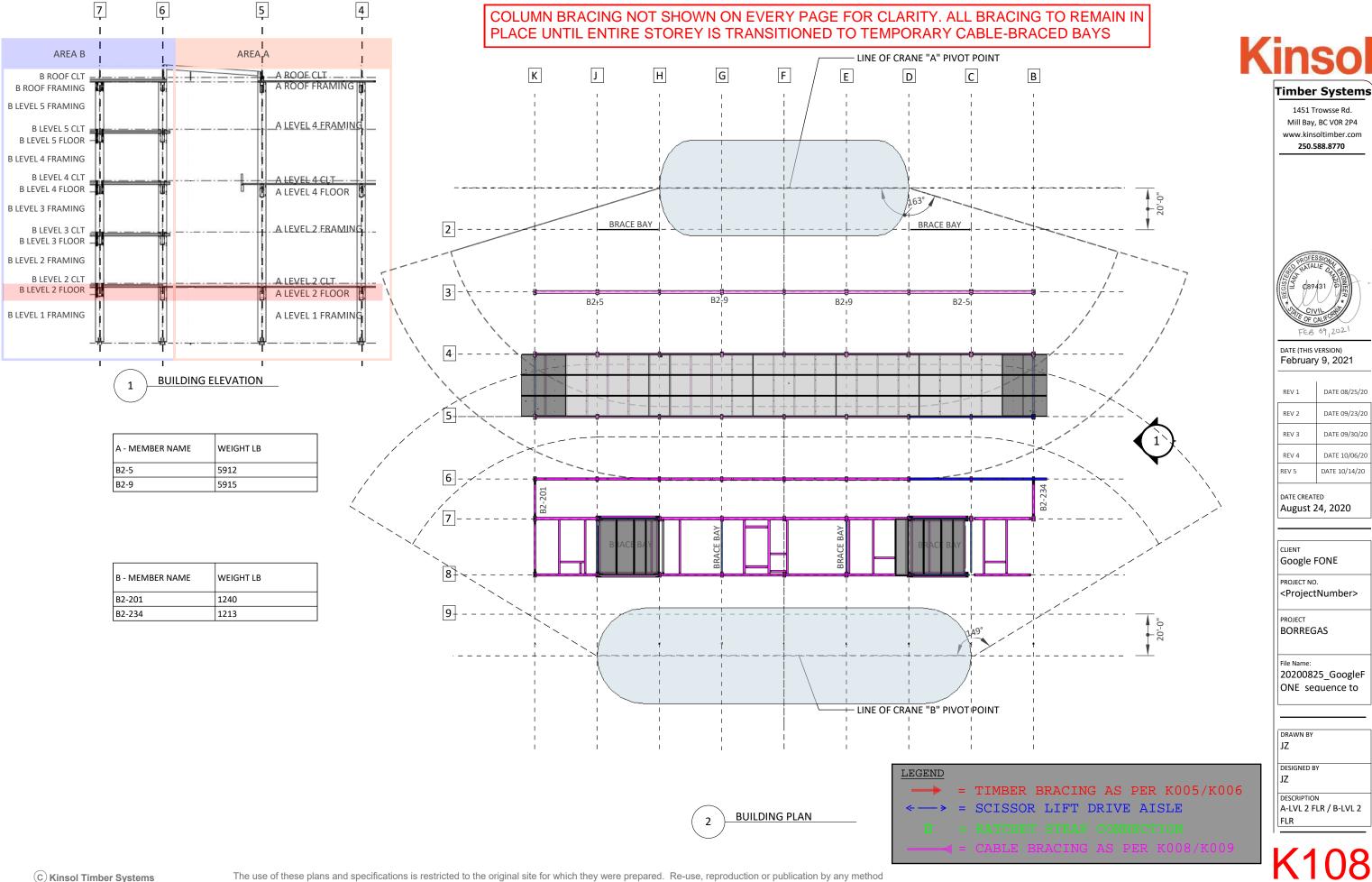
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REV 4

DATE 08/25/20 DATE 09/23/20

DATE 09/30/20

DATE 10/06/20

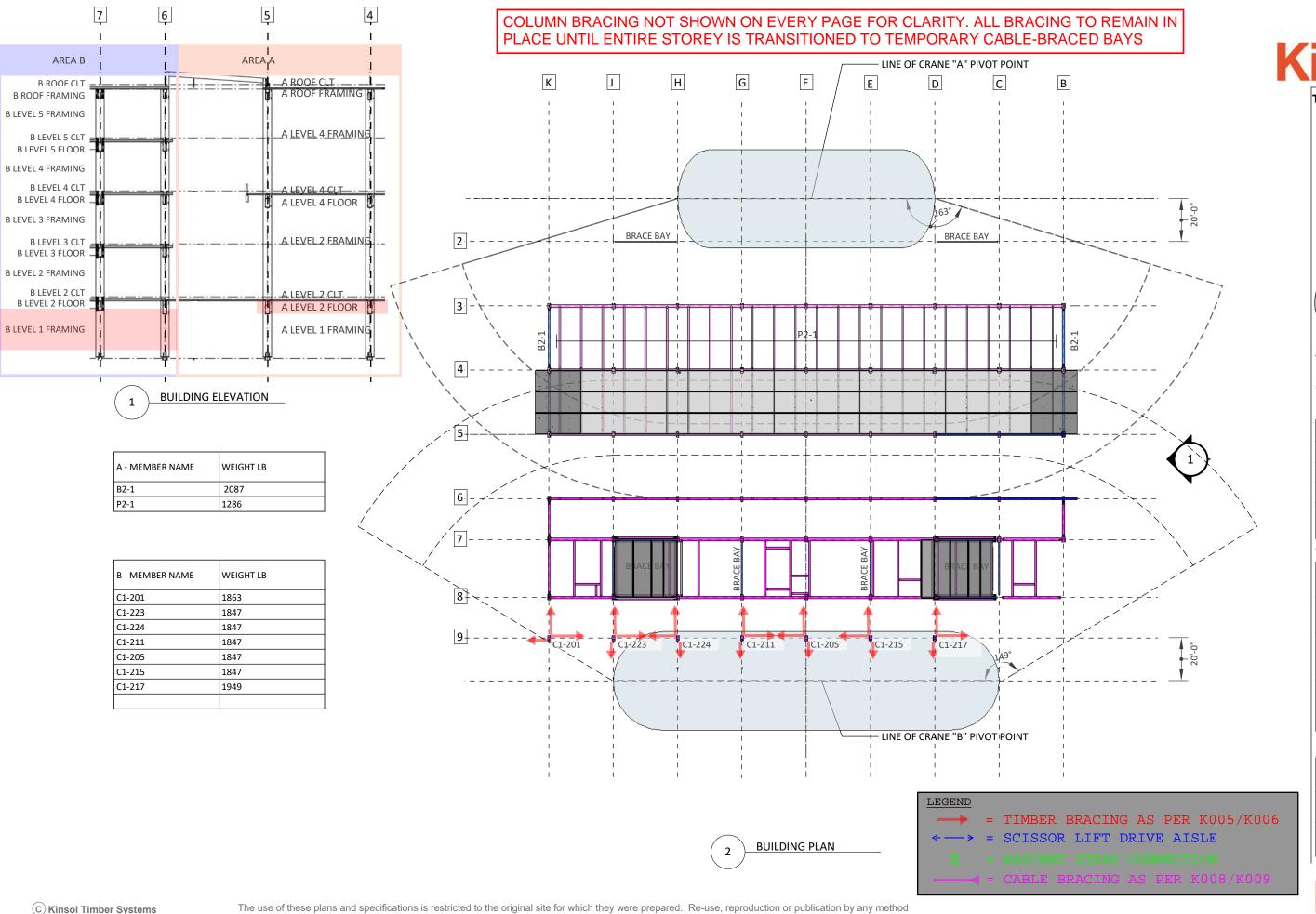


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DATE 09/30/20

DATE 10/06/20



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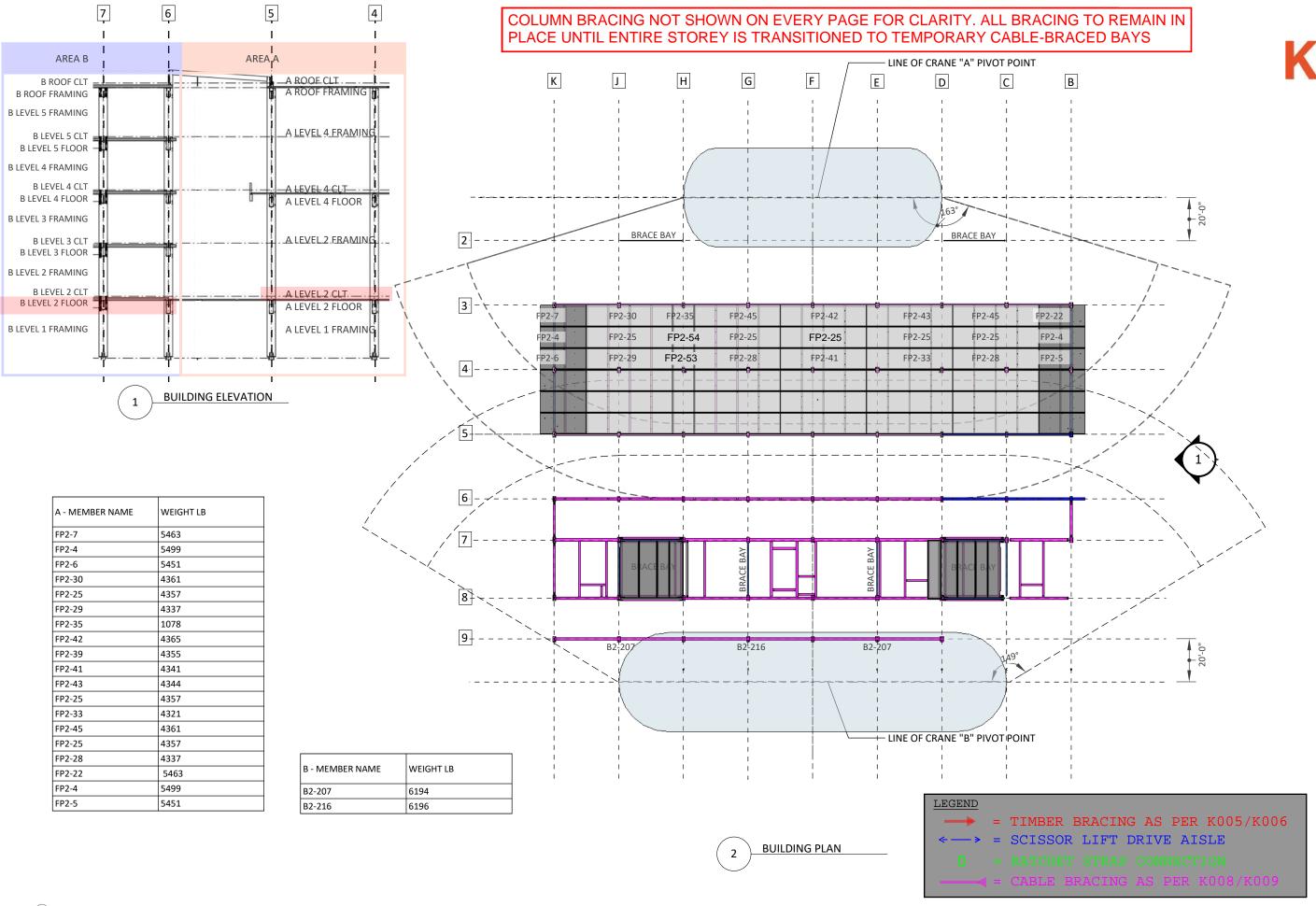
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REV 4	DATE 10/06/20
REV 3	DATE 09/30/20
REV 2	DATE 09/23/20
REV 1	DATE 08/25/20

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REV 3	DATE 09/30/20
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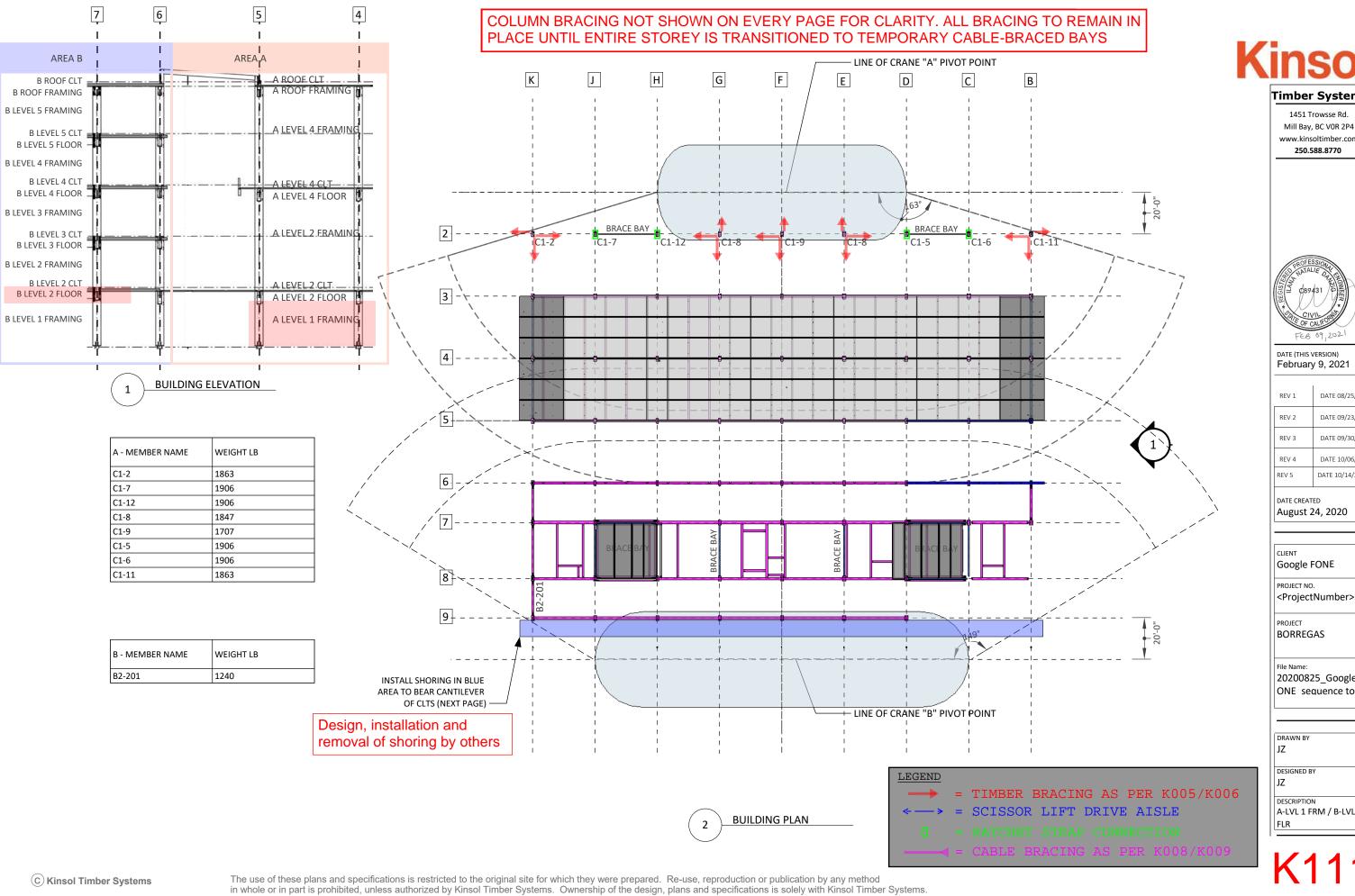
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REV 5	DATE 10/14/20
DATE CREATED	

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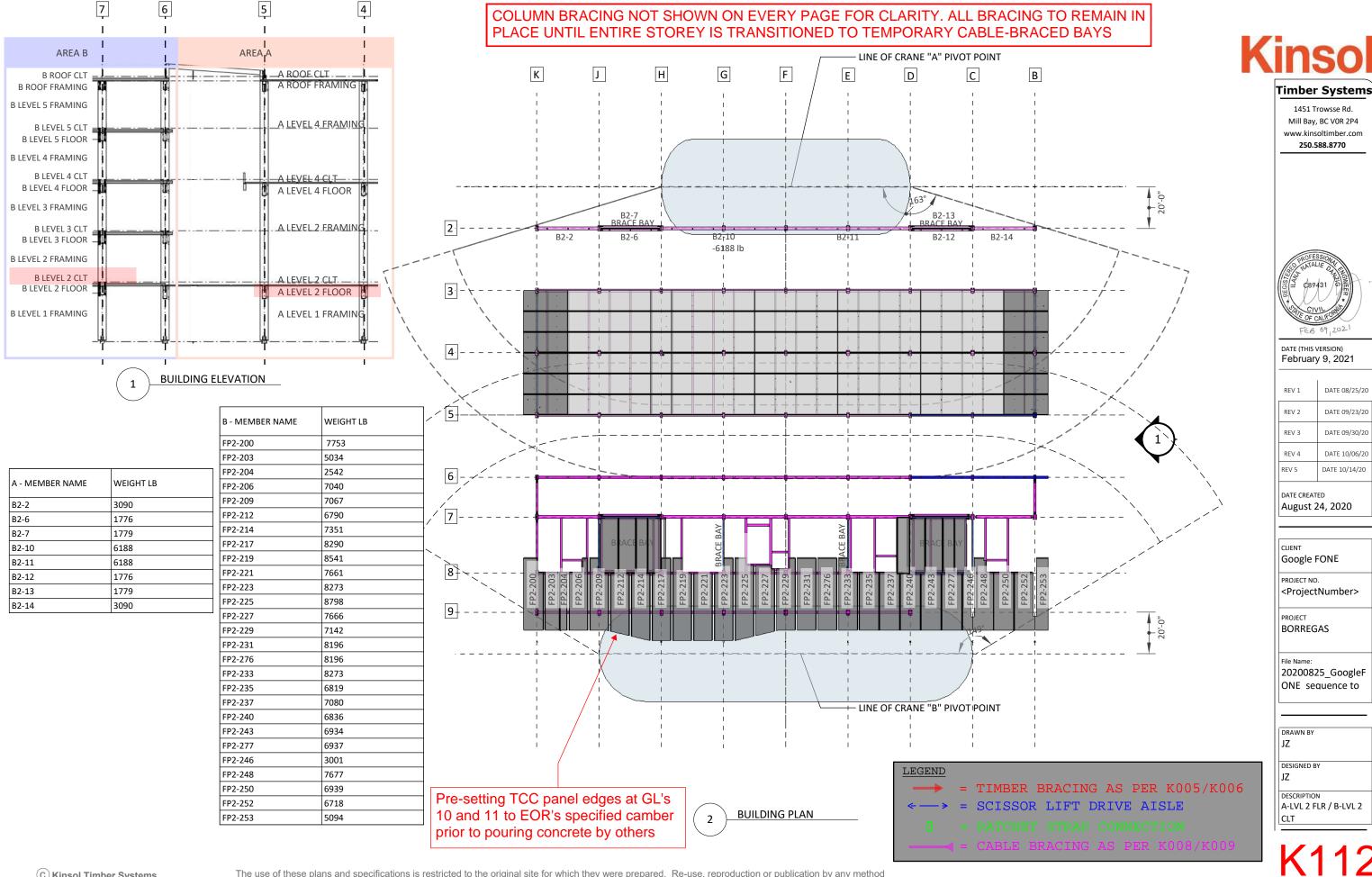
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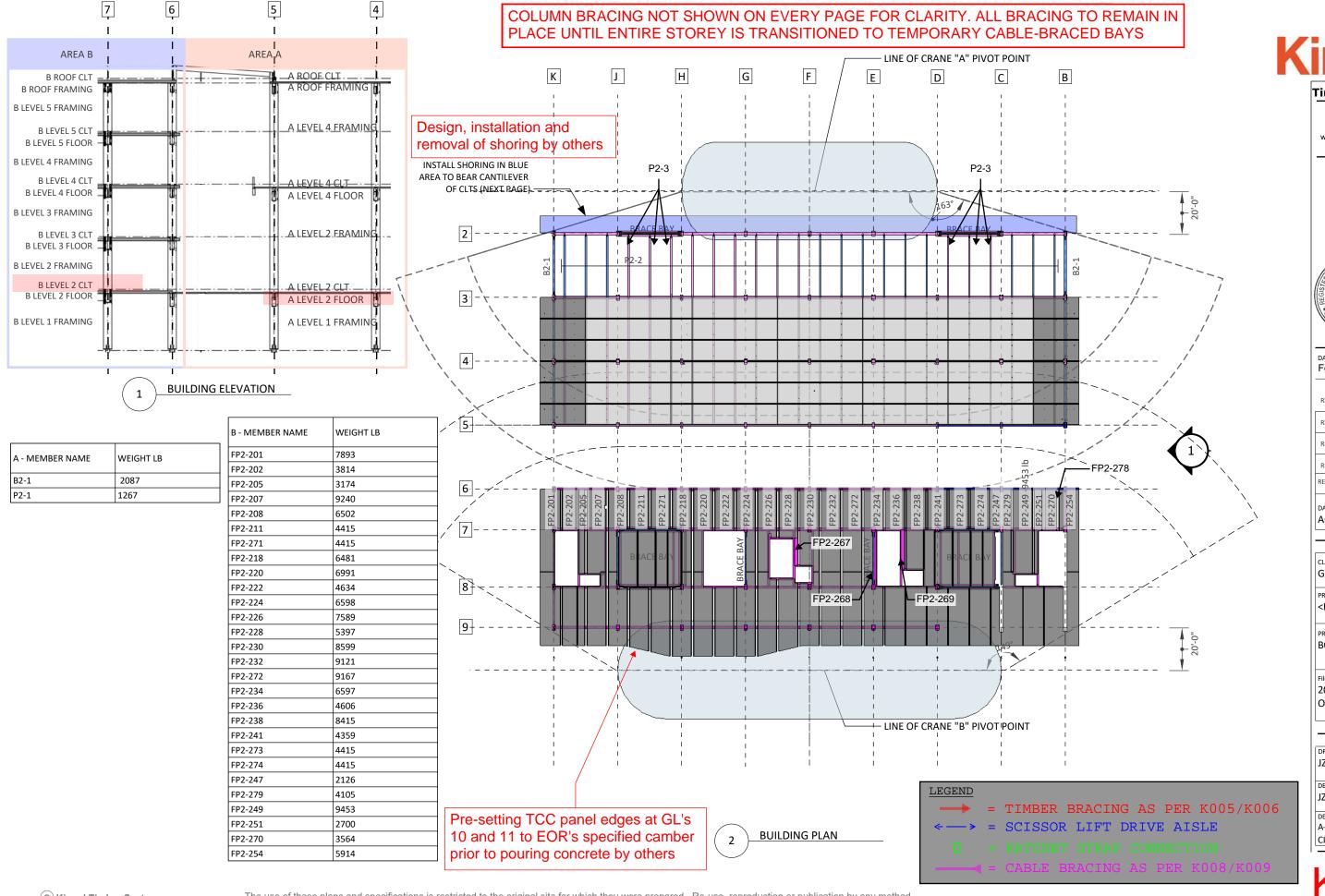
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DATE 10/14/20

REV 1

REV 2

REV 4



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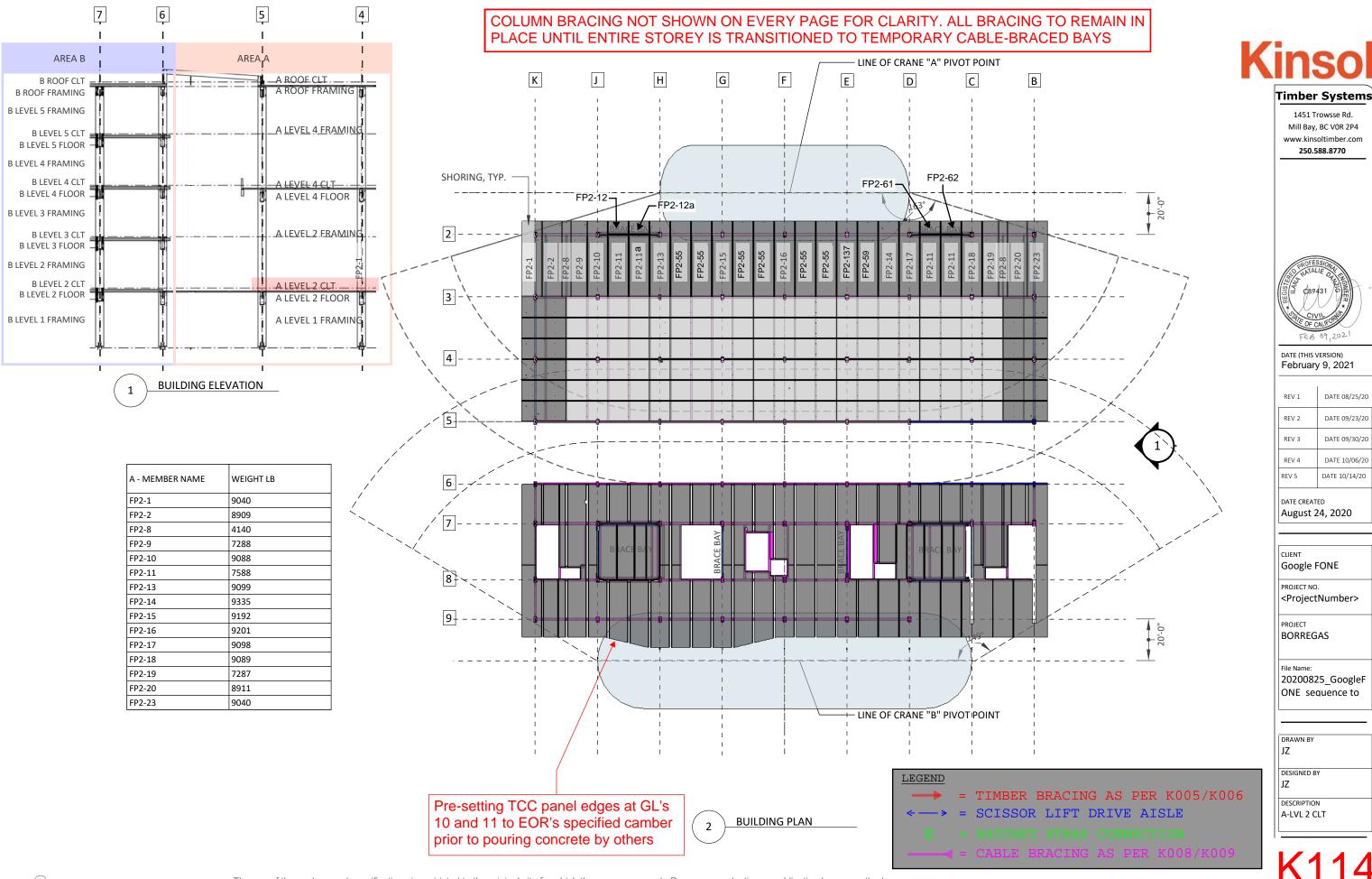
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DESCRIPTION
A-LVL 2 FLR / B-LVL 2

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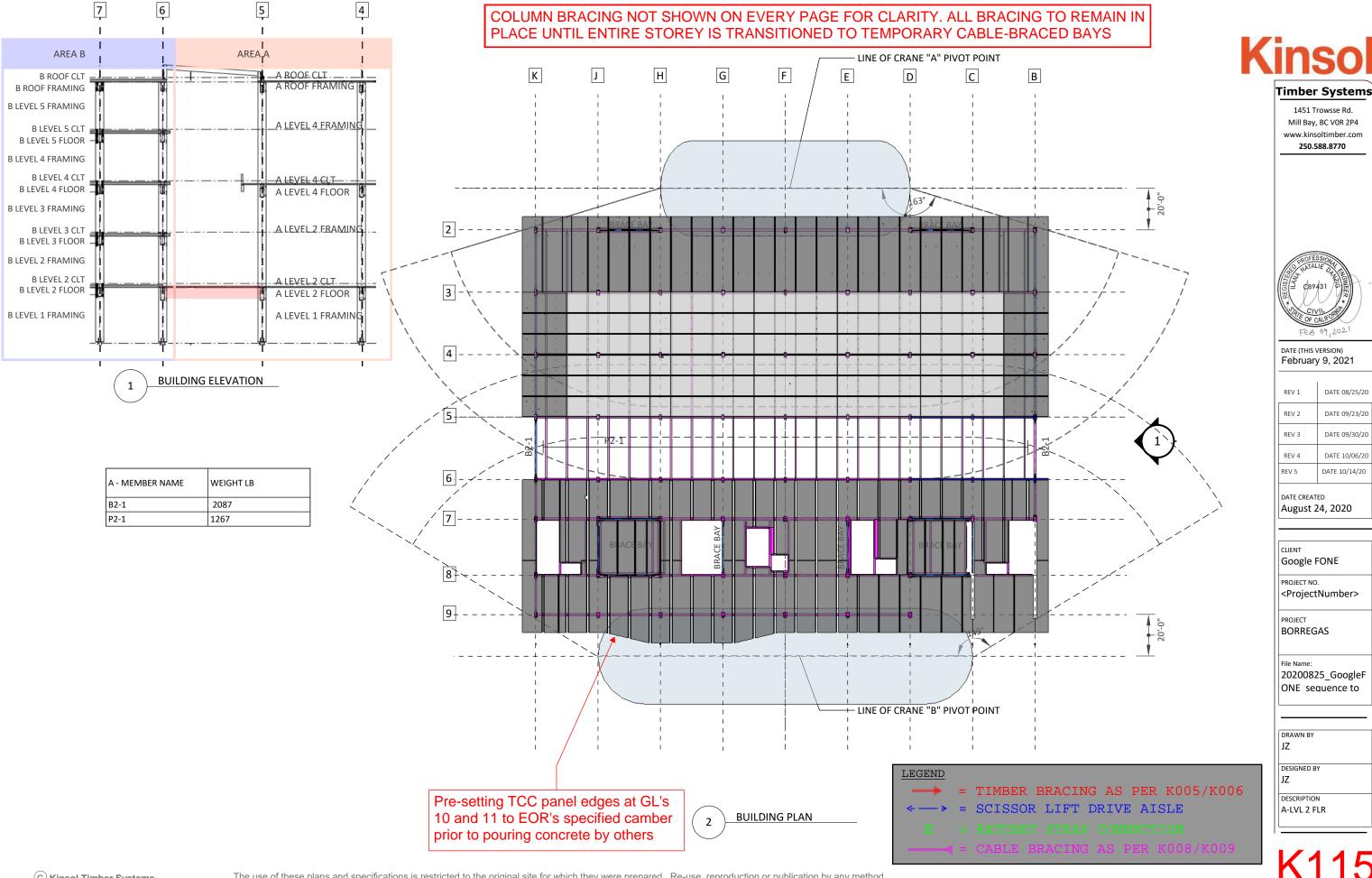


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DATE 09/30/20

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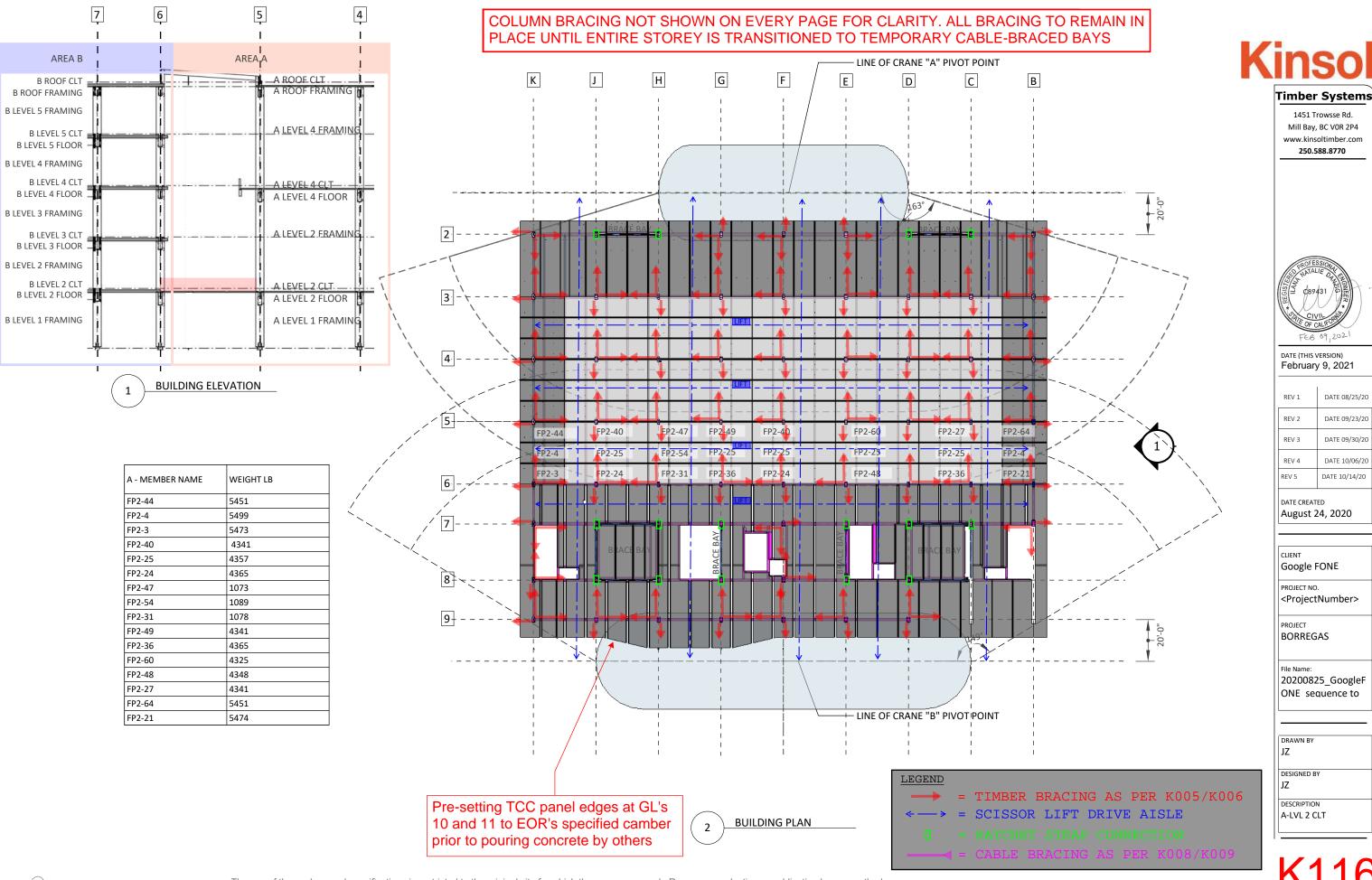
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DATE 09/30/20

DATE 10/06/20

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LOADING PER ASCE 37-14

Table 4-4. Classes of Working Surfaces for Combined Uniformly Distributed Loads

Operational Class	Uniform Load [psf (kN/m²)]
Very Light Duty: sparsely populated with personnel, hand tools, very small amounts of construction materials.	20 (0.96)
^b Light Duty: sparsely populated with personnel, hand-operated equipment, staging of materials for lightweight construction.	25 (1.20)
^b Medium Duty: concentrations of personnel, staging of materials for average construction.	50 (2.40)
bHeavy Duty: material placement by motorized buggies, staging of materials for heavy construction.	75 (3.59)

 aLoads do not include dead load, D; construction dead load, $C_D,$ or fixed material loads, $C_{FML}.$ bOSHA categories.

Examples of construction operations that have traditionally been designed for the loads given in the table are:

Very Light Duty: Roofing, reroofing, excepting situations with stockpiles of ballast

Access catwalks

Painting, caulking

concrete units

Maintenance using hand tools

Light Duty:

Light frame construction

Concrete transport and placement by hose and concrete finishing with hand tools

Medium Duty:

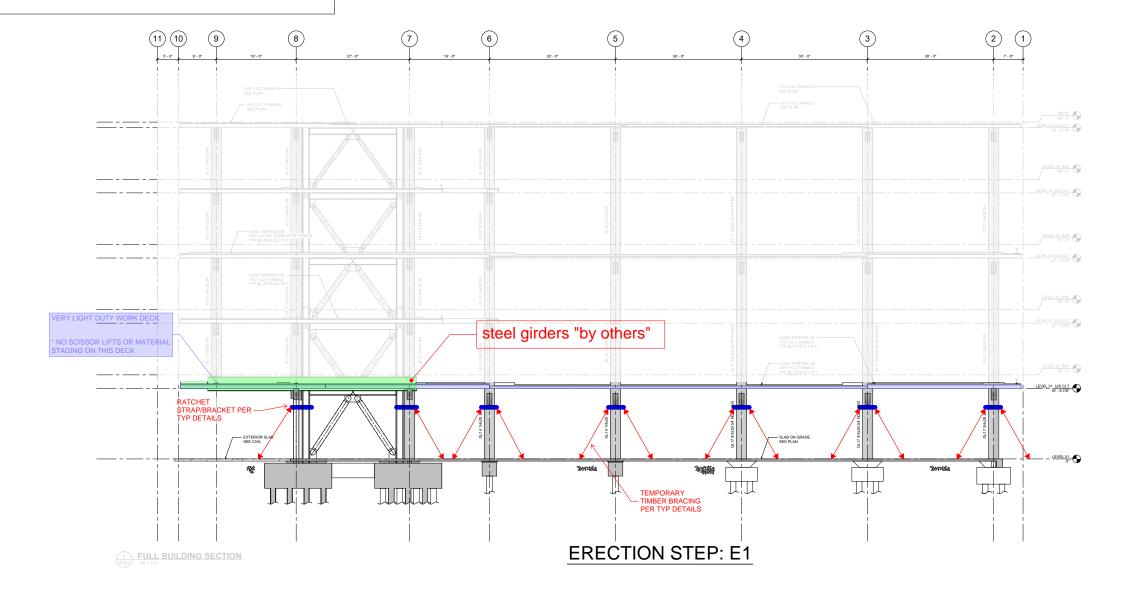
Concrete transport and placement by buckets, chutes, or handcarts

Concrete finishing using motorized screeds
Masonry construction with tile or hollow lightweight

Structural steel erection or concrete reinforcing steel placement

Erection Sequence

Erection Step	Procedure	
E1	E1.1: Place anchors/embeds in slab	
	E1.2: Install steel frame. Brace steel per installer.	
	E1.3: Install timber columns per sequencing drawings. Brace columns per 15' timber brace detail.	
	E1.4: Block out gap between glulam columns & steel brace frame columns. Wrap truck straps around columns per drawings.	
	E1.5: Install level 2 deck. Install all splines and CLT deck connections per structural drawings and temporary strapping plan on level 2.	
	* Note: Level 2 is now considered a "Very Light Duty" deck. No scissor lifts or material staging are permitted on this deck.	



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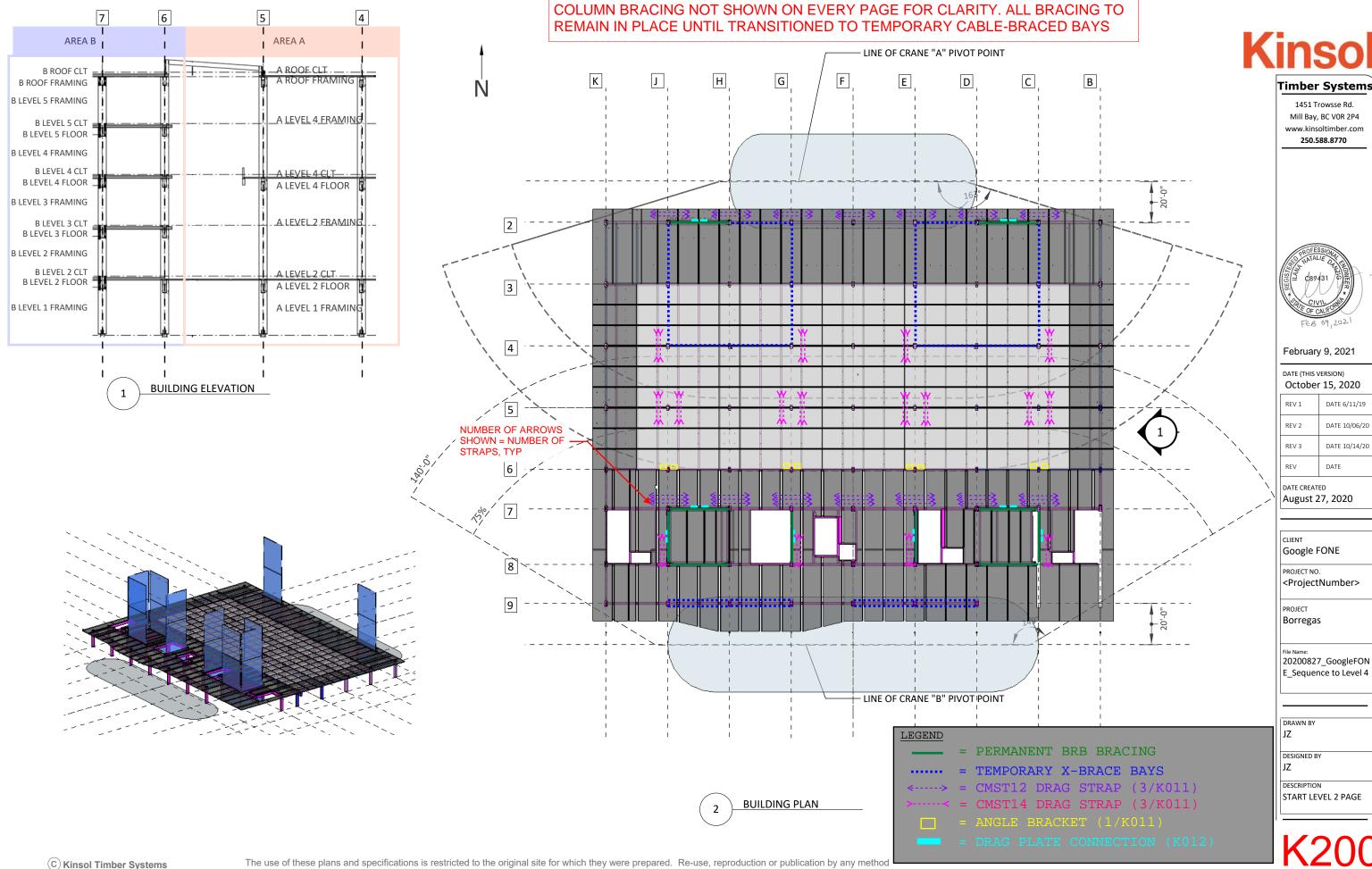
Date:	Revision / Issue:	No
2020-09-22	Issued for Coordination	
2020-09-29	Issued for Coordination	
2020-10-06	Issued for Coordination	
2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	1
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Plot Date

2020-10-19

Erection Step 1 Bracing

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LOADING PER ASCE 37-14

Table 4-4. Classes of Working Surfaces for Combined Uniformly Distributed Loads

Operational Class	Uniform Load ^a [psf (kN/m²)]	Very Light Duty: Roofing, reroofing, excepting situations with stockpiles
Very Light Duty: sparsely populated with personnel, hand tools, very small amounts of construction materials.	20 (0.96)	of ballast Access catwalks Painting, caulking
^b Light Duty: sparsely populated with personnel, hand-operated equipment, staging of materials for lightweight construction.	25 (1.20)	Maintenance using hand tools Light Duty: Light frame construction
^b Medium Duty: concentrations of personnel, staging of materials for average construction.	50 (2.40)	Concrete transport and placement by hose and concrete finishing with hand tools
^b Heavy Duty: material placement by motorized buggies, staging of materials for heavy construction.	75 (3.59)	Medium Duty: Concrete transport and placement by buckets, chutes, or
$^{8}\mathrm{Loads}$ do not include dead load, D; construction dead load, $C_{D},$ or fixed material loads, $C_{FML}.$ $^{8}\mathrm{DSHA}$ categories.		handcarts Concrete finishing using motorized screeds Masonry construction with tile or hollow lightweigh concrete units

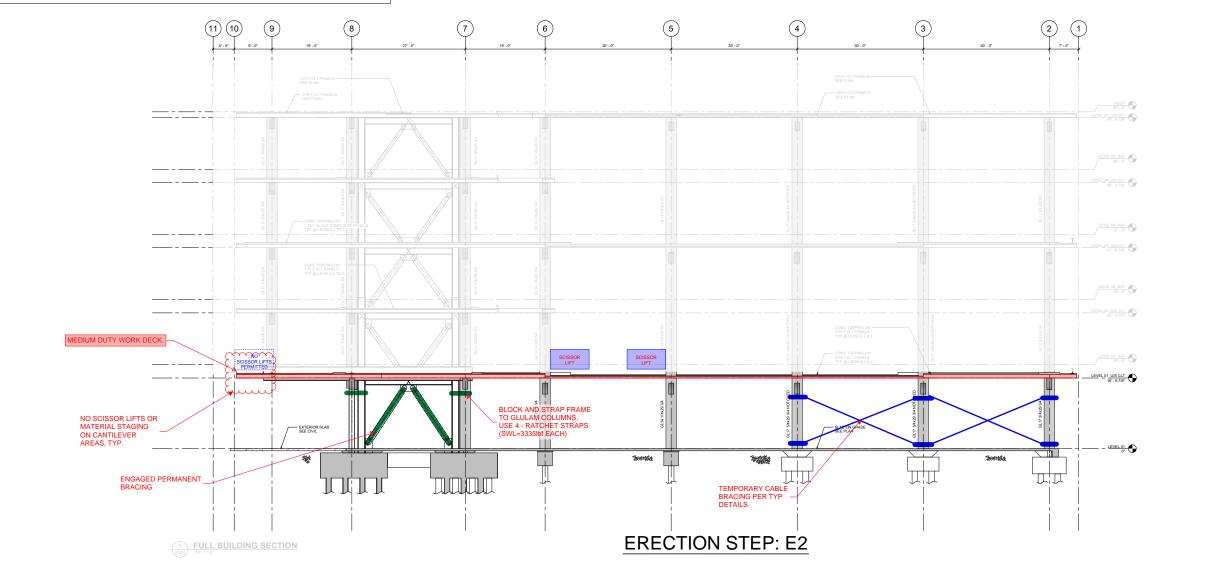
placement

Examples of construction operations that have traditionally been designed for the loads given in the table are:

Structural steel erection or concrete reinforcing steel

Erection Sequence

Procedure	
E2.1: Replace level 1 timber temporary bracing with cable bracing per typical detail.	
* Note: Level 2 deck can now be classified as "Medium Duty". Therefore, scissor lifts can now be driven on this deck and material can be staged.	



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2021-02-09	Issued for Construction	2

Plot Date:

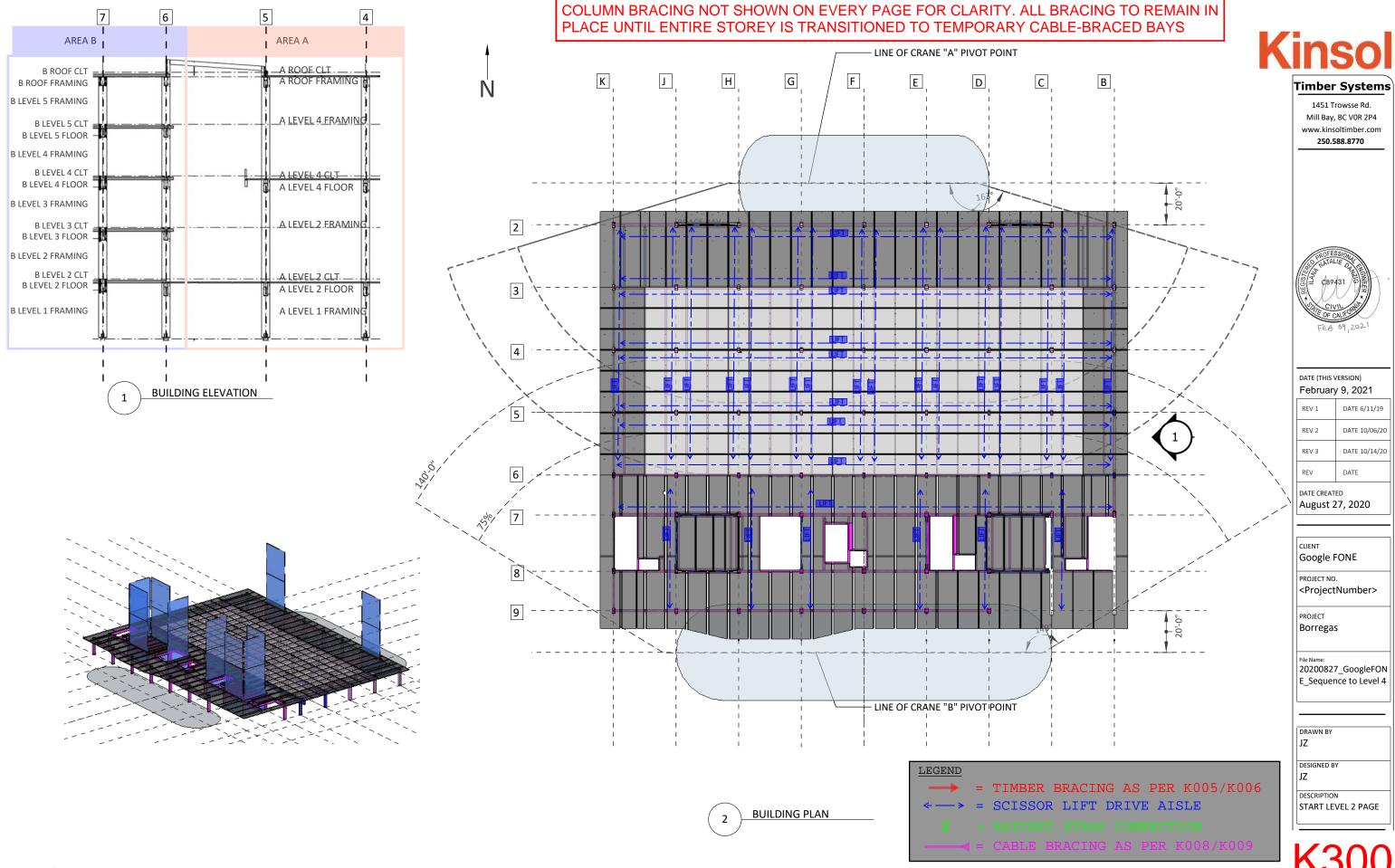
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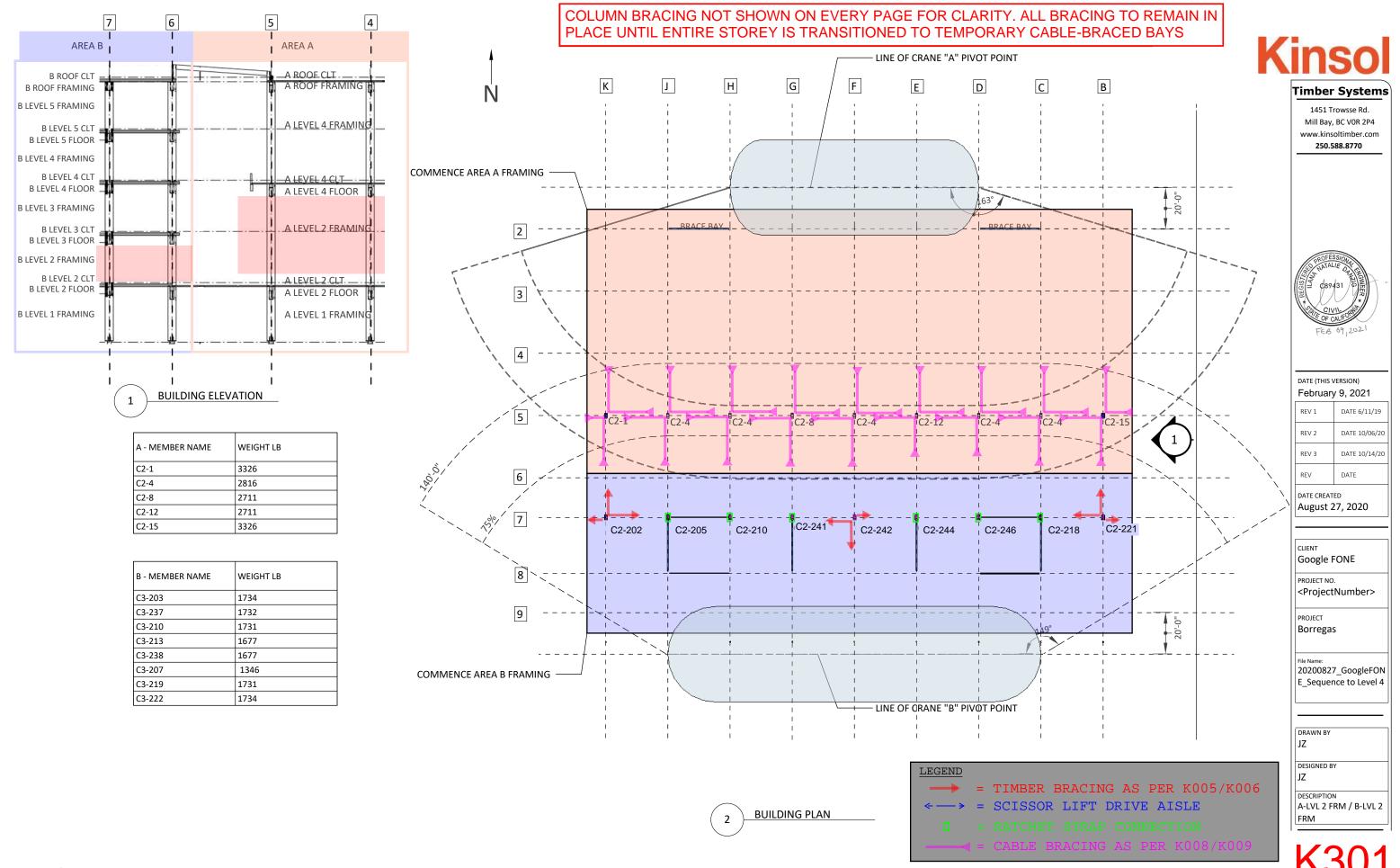
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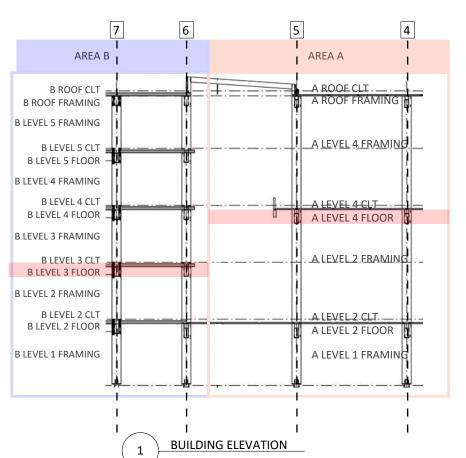
Erection Step 2 Bracing

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Prawing No.: Revision No.:







COLUMN BRACING NOT SHOWN ON EVERY PAGE FOR CLARITY. ALL BRACING TO REMAIN IN
PLACE UNTIL ENTIRE STOREY IS TRANSITIONED TO TEMPORARY CABLE-BRACED BAYS

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BRACE BAY

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В

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REV	DATE

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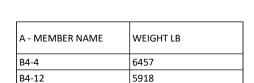
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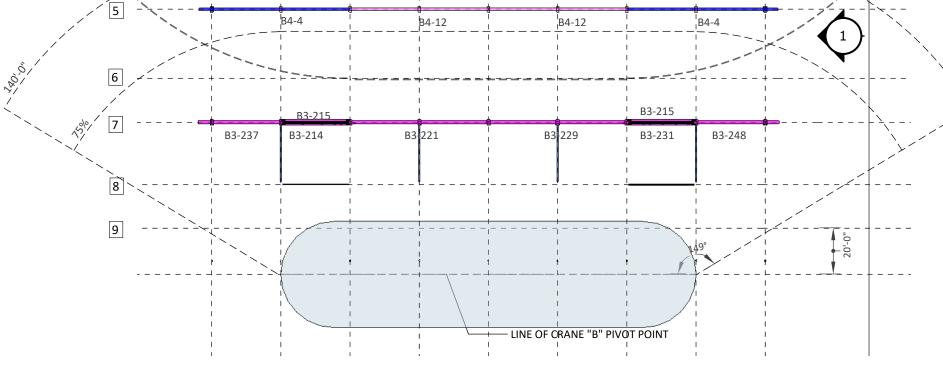
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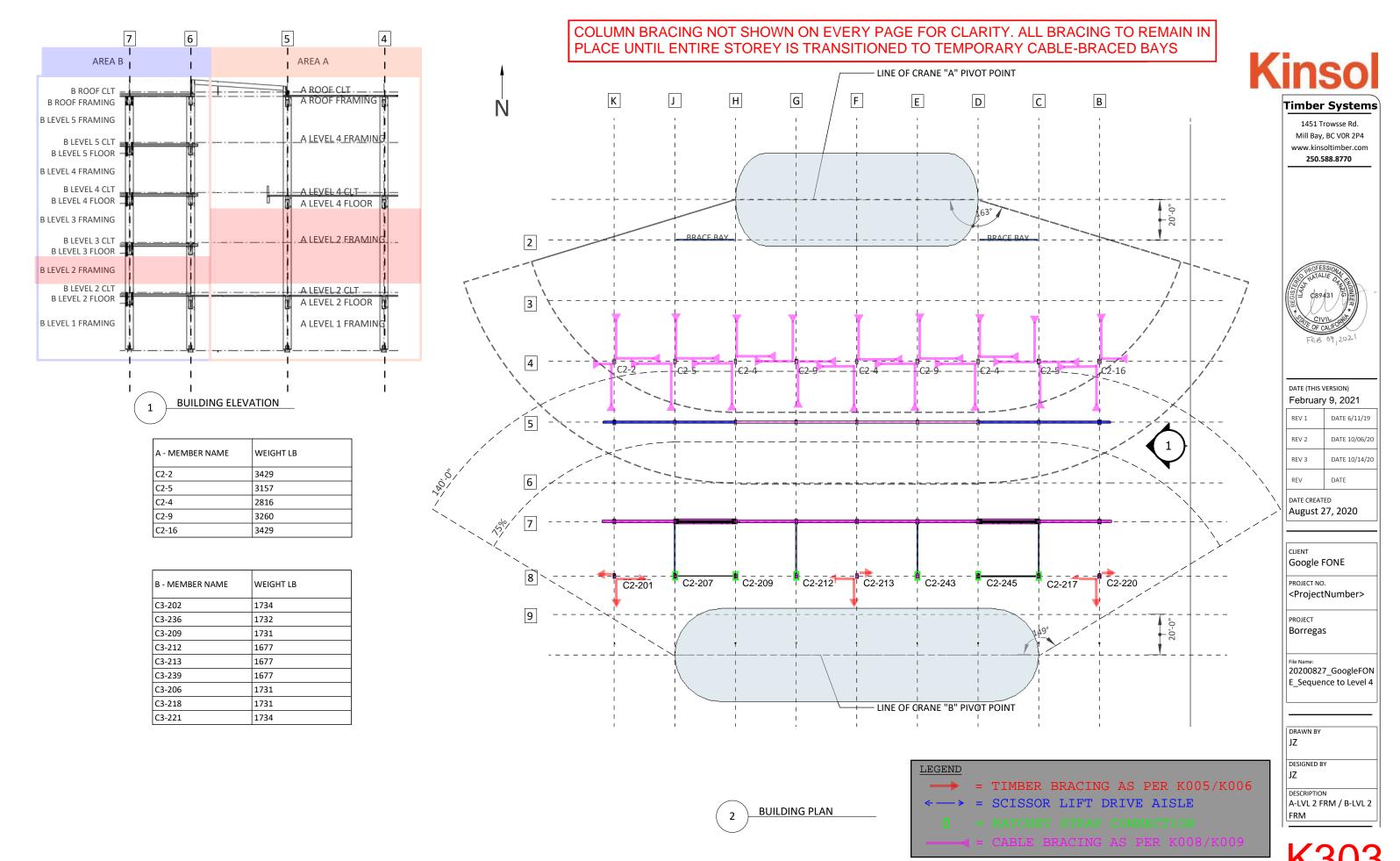


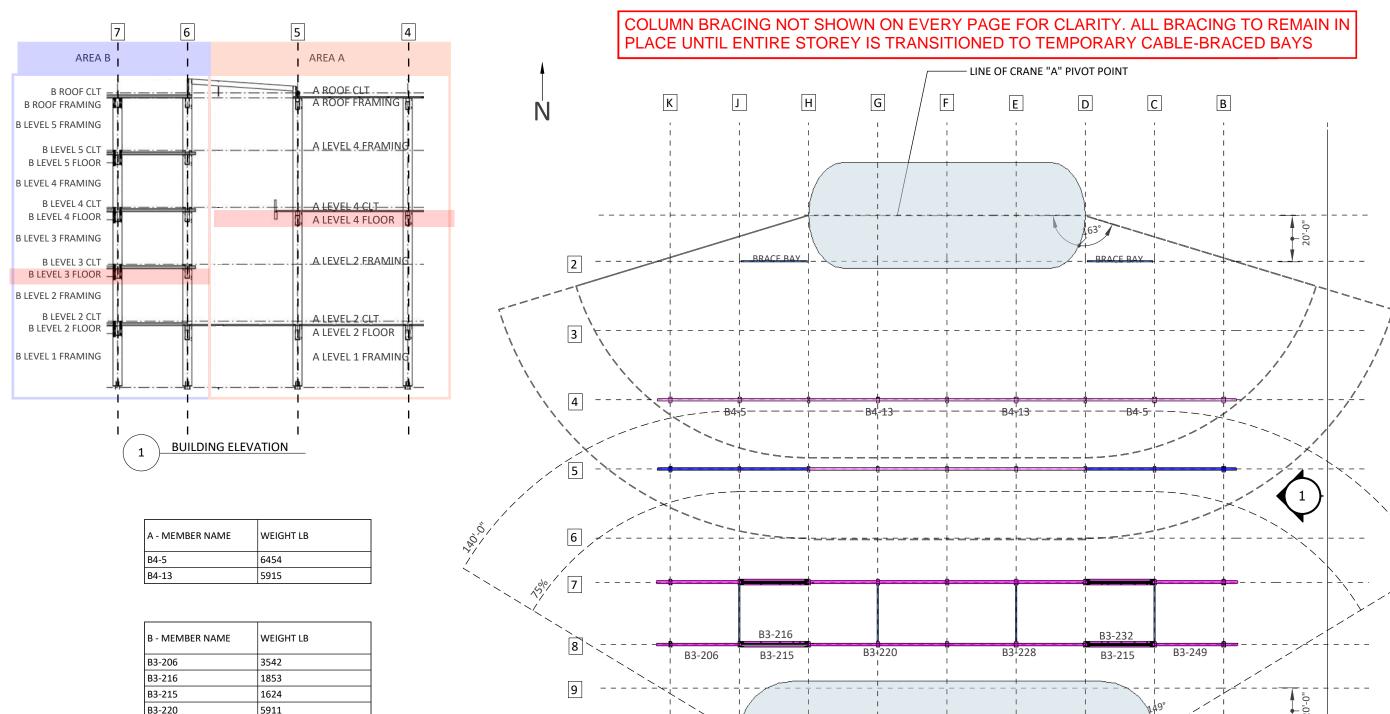
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B3-237	3542
B3-215	1624
B3-214	1853
B3-221	5911
B3-229	5910
B3-215	1624
B3-231	1853
B3-248	3542



BUILDING PLAN

LEGEND





2 BUILDING PLAN

LINE OF ORANE "B" PIVOT POINT

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A-LVL 4 FLR / B-LVL 3

K304

B3-228

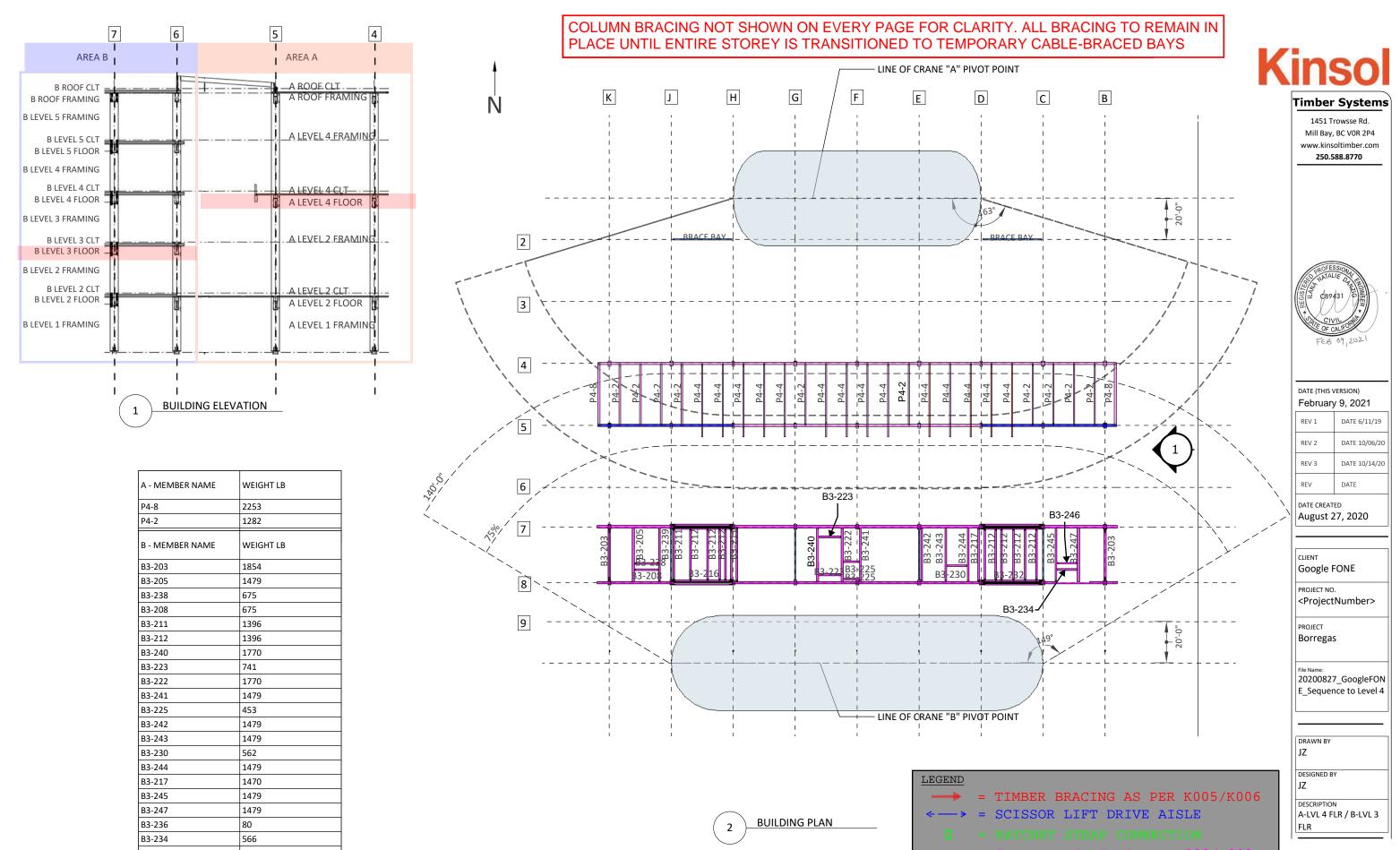
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B3-249

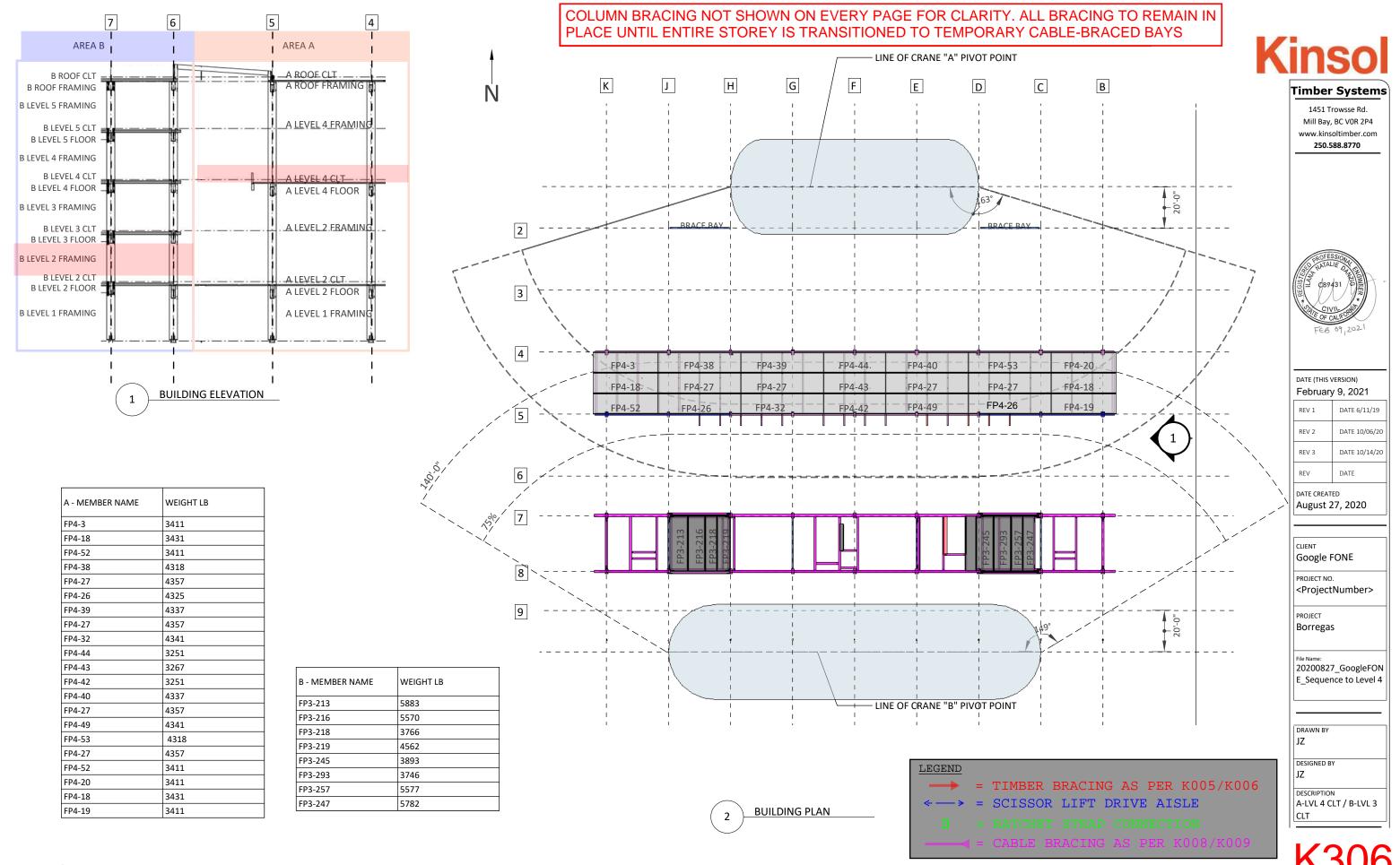
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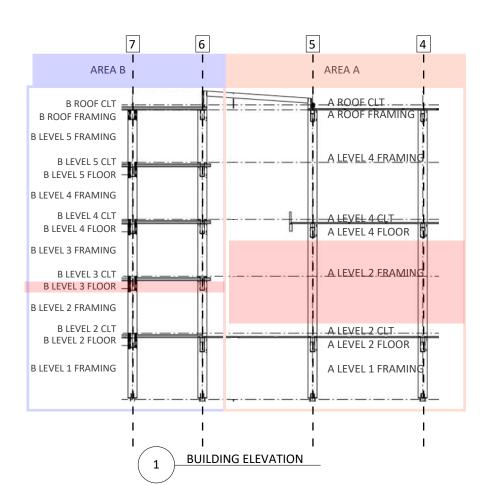
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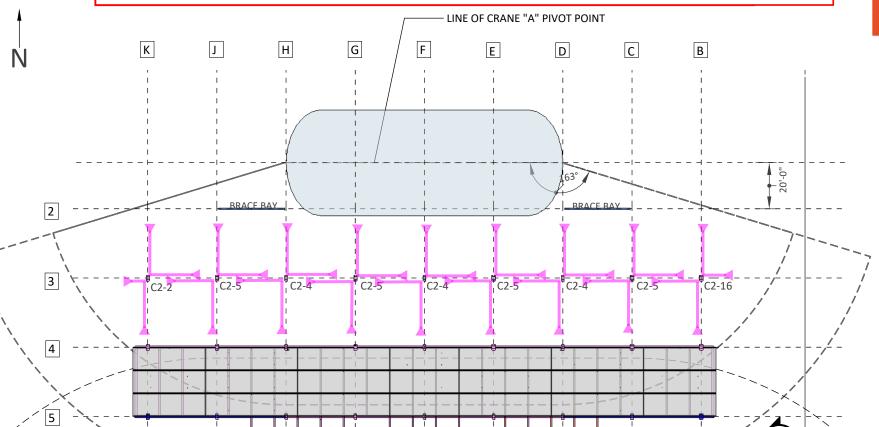




A - MEMBER NAME	WEIGHT LB
C2-2	3429
C2-5	3157
C2-4	2816
C2-16	3429

B - MEMBER NAME	WEIGHT LB
C3-204	1332
C3-225	1346
C3-226	1346
C3-214	1297
C3-217	1297
C3-208	1346
C3-223	1332

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C2-214 TO BE INSTALLED LATER

C2-208

C2-224

LEGEND

C2-208 8 9 LINE OF CRANE "B" PIVOT POINT

C2-224

BUILDING PLAN

= TIMBER BRACING AS PER K005/K006 SCISSOR LIFT DRIVE AISLE

C2-222

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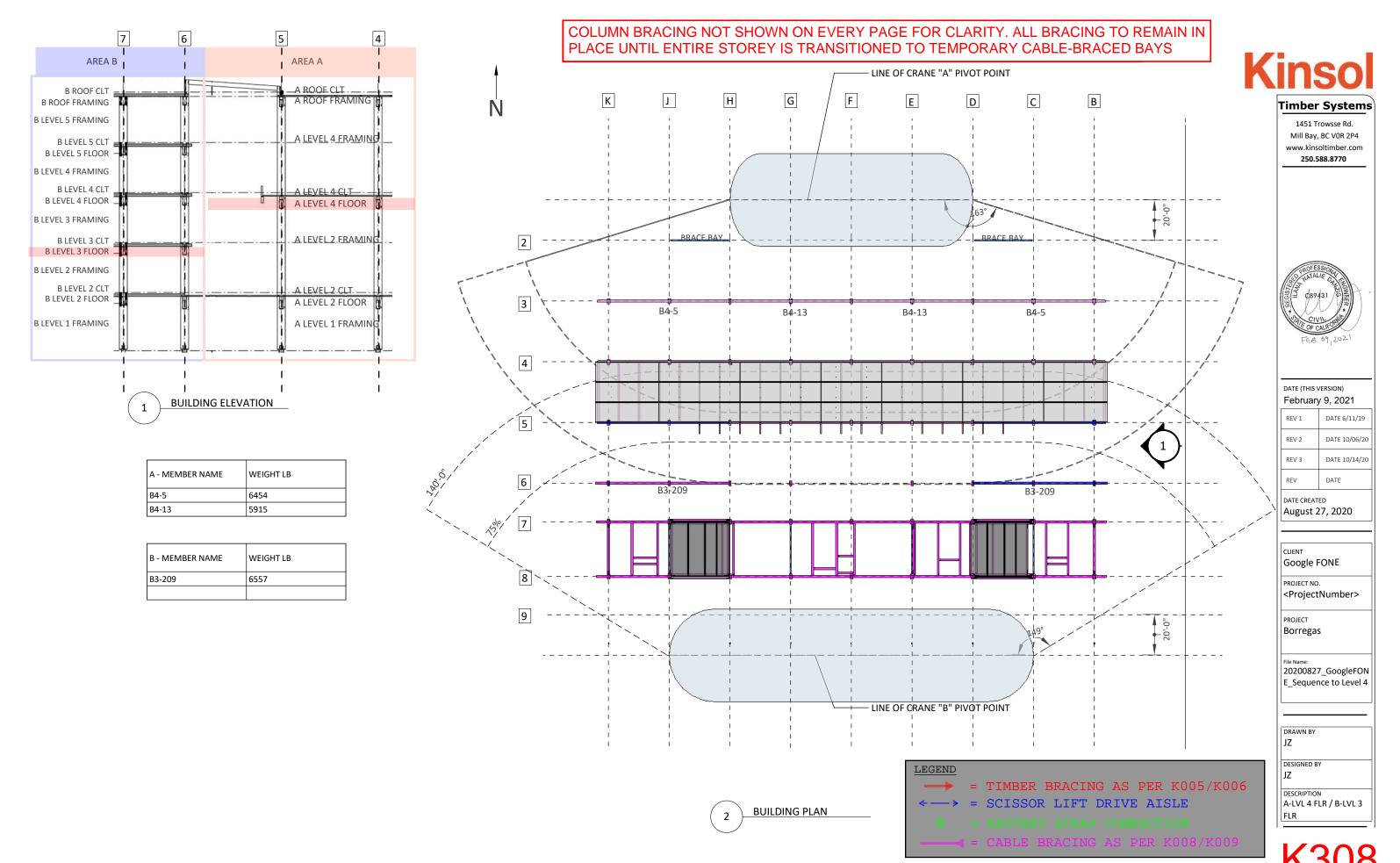
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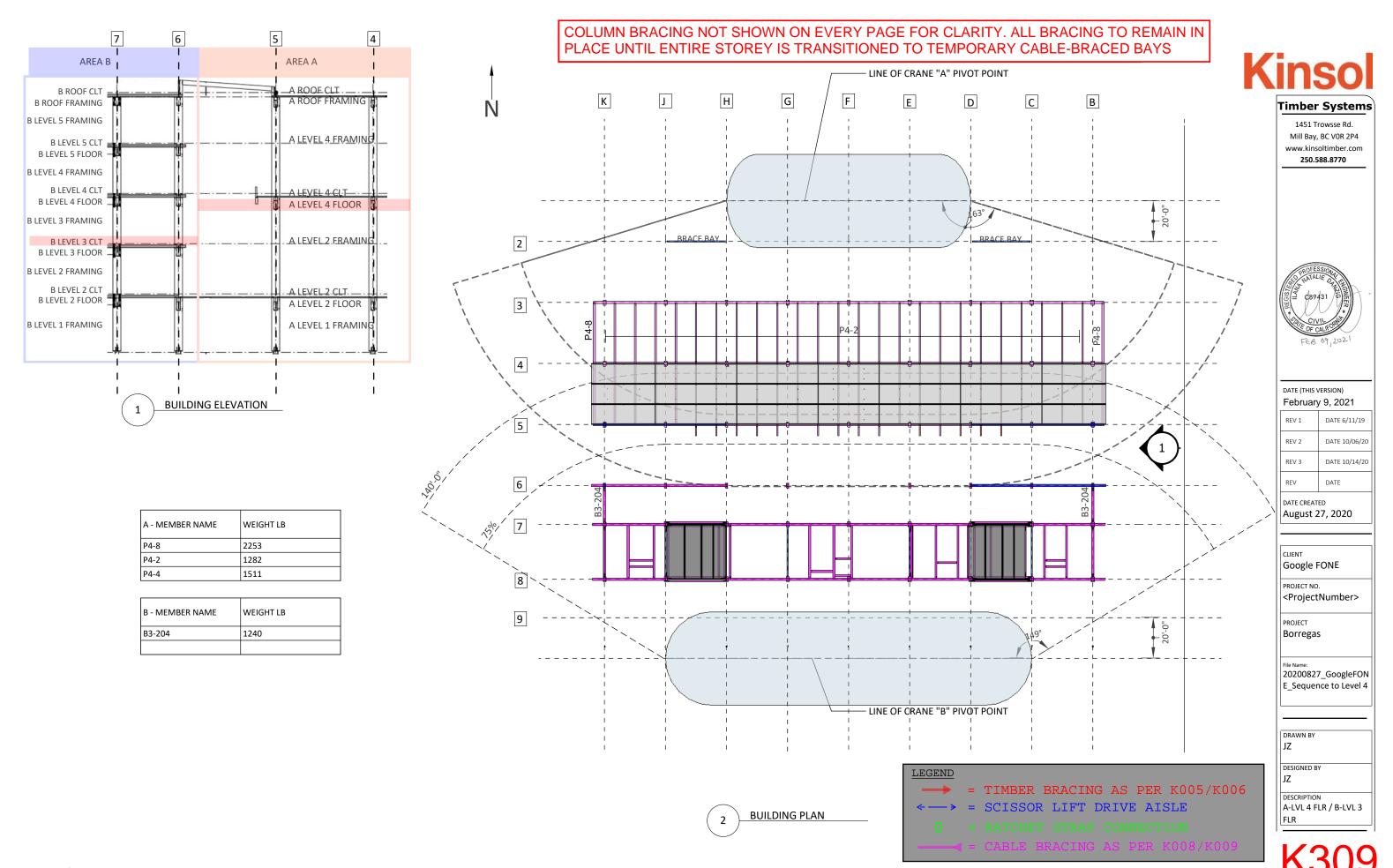
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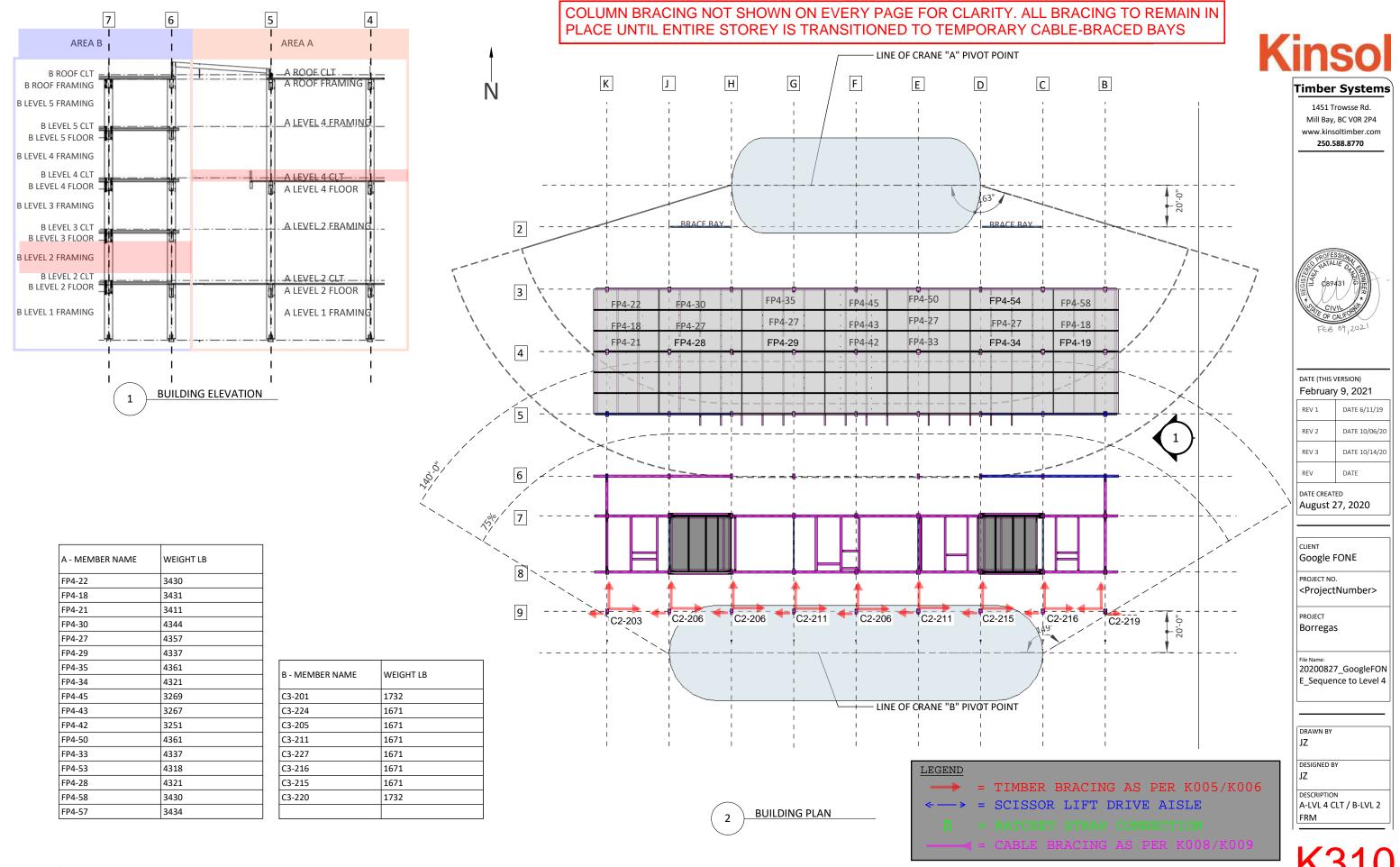
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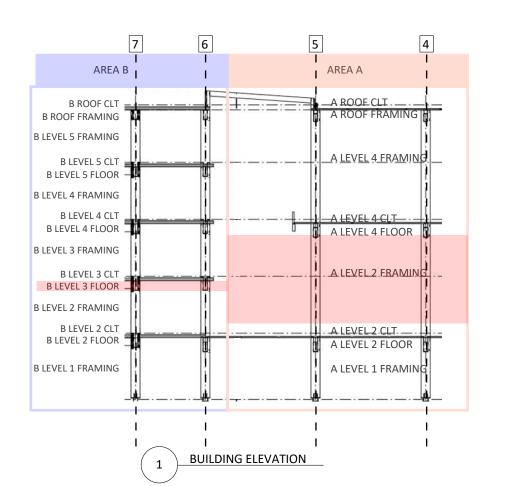
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C2-224





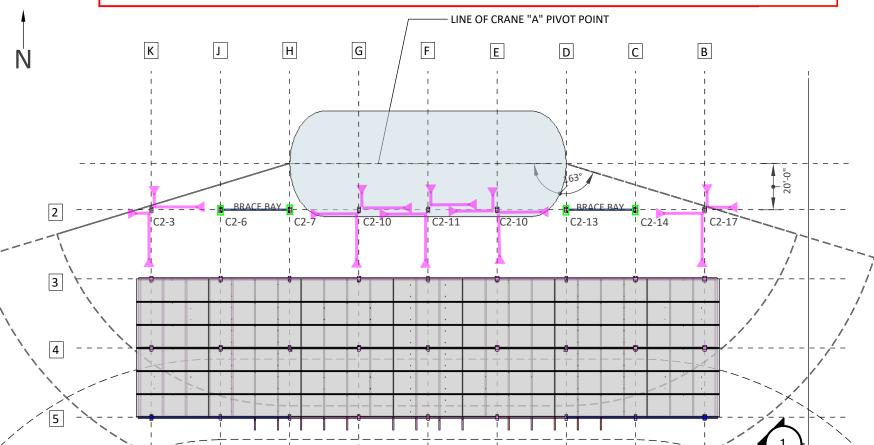




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C2-3	3422
C2-6	3433
C2-7	3433
C2-10	3277
C2-11	3137
C2-13	3433
C2-14	3433
C2-17	3422

B - MEMBER NAME	WEIGHT LB
B3-210	6813
B3-219	6196

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9 B3-210 B3-219 B3-210 B3-210

LEGEND

BUILDING PLAN

= CABLE BRACING AS PER K008/K009

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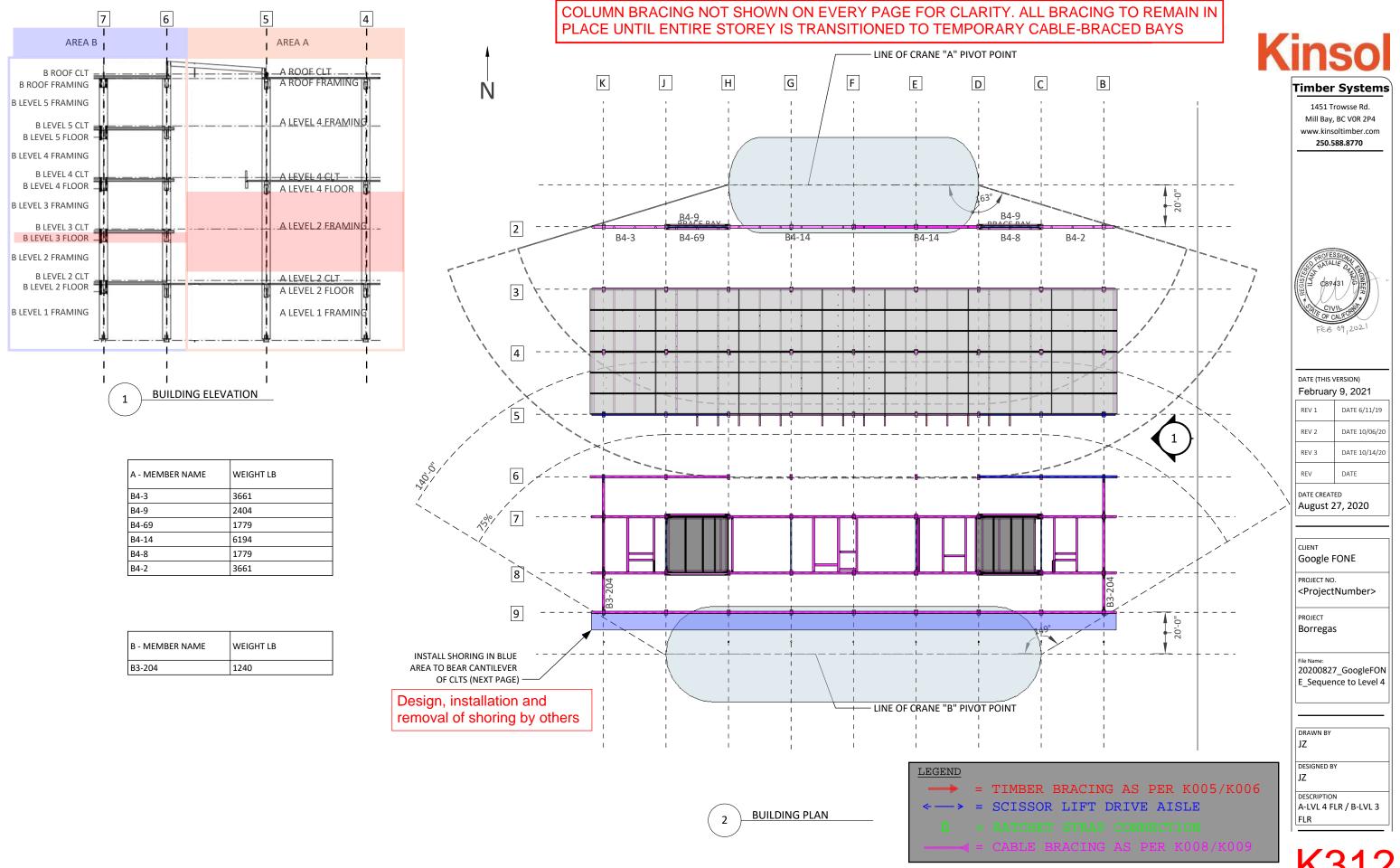
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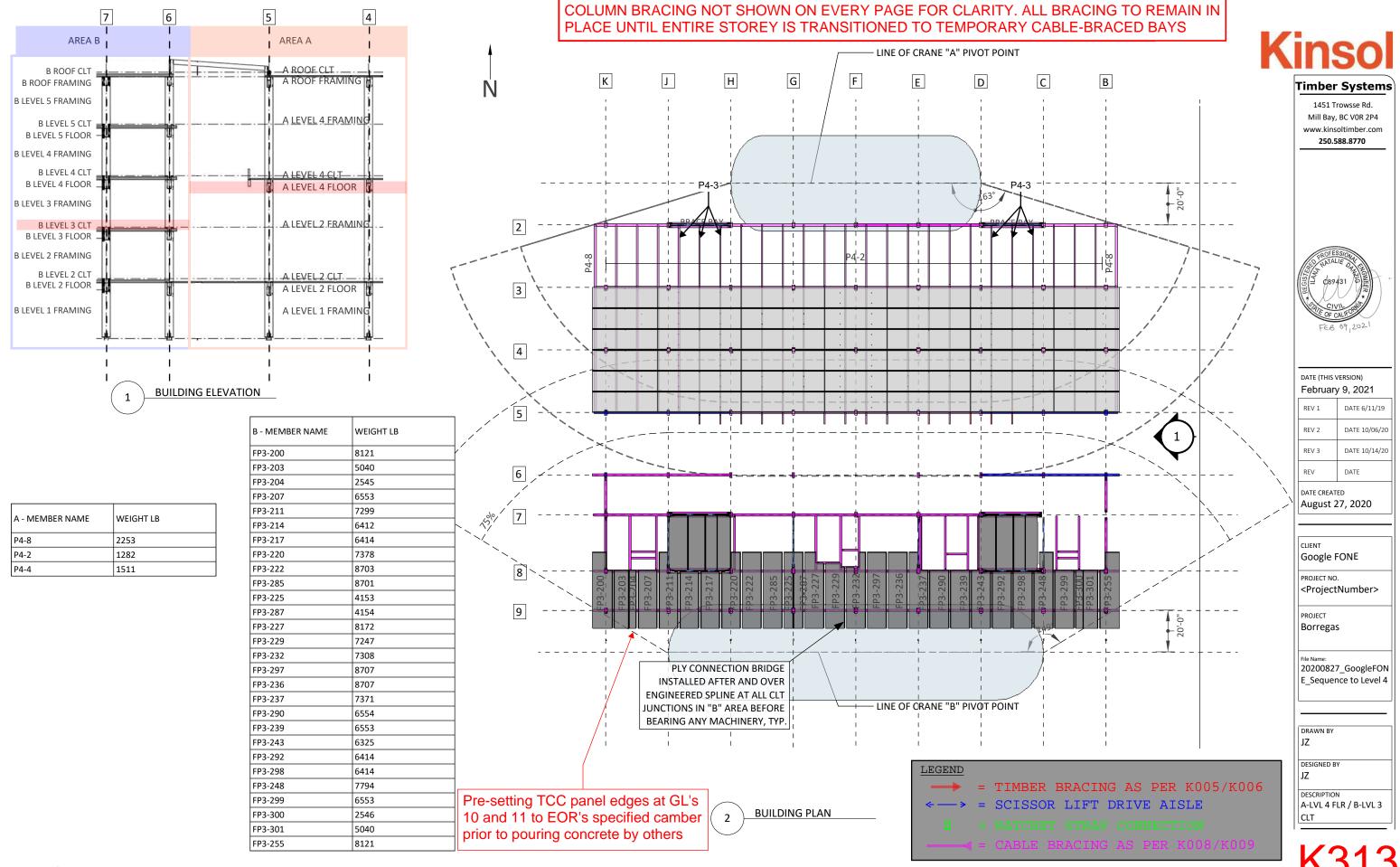
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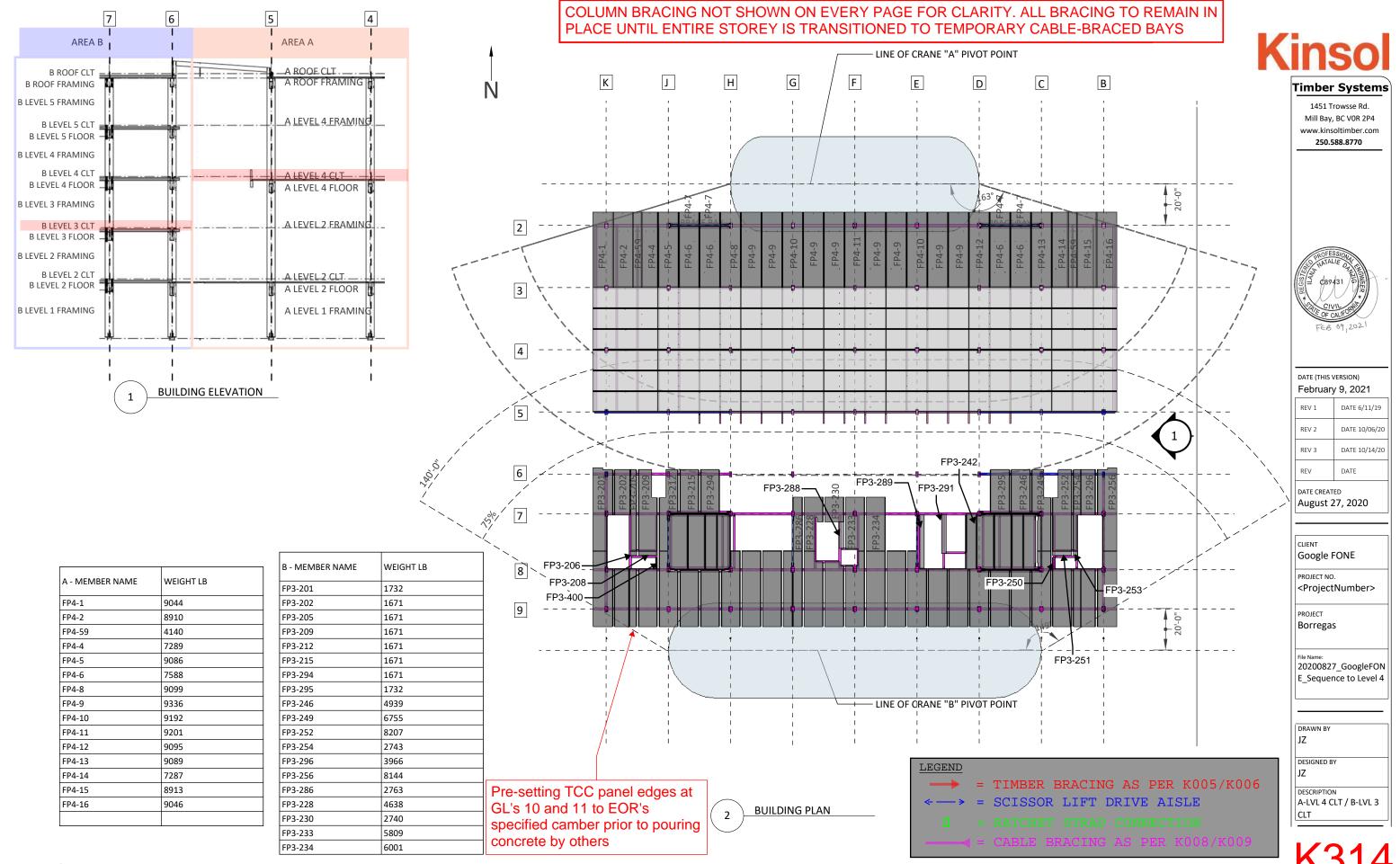
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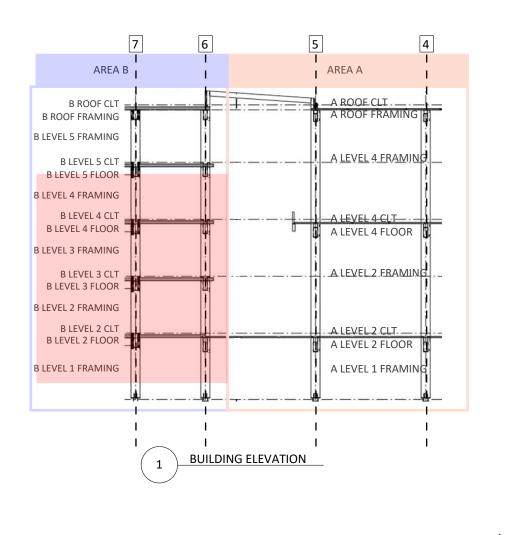
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K311





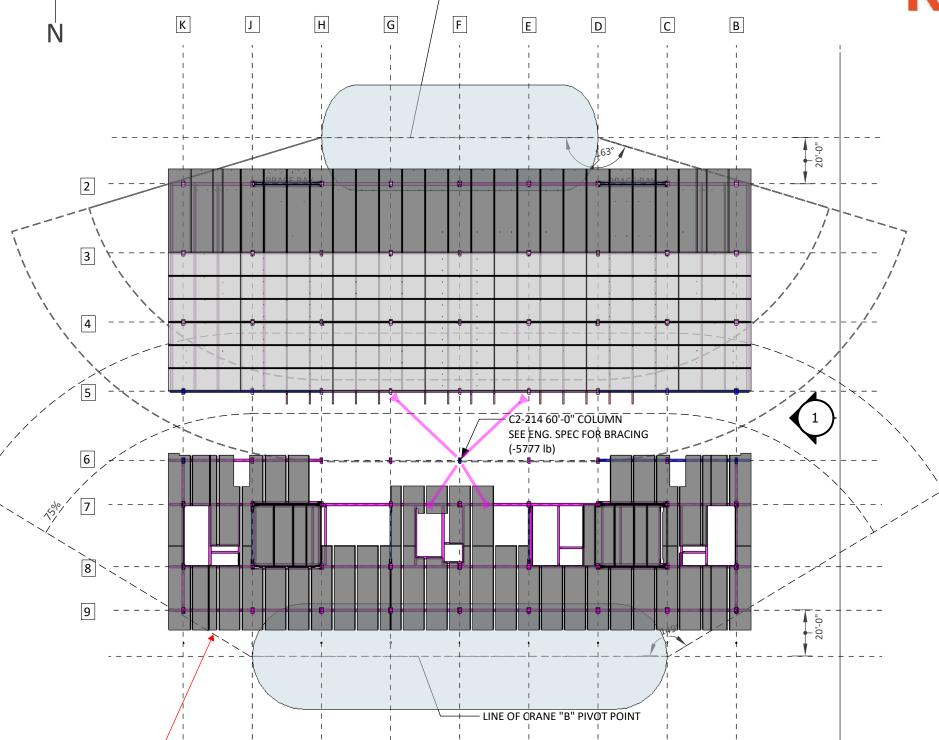




B - MEMBER NAME	WEIGHT LB
C2-214	5777

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- LINE OF CRANE "A" PIVOT POINT



Pre-setting TCC panel edges at GL's 10 and 11 to EOR's specified camber prior to pouring concrete by others

BUILDING PLAN

= TIMBER BRACING AS PER K005/K006

----> = SCISSOR LIFT DRIVE AISLE

LEGEND

= RATCHET STRAP CONNECTION

= CABLE BRACING AS PER K008/K009

K315

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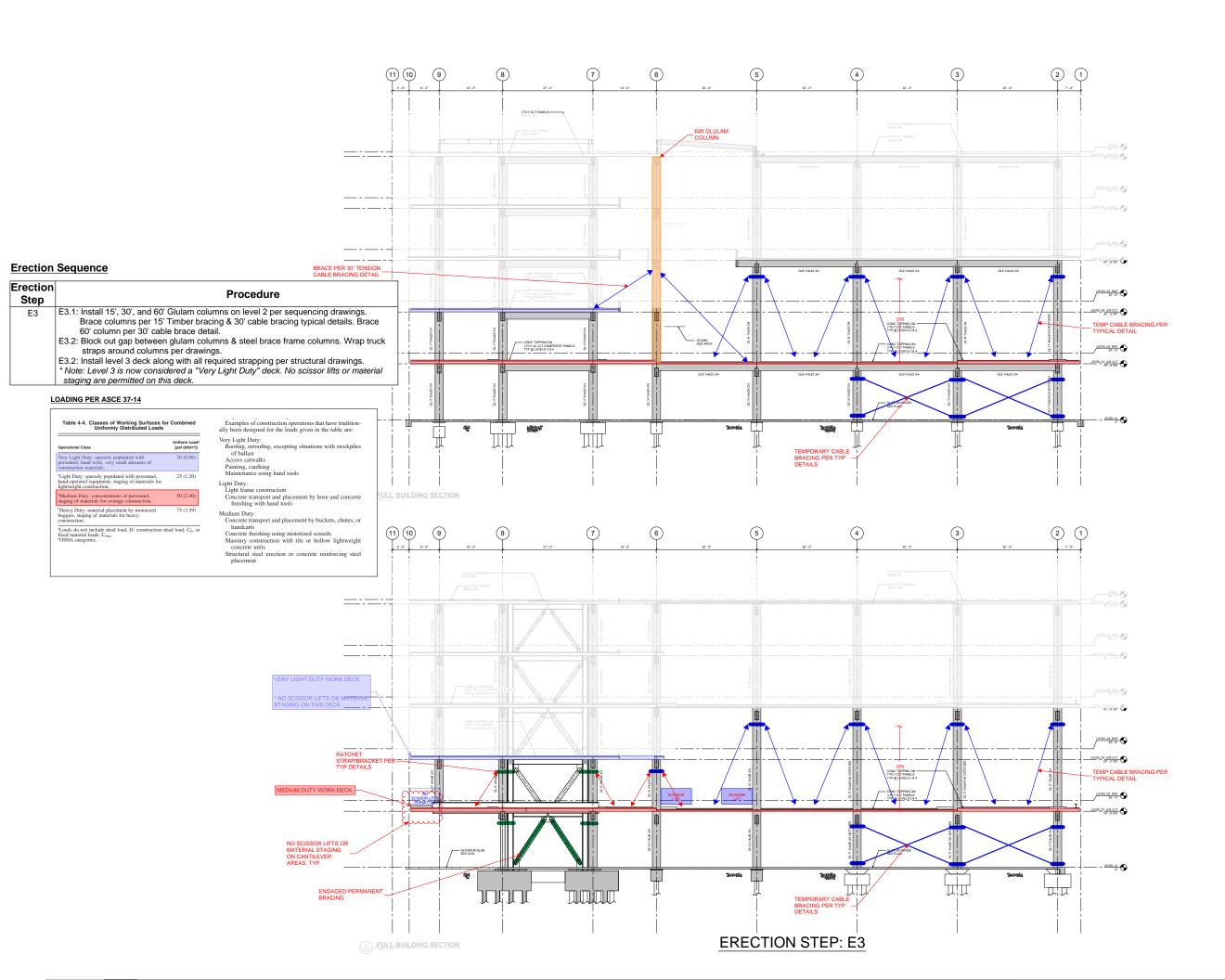
B-LVL 1-4 COLUMN

DATE 6/11/19

DATE 10/06/20

DATE 10/14/20

DATE



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ASPECT STRUCTURAL ENGINEERS

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2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	1
2021-02-09	Issued for Construction	2

Plot Date:

2020-10-19

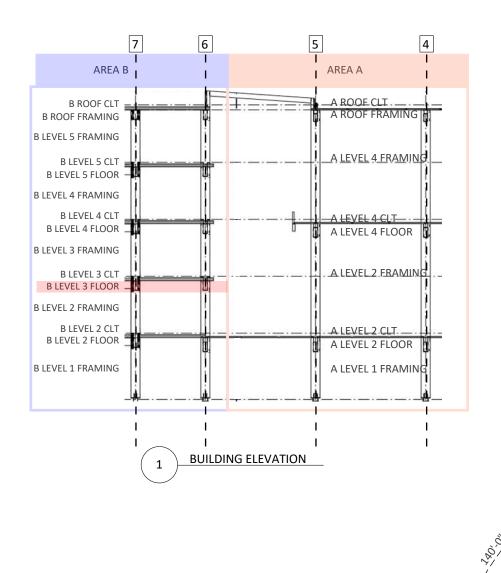
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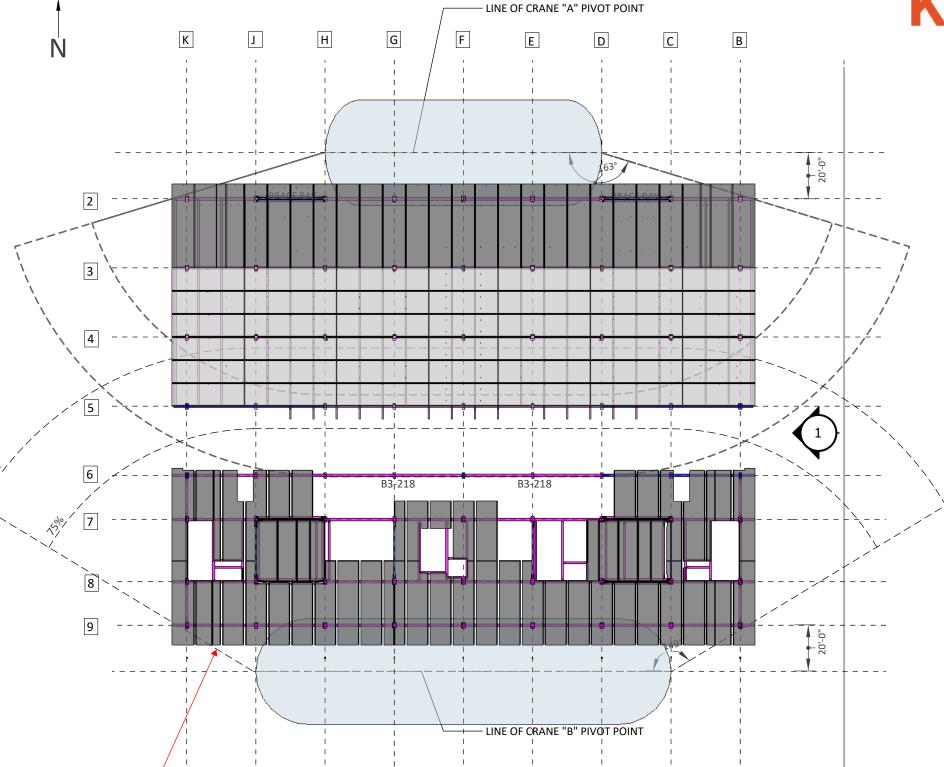
awing No.: Revision No.

K316



A - MEMBER NAME	WEIGHT LB	
B3-218	5850	





BUILDING PLAN

2

<u>LEGEND</u>

= TIMBER BRACING AS PER K005/K006

—→ = SCISSOR LIFT DRIVE AISLE

Π = RATCHET STRAP CONNECTION

= CABLE BRACING AS PER K008/K009

K317

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B-LVL 3 FLR

DATE 6/11/19

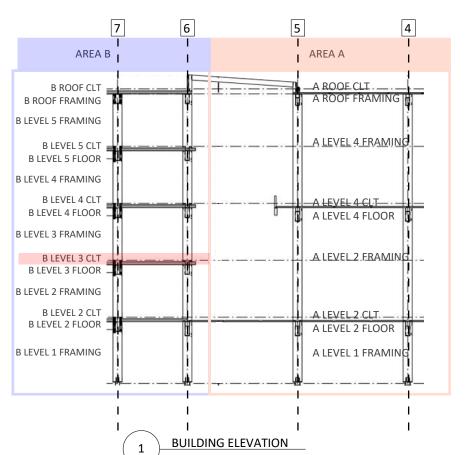
DATE 10/06/20

DATE 10/14/20
DATE

Pre-setting TCC panel edges at GL's

10 and 11 to EOR's specified camber

prior to pouring concrete by others



B - MEMBER NAME	WEIGHT LB
FP3-221	6885
FP3-223	9185
FP3-224	9185
FP3-226	4068
FP3-286	2763
FP3-228	4638
FP3-230	2740
FP3-233	5809
FP3-234	6001
FP3-235	7515
FP3-238	6657
FP3-240	5158
FP3-241	8917

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- LINE OF CRANE "A" PIVOT POINT

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REV

DATE CREATED August 27, 2020

Google FONE PROJECT NO. <ProjectNumber>

20200827_GoogleFON E_Sequence to Level 4

PROJECT Borregas

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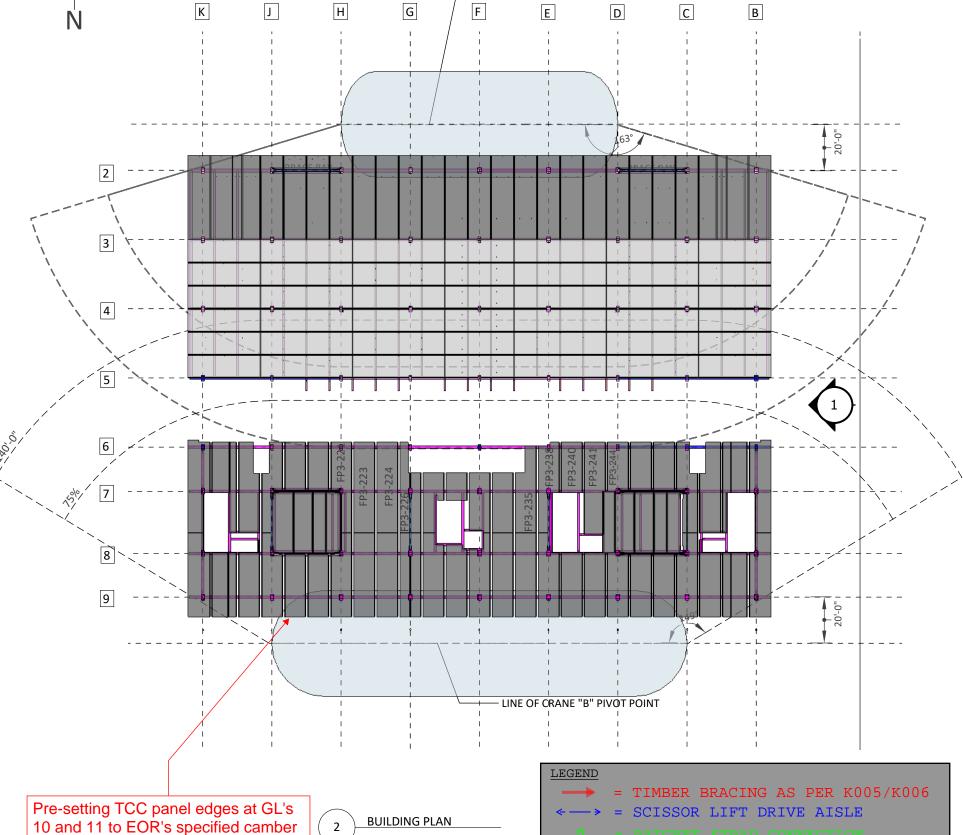
DESIGNED BY

DESCRIPTION

B-LVL 3 CLT

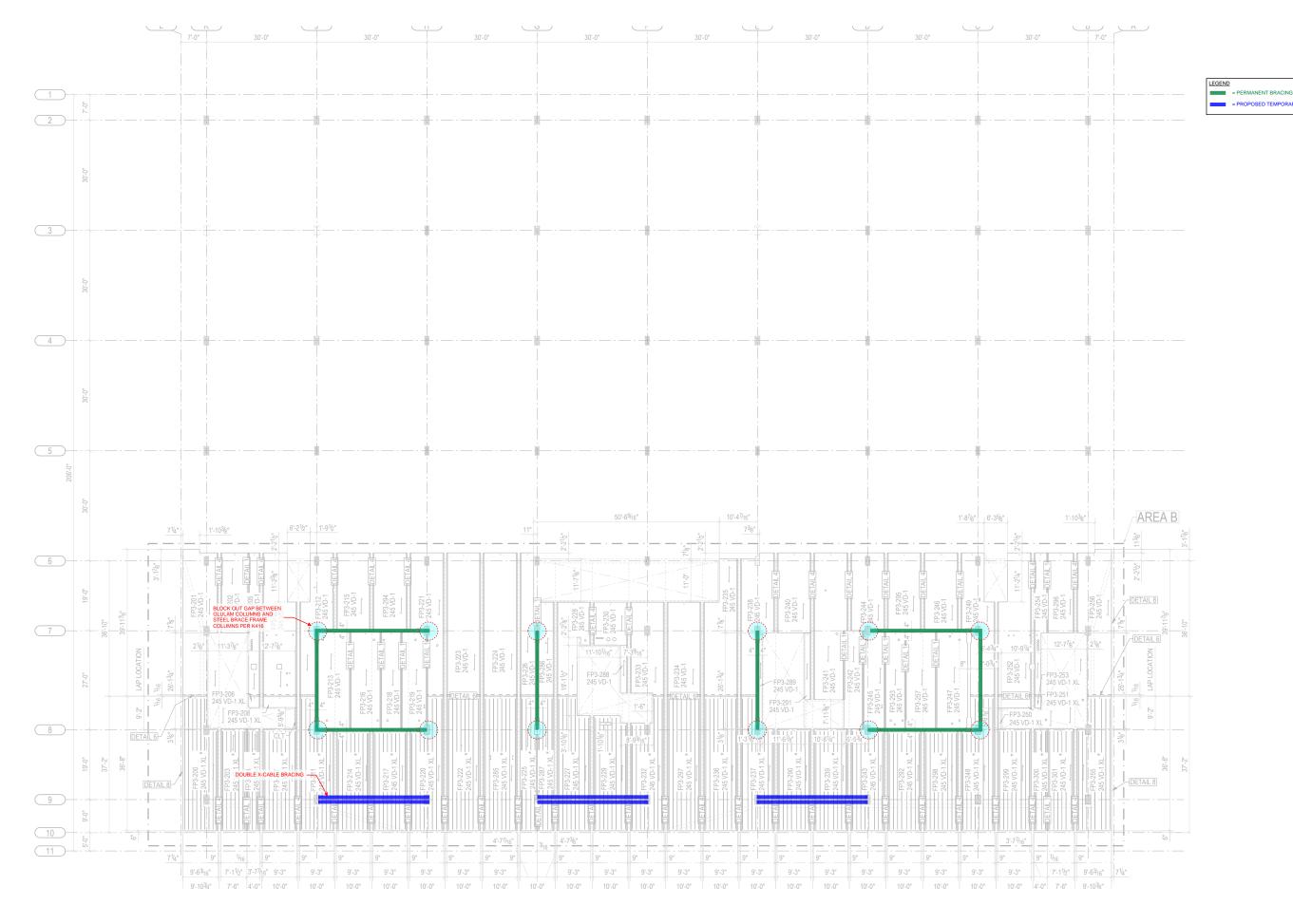
DATE 6/11/19 DATE 10/06/20

DATE 10/14/20 DATE



prior to pouring concrete by others

2



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2020-12-18 Issued for Construction

Plot Date: 2020-10-16

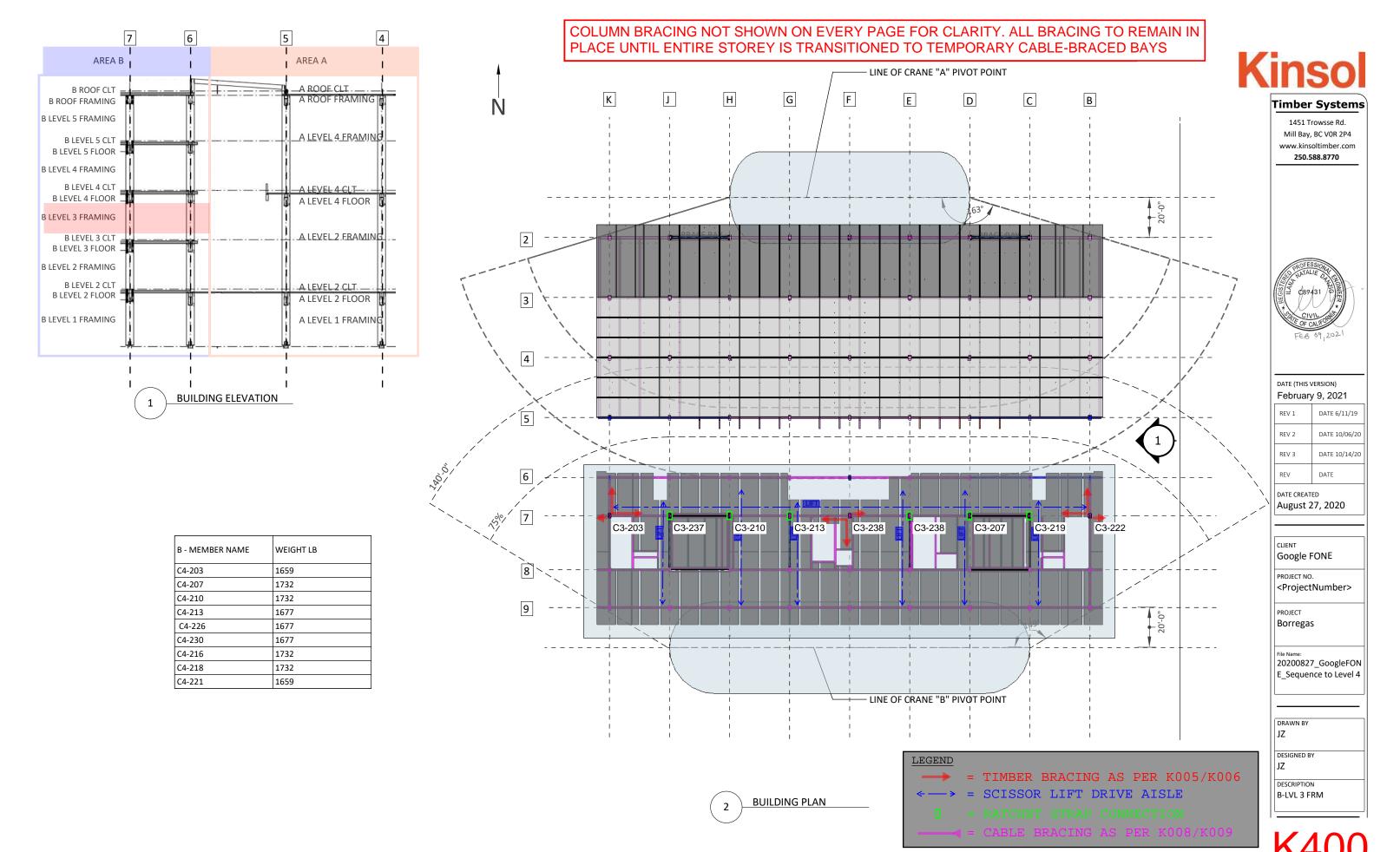
LVL 3 Bracing Layout Plan (Medium Duty Deck)

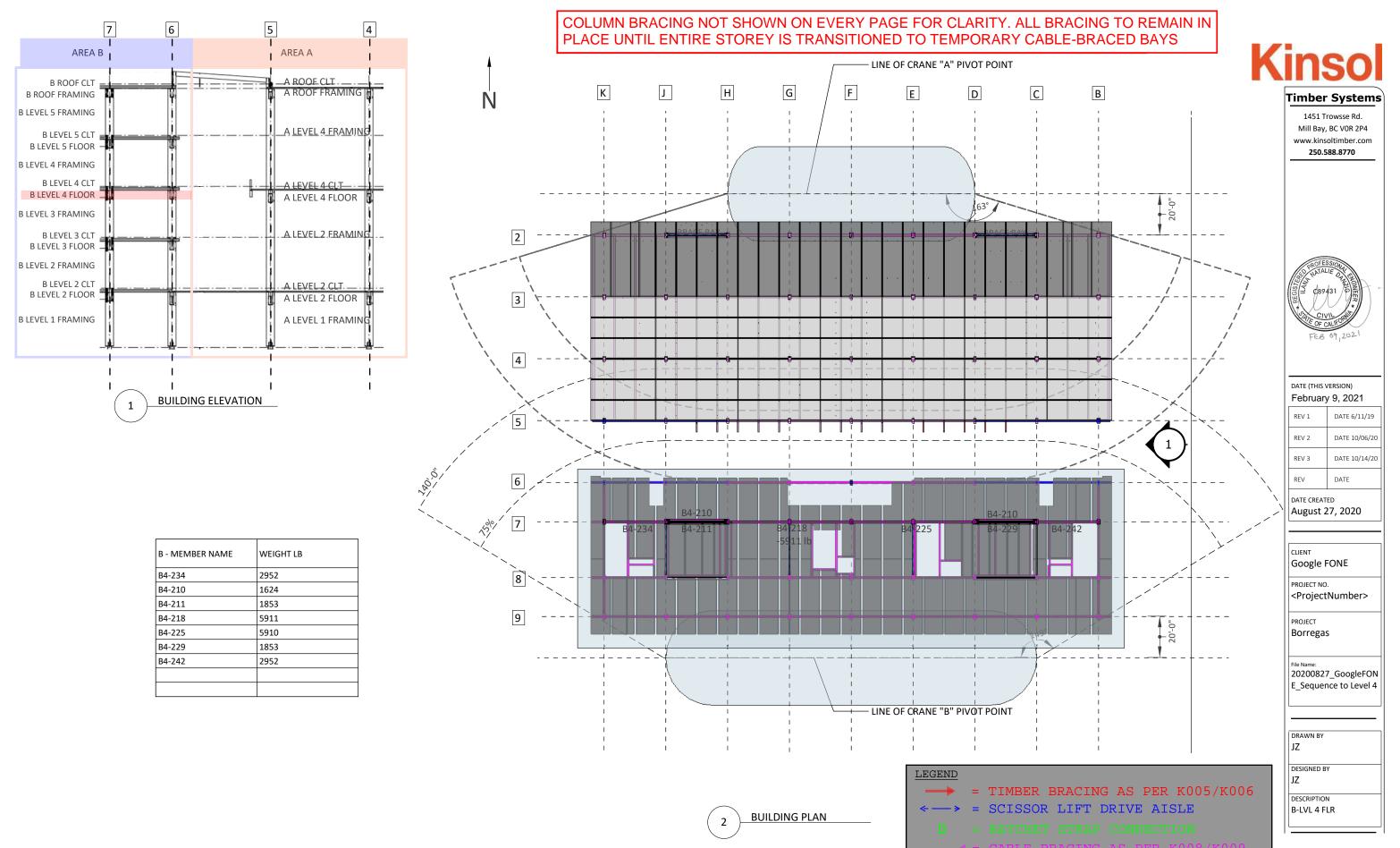
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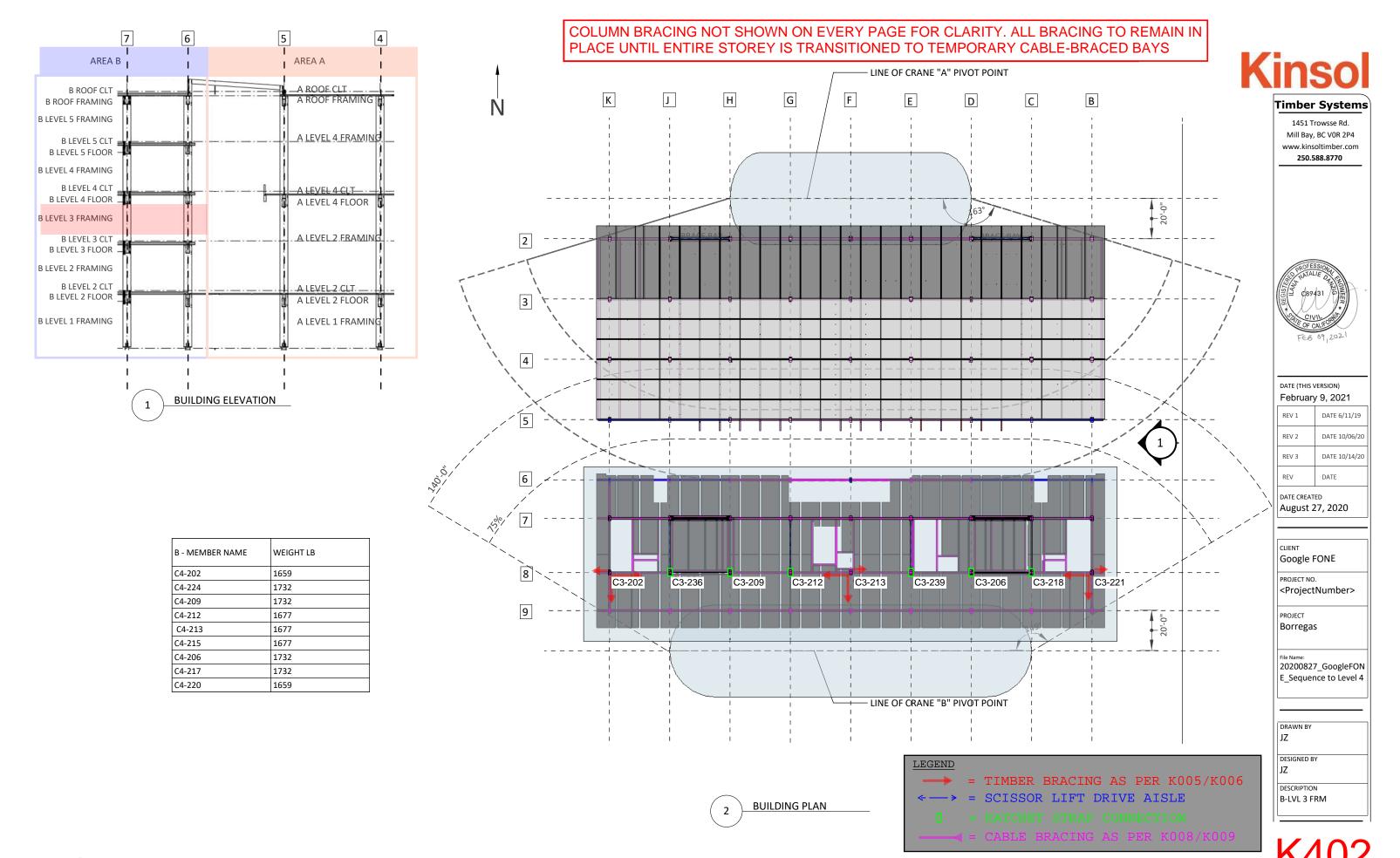
K319

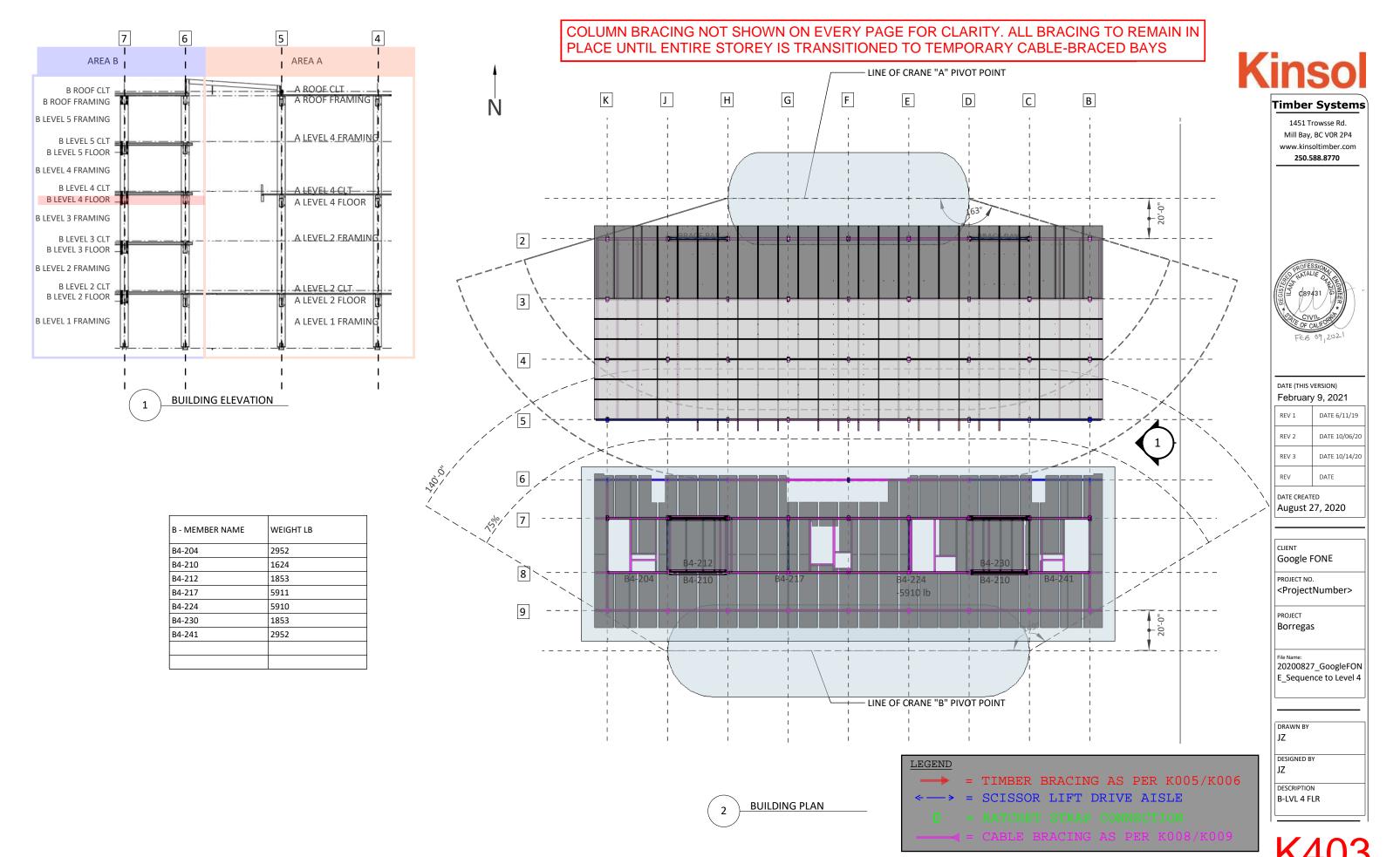
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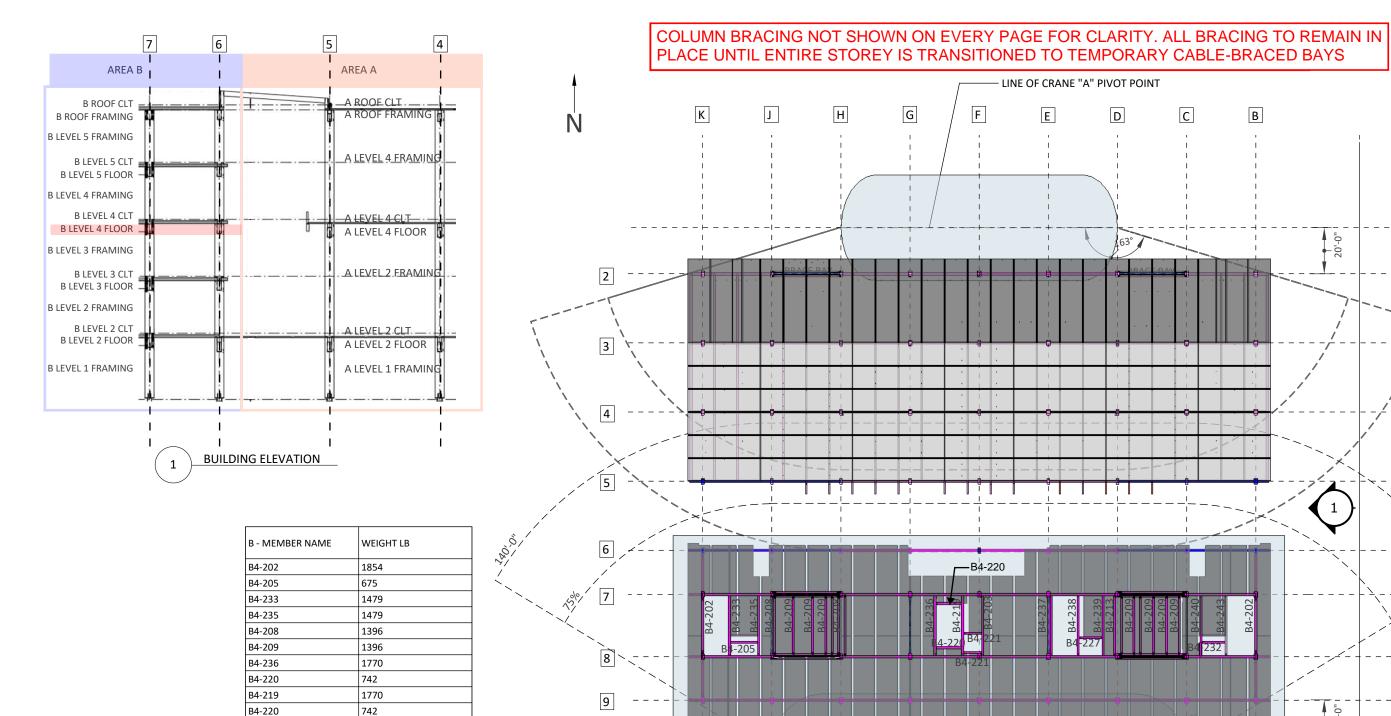
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REV 2	DATE 10/06/20
REV 3	DATE 10/14/20
REV	DATE

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DESCRIPTION B-LVL 4 FLR

LEGEND = TIMBER BRACING AS PER K005/K006 = SCISSOR LIFT DRIVE AISLE

BUILDING PLAN

- LINE OF CRANE "B" PIVOT POINT

B4-221

B4-203 B4-237

B4-238

B4-227

B4-239

B4-213

B4-240

B4-232

B4-243

453 1479

1479

1479

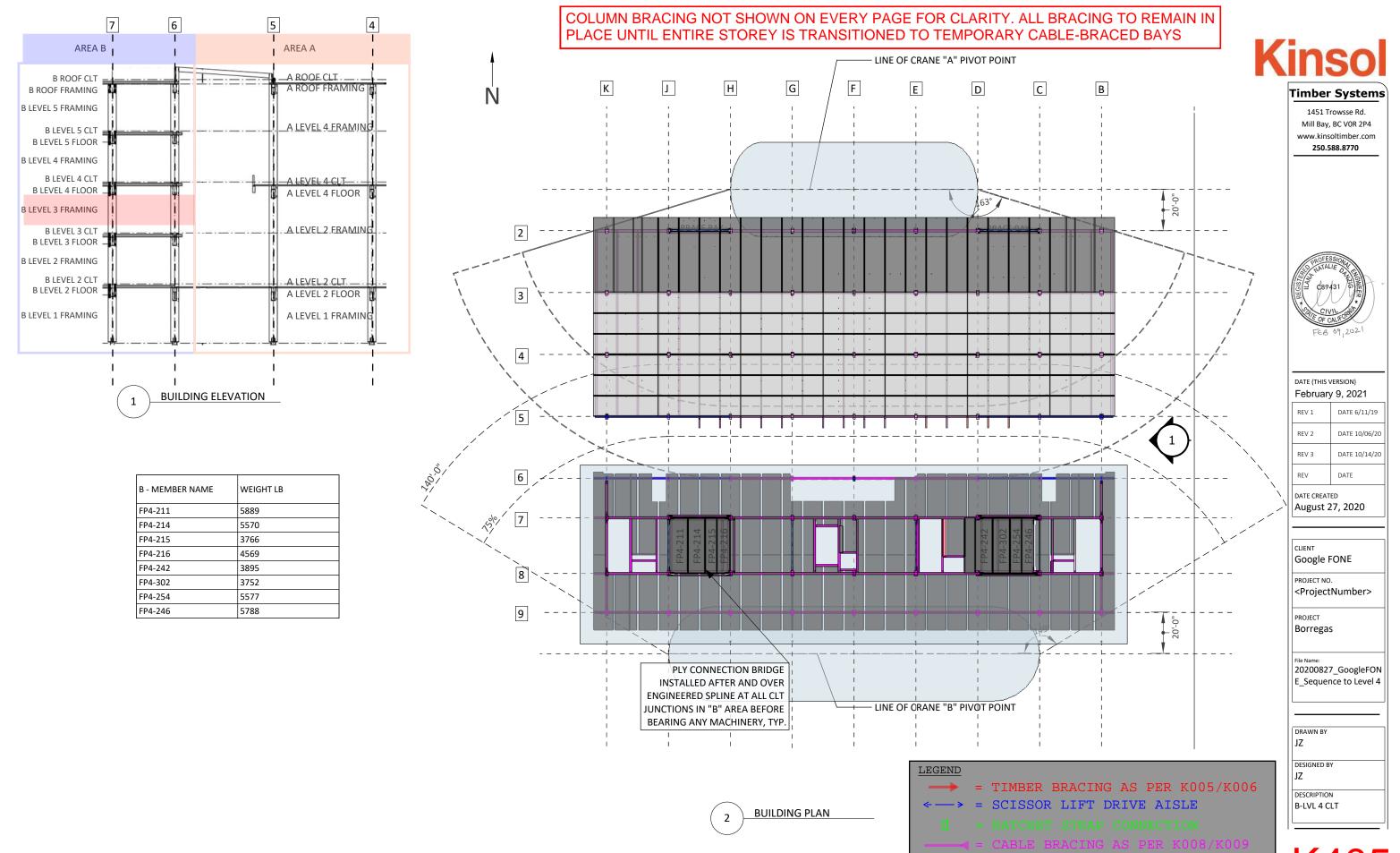
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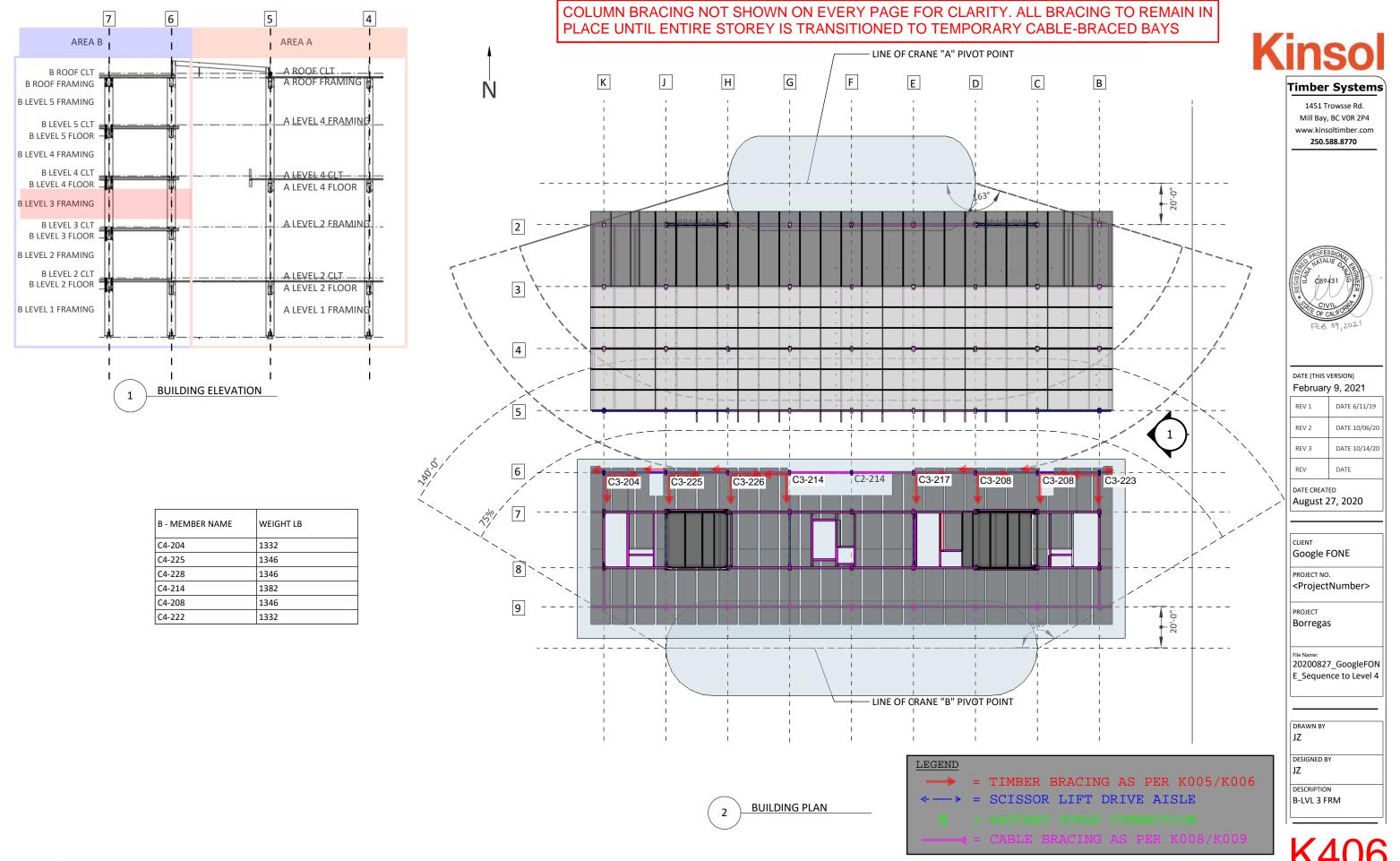
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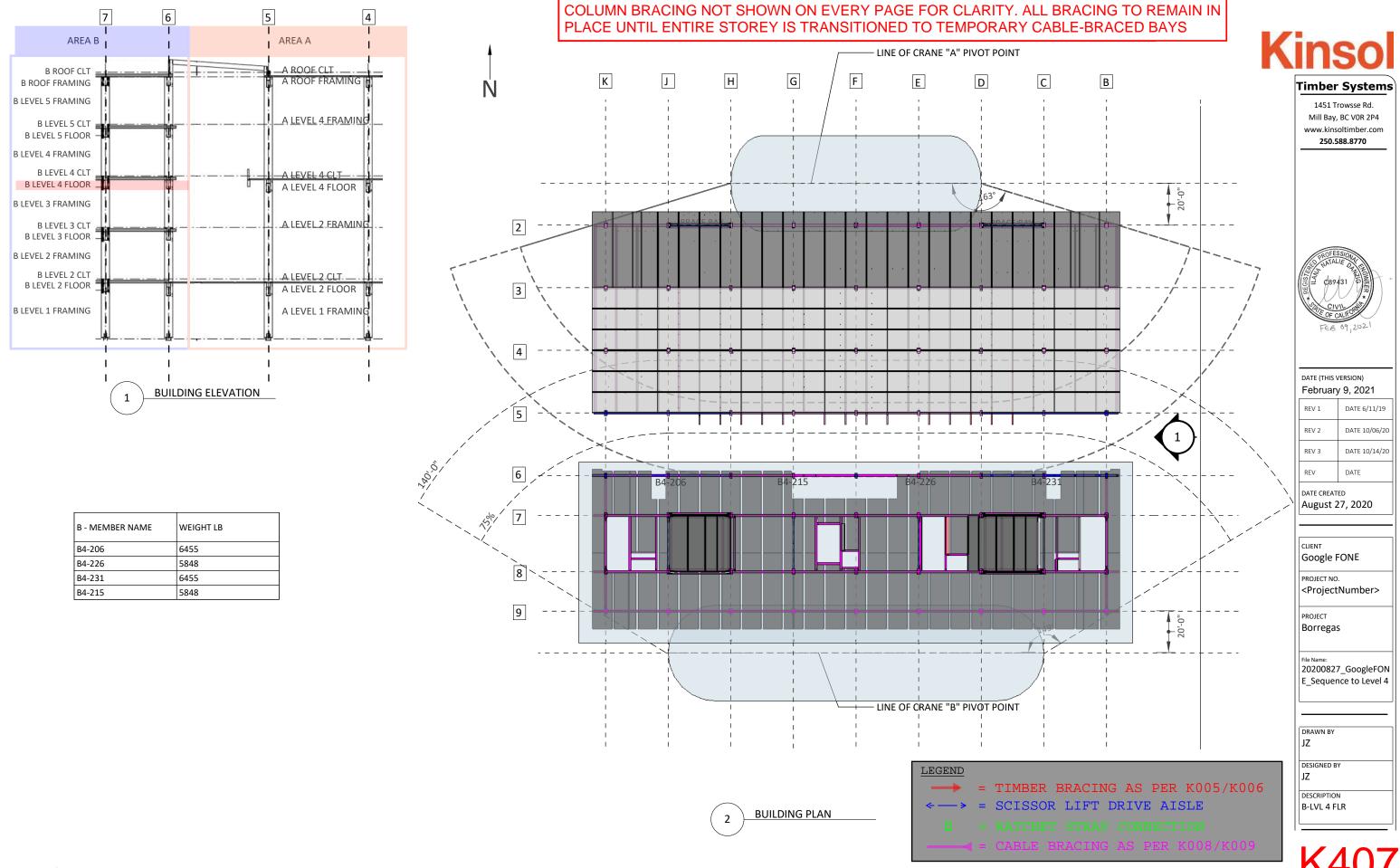
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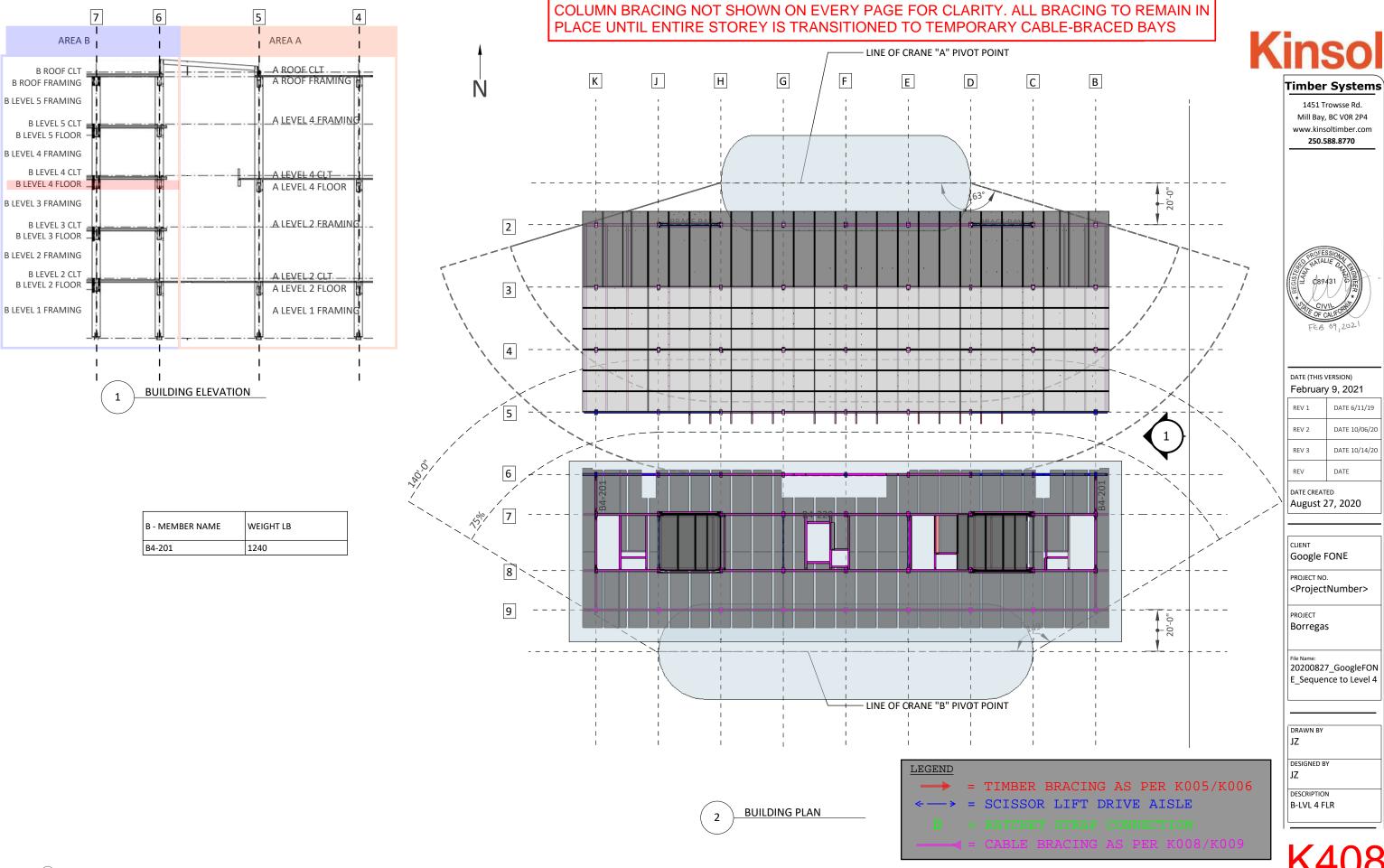
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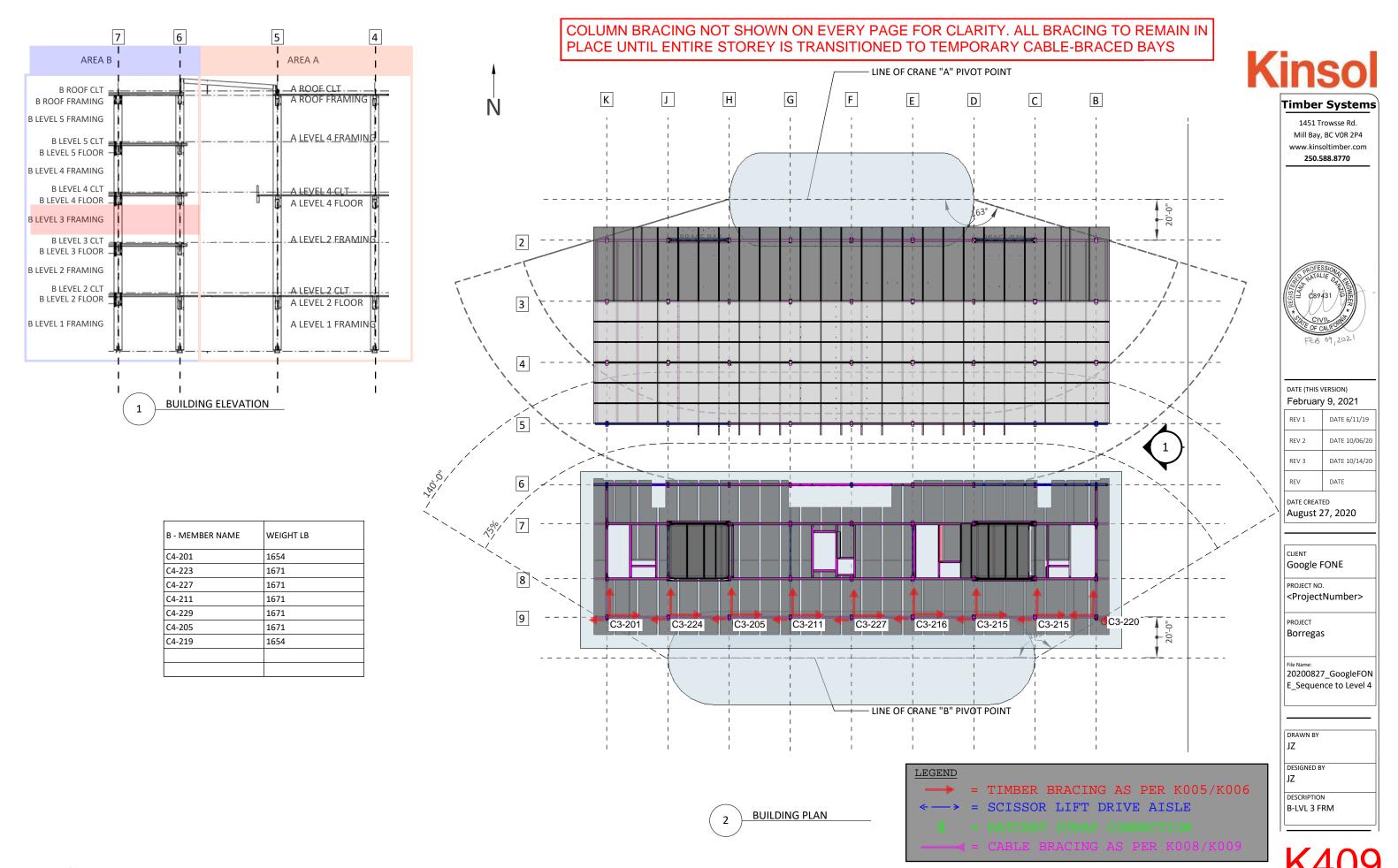
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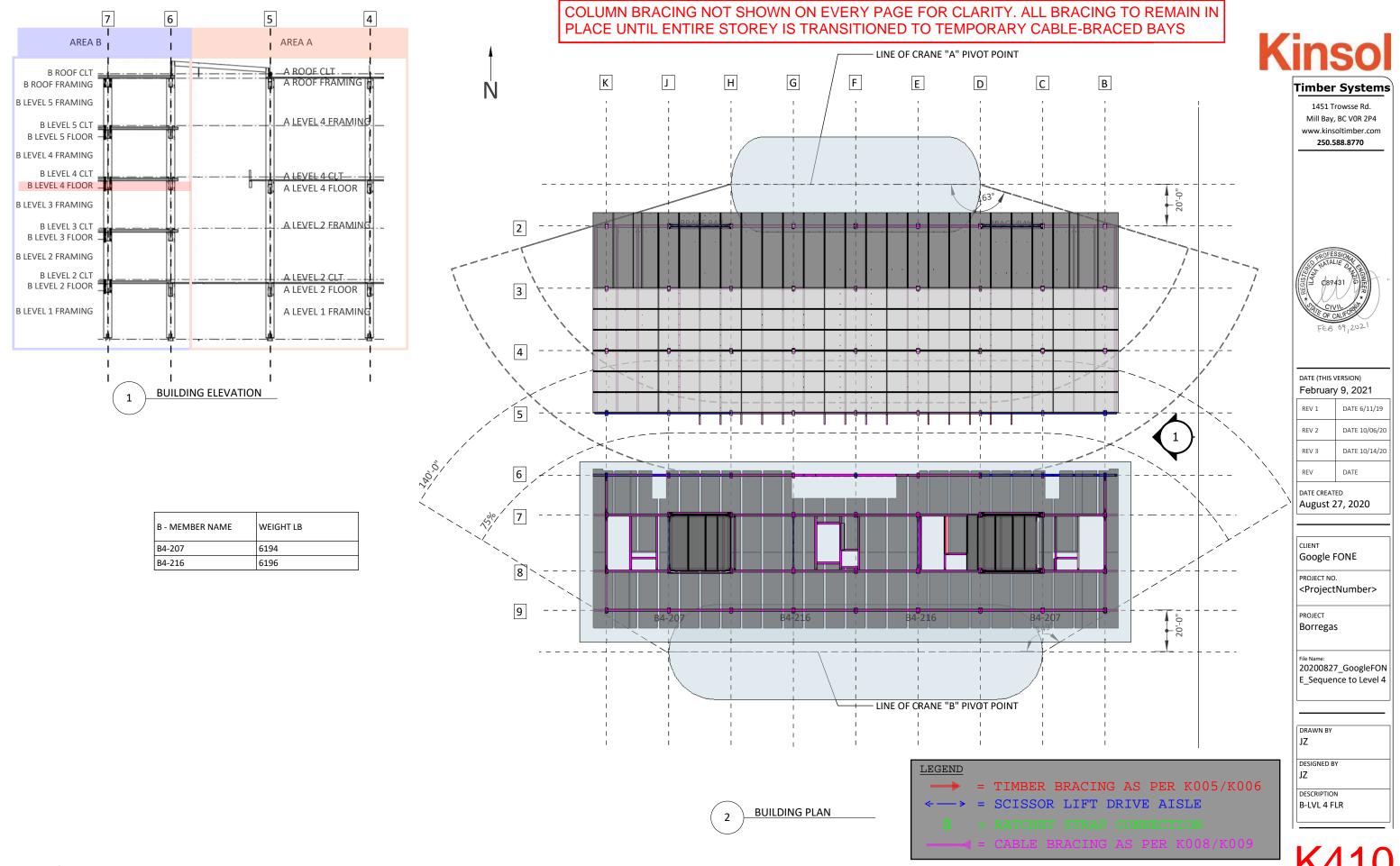


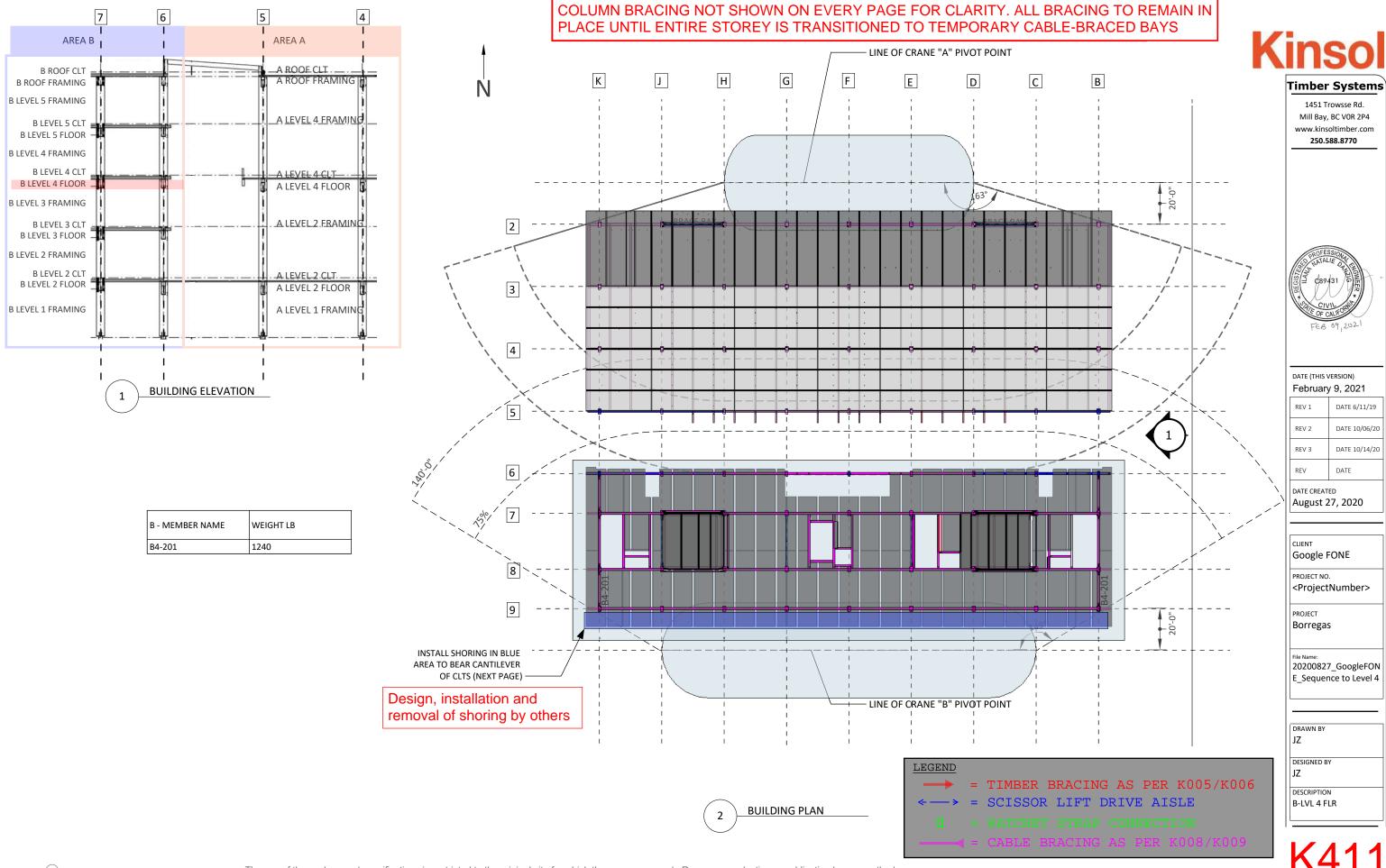


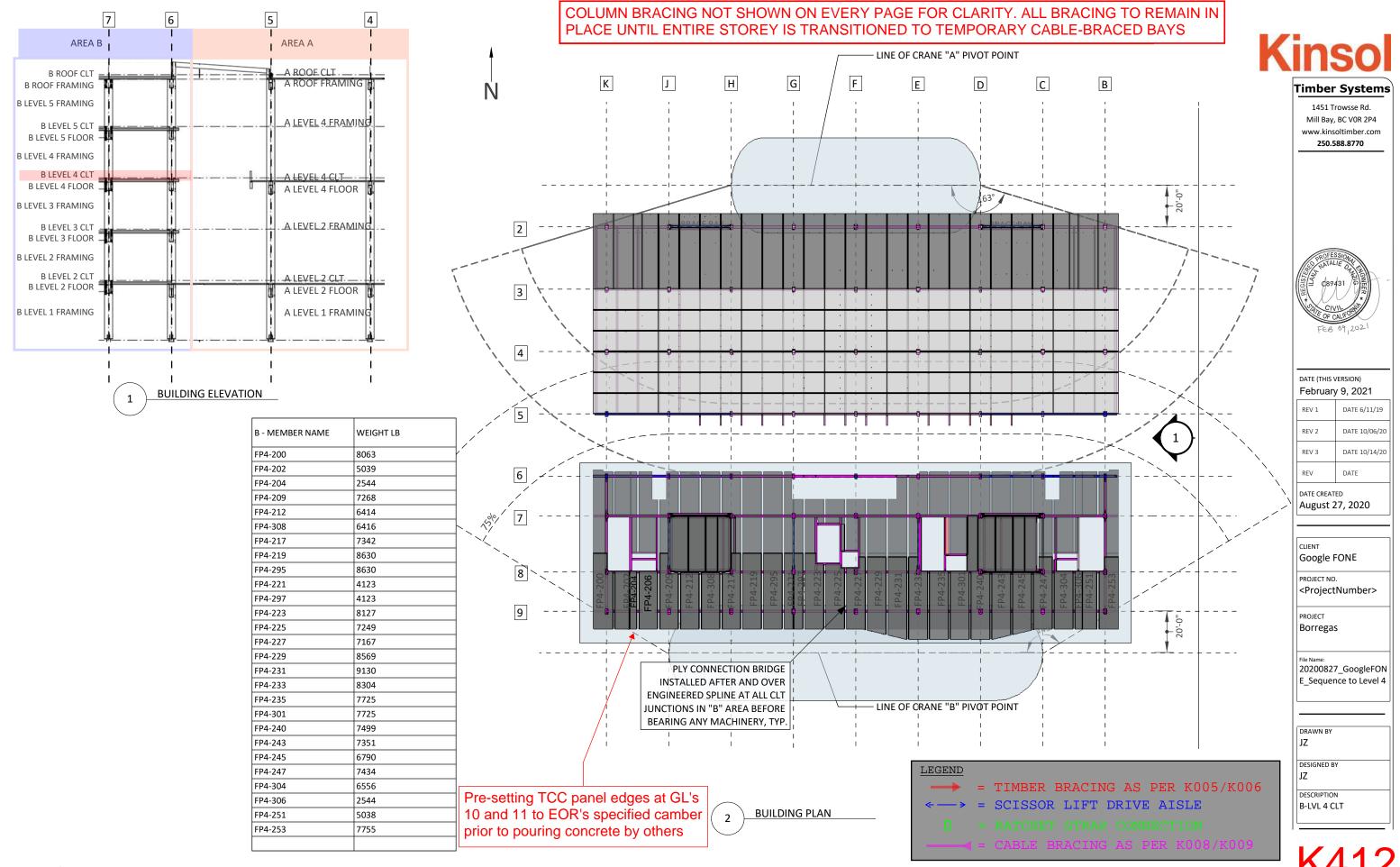


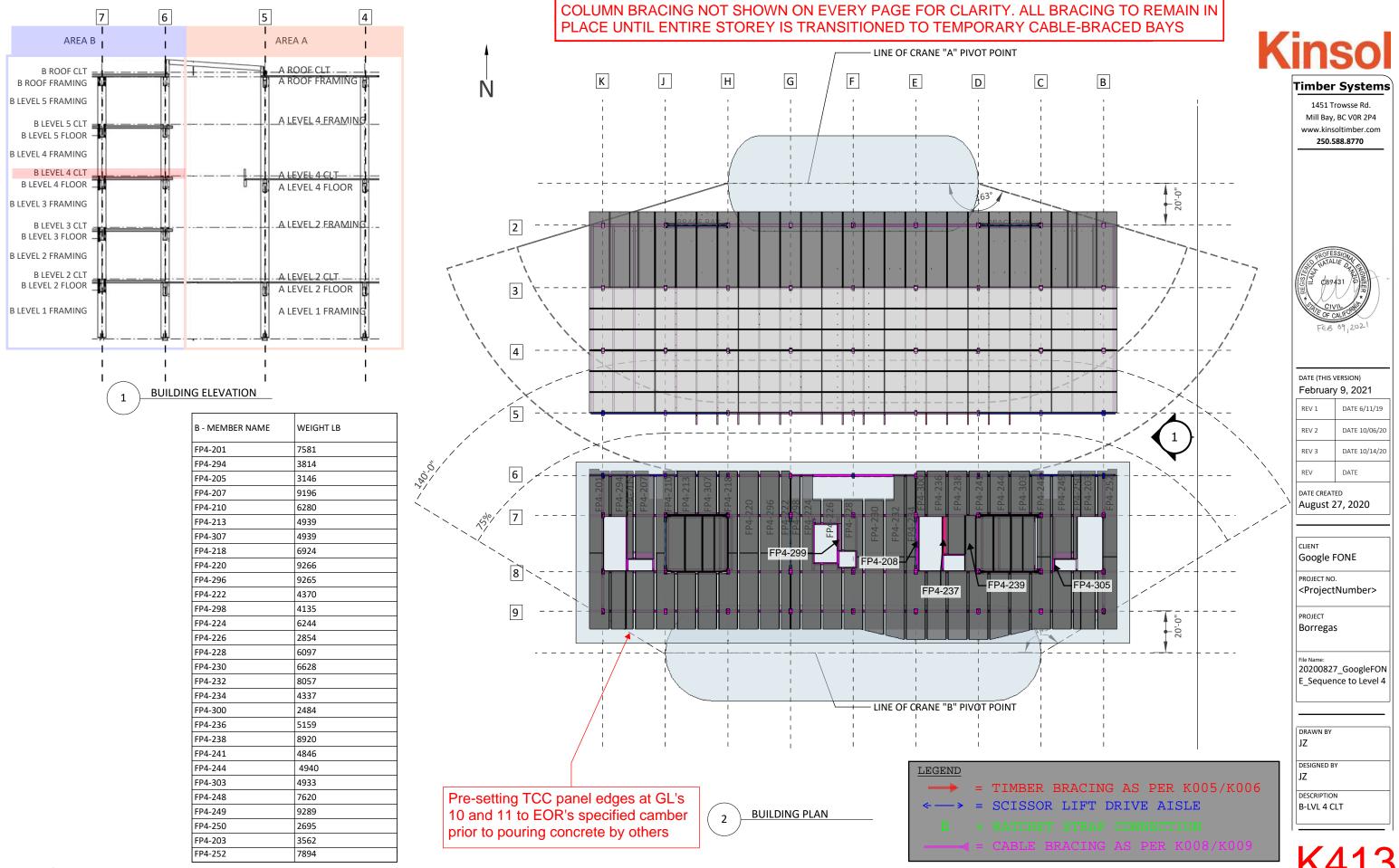


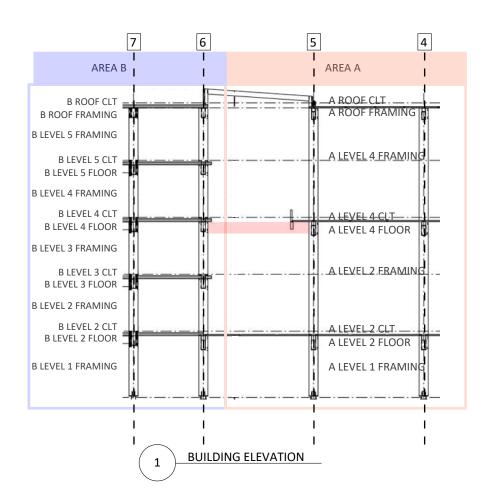






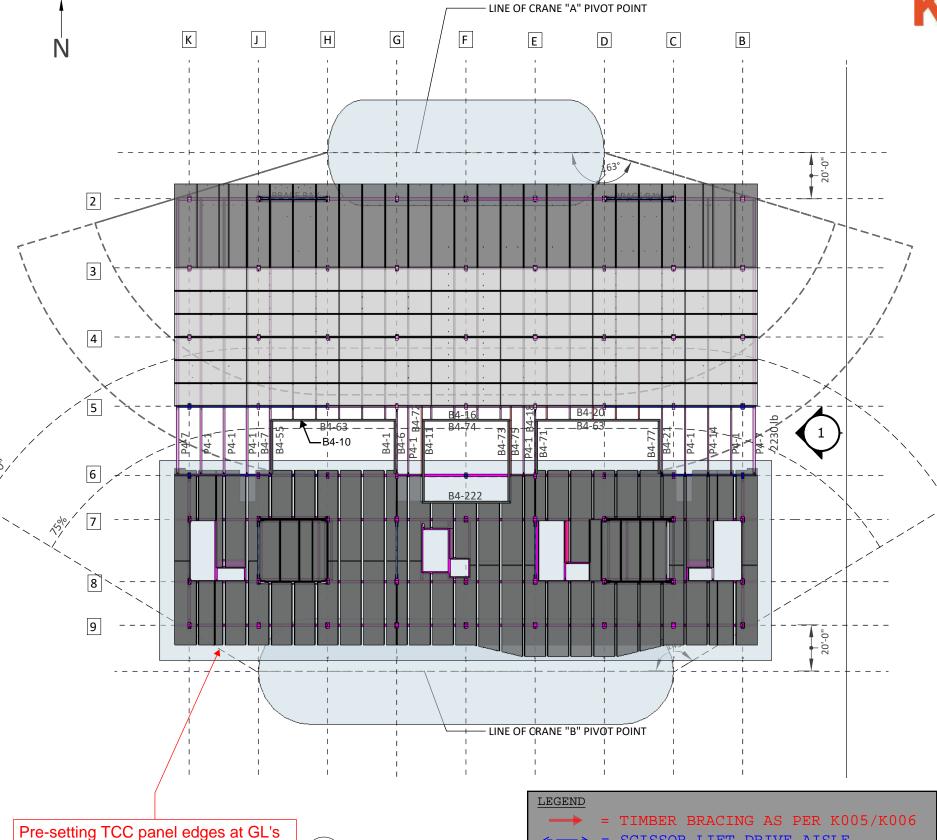






B - MEMBER NAME	WEIGHT LB
P4-7	2230
P4-1	1265
B4-7	1264
B4-55	974
B4-63	2407
B4-1	1178
B4-6	1017
B4-72	1277
B4-11	1550
B4-74	1682
B4-16	1653
B4-73	1550
B4-75	1277
B4-18	1178
B4-71	1017
B4-20	2367
B4-63	2407
B4-77	1072
B4-21	1264
P4-14	1265
B4-222	1682

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BUILDING PLAN

2

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DESIGNED BY

DESCRIPTION

A/B LVL 4 FLR

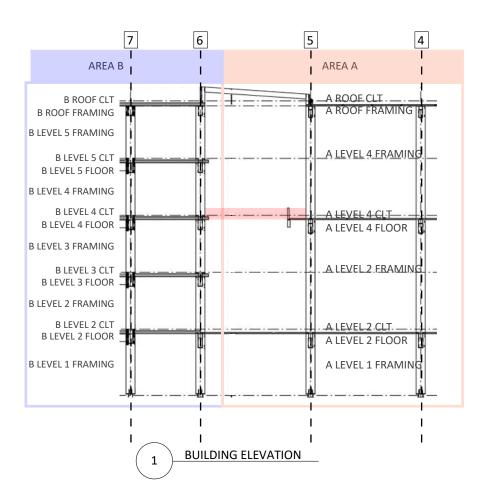
SCISSOR LIFT DRIVE AISLE

DATE 6/11/19 DATE 10/06/20

DATE 10/14/20 DATE

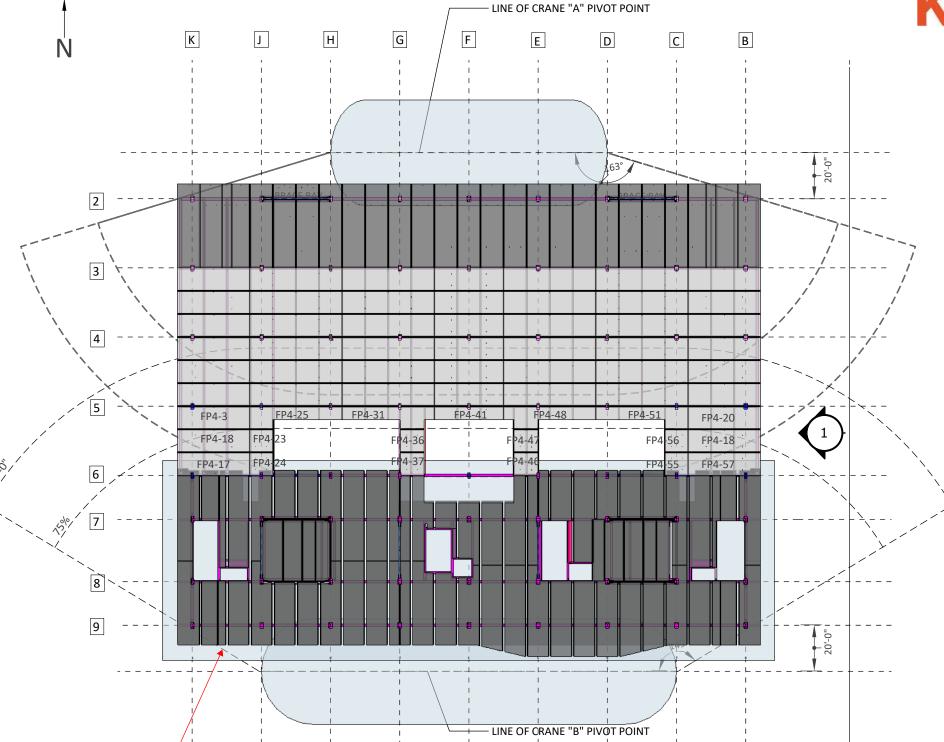
10 and 11 to EOR's specified camber

prior to pouring concrete by others



B - MEMBER NAME	WEIGHT LB
FP4-3	3411
FP4-18	3431
FP4-17	3433
FP4-25	2955
FP4-23	1079
FP4-24	1069
FP4-31	2999
FP4-36	1149
FP4-37	1148
FP4-41	1887
FP4-48	2996
FP4-47	1141
FP4-46	1140
FP4-51	2955
FP4-56	1079
FP4-55	1069
FP4-20	3411
FP4-18	3431
FP4-57	3434

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BUILDING PLAN

2

LEGEND

= RATCHET STRAP CONNECTION

= CABLE BRACING AS PER K008/K009

K415

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DESCRIPTION

A/B LVL 4 CLT

DATE 6/11/19

DATE 10/06/20

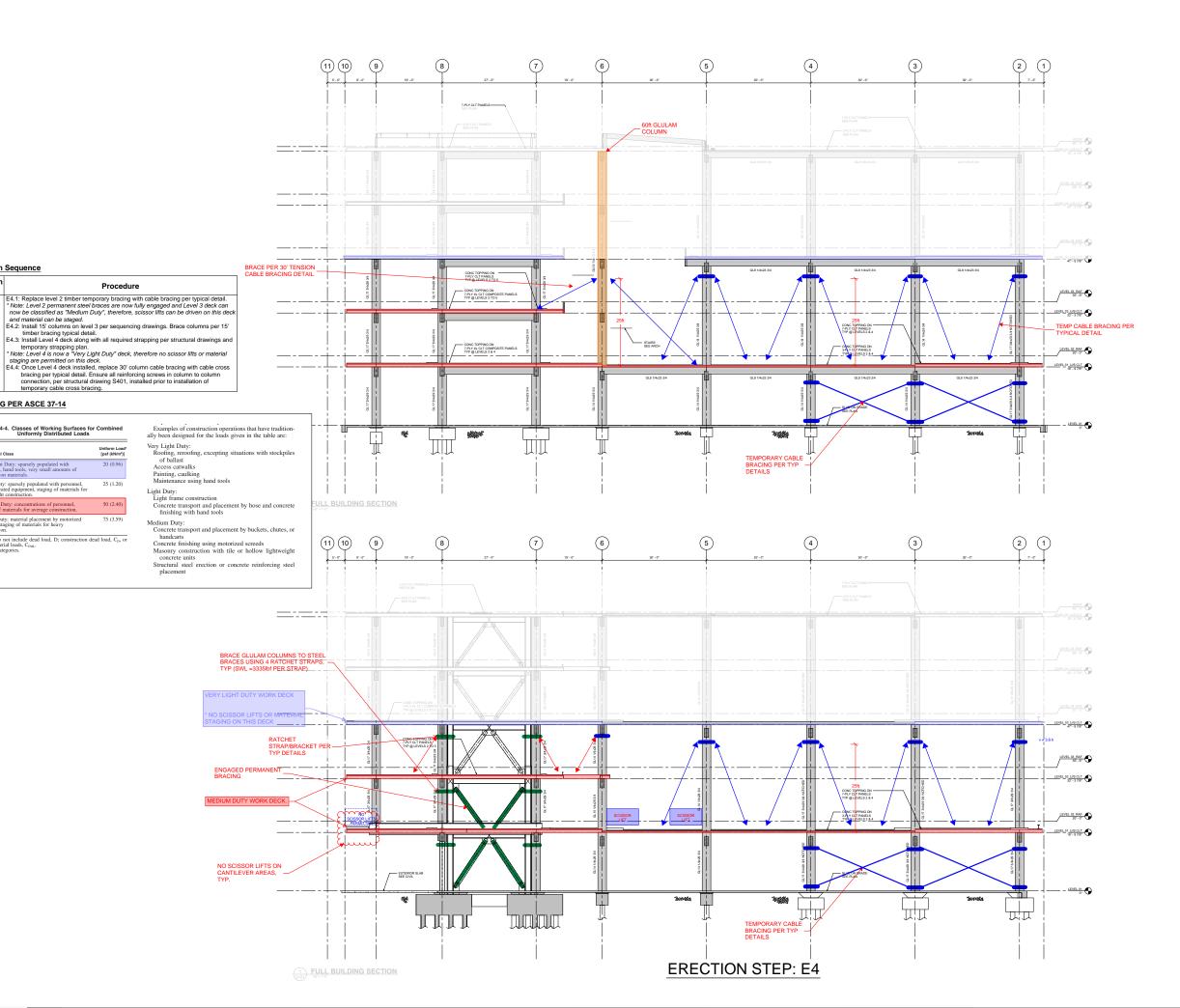
DATE 10/14/20

DATE

Pre-setting TCC panel edges at GL's

10 and 11 to EOR's specified camber

prior to pouring concrete by others



Erection Sequence Erection

LOADING PER ASCE 37-14

Very Light Duty: sparsely populated with personnel, hand tools, very small amounts of construction materials.

Step E4

Procedure

20 (0.96) 25 (1.20)

temporary cable cross bracing.

Table 4-4. Classes of Working Surfaces for Combined Uniformly Distributed Loads

 $\overline{^{1}}\text{Loads do not include dead load, D; construction dead load, C_{D}, or fixed material loads, C_{FML}.$ $<math display="inline">^{\text{b}}\text{OSHA}$ categories.

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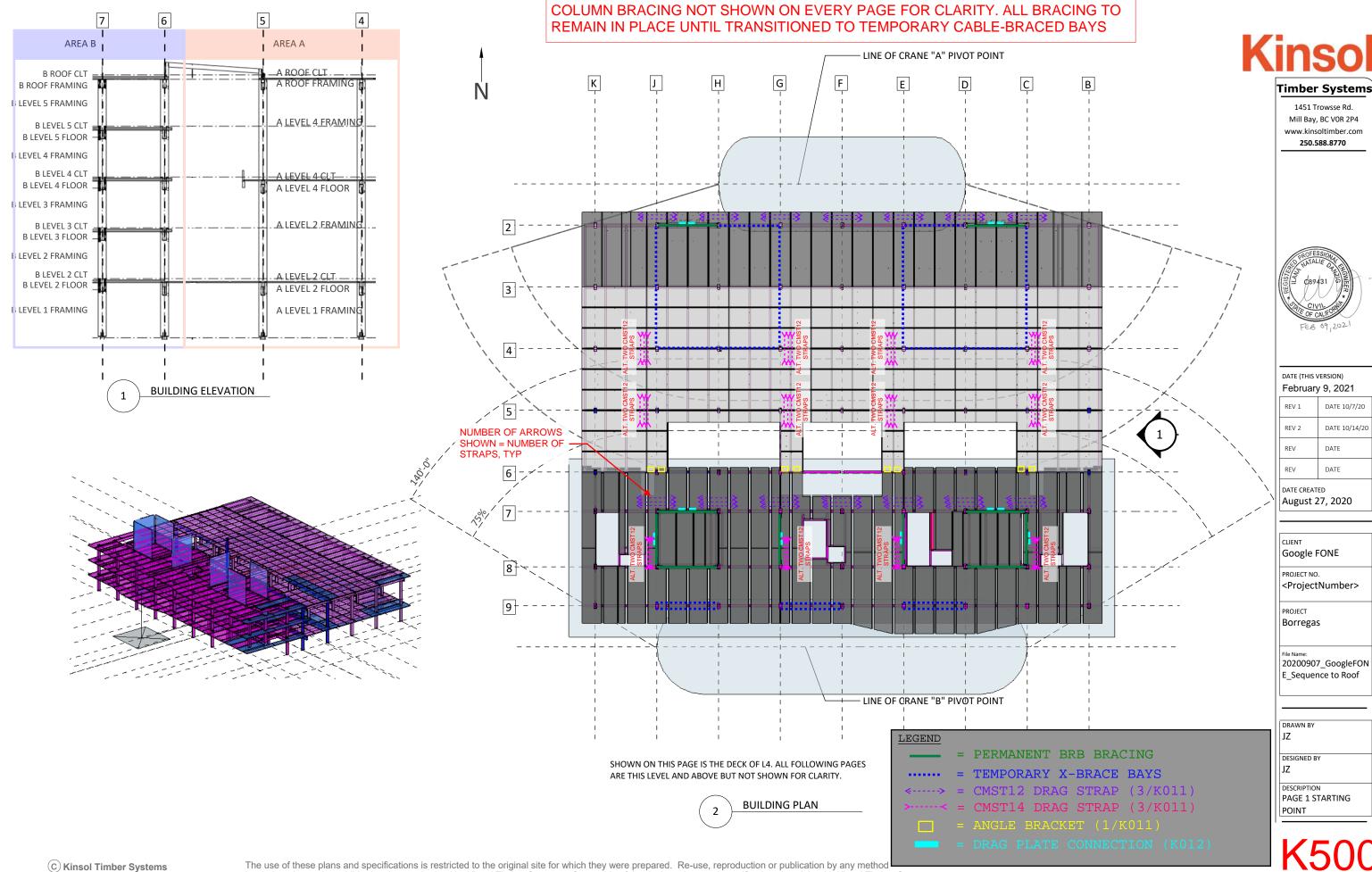
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2020-09-22	Issued for Coordination	
2020-09-29	Issued for Coordination	
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2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	
2021-02-09	Issued for Construction	

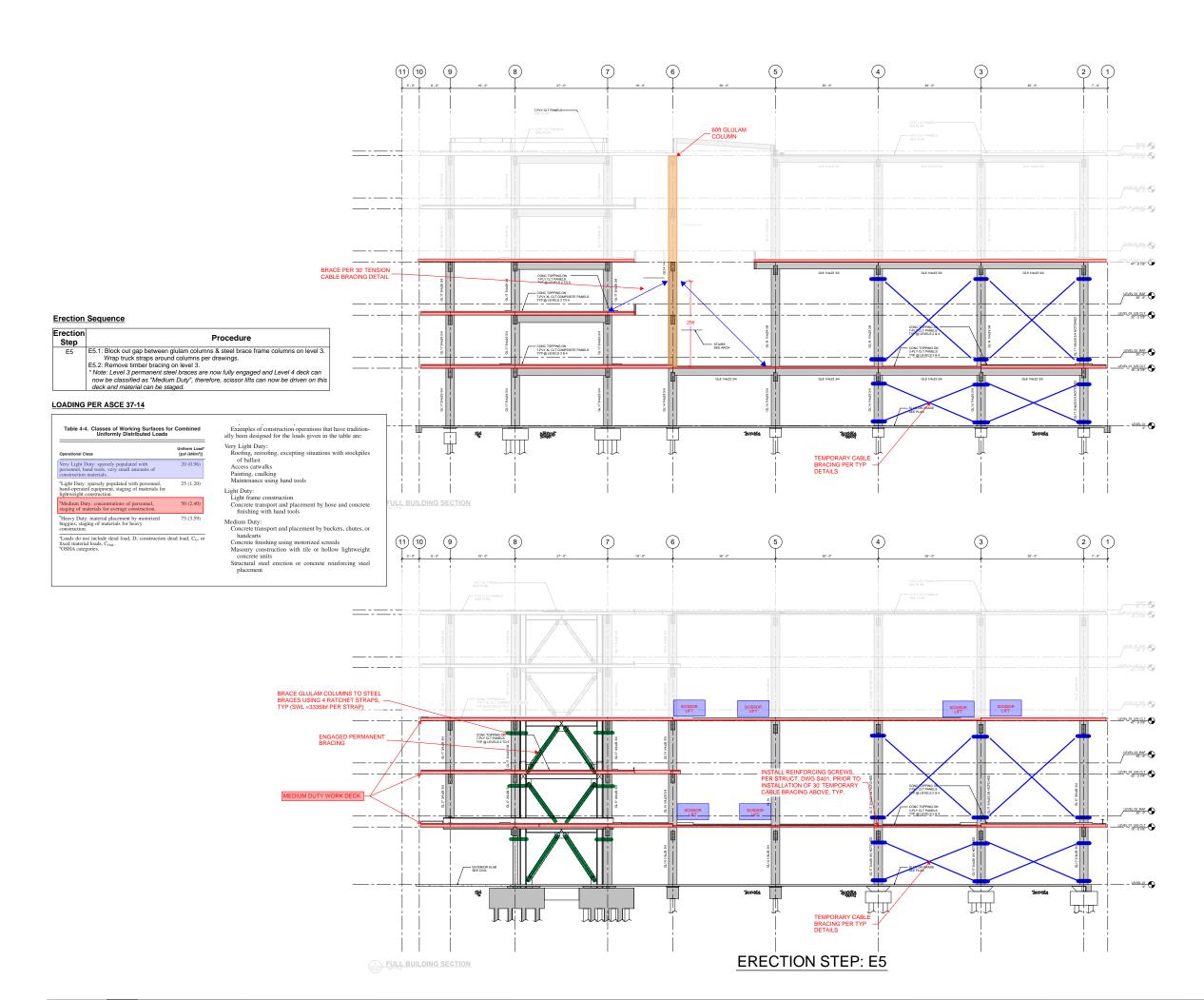
Plot Date:

2020-10-19

Erection Step 4 Bracing

NTS





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Sea



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2020-12-18	Issued for Construction	1
2021-02-09	Issued for Construction	2

Plot Date:

2020-10-19

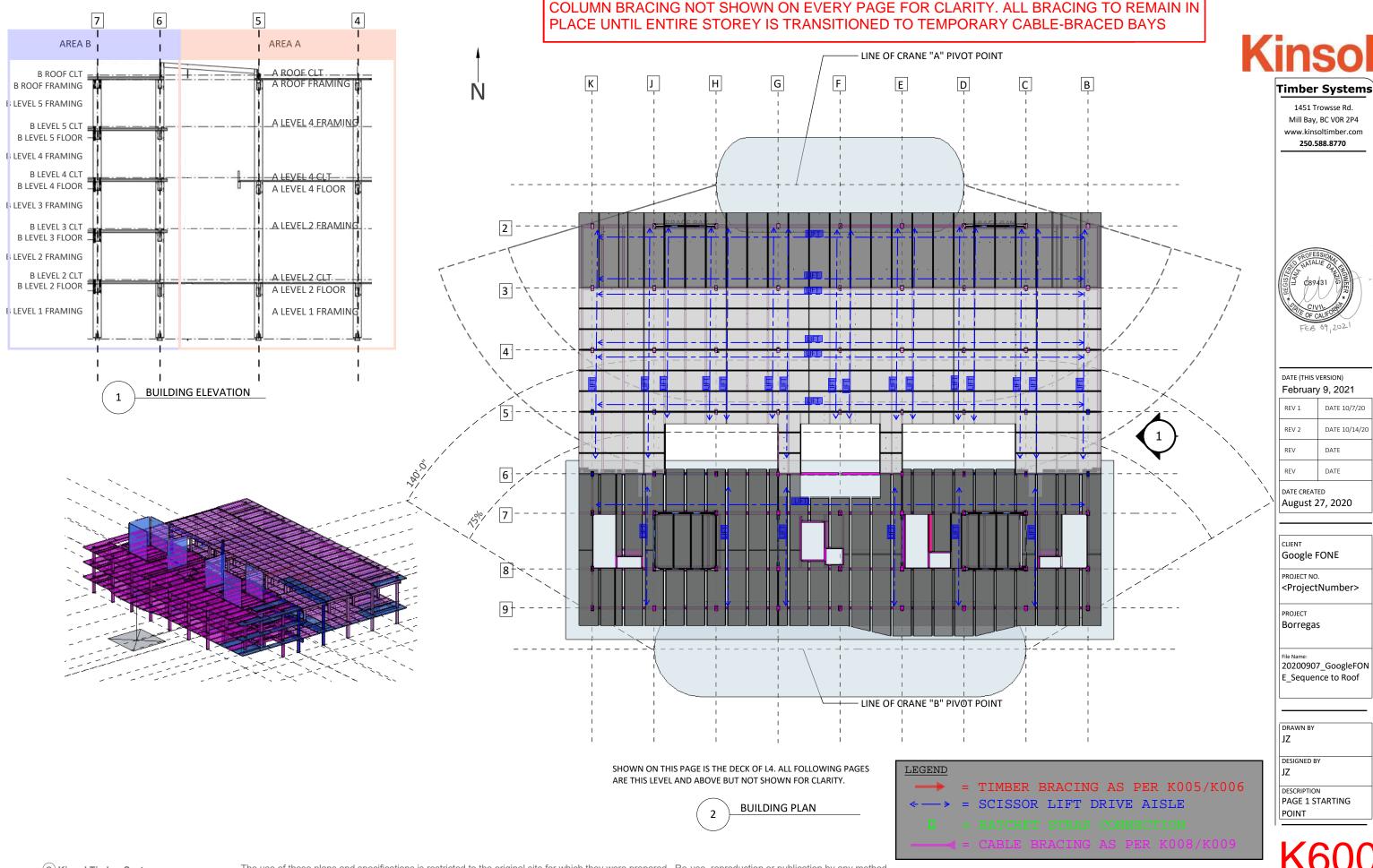
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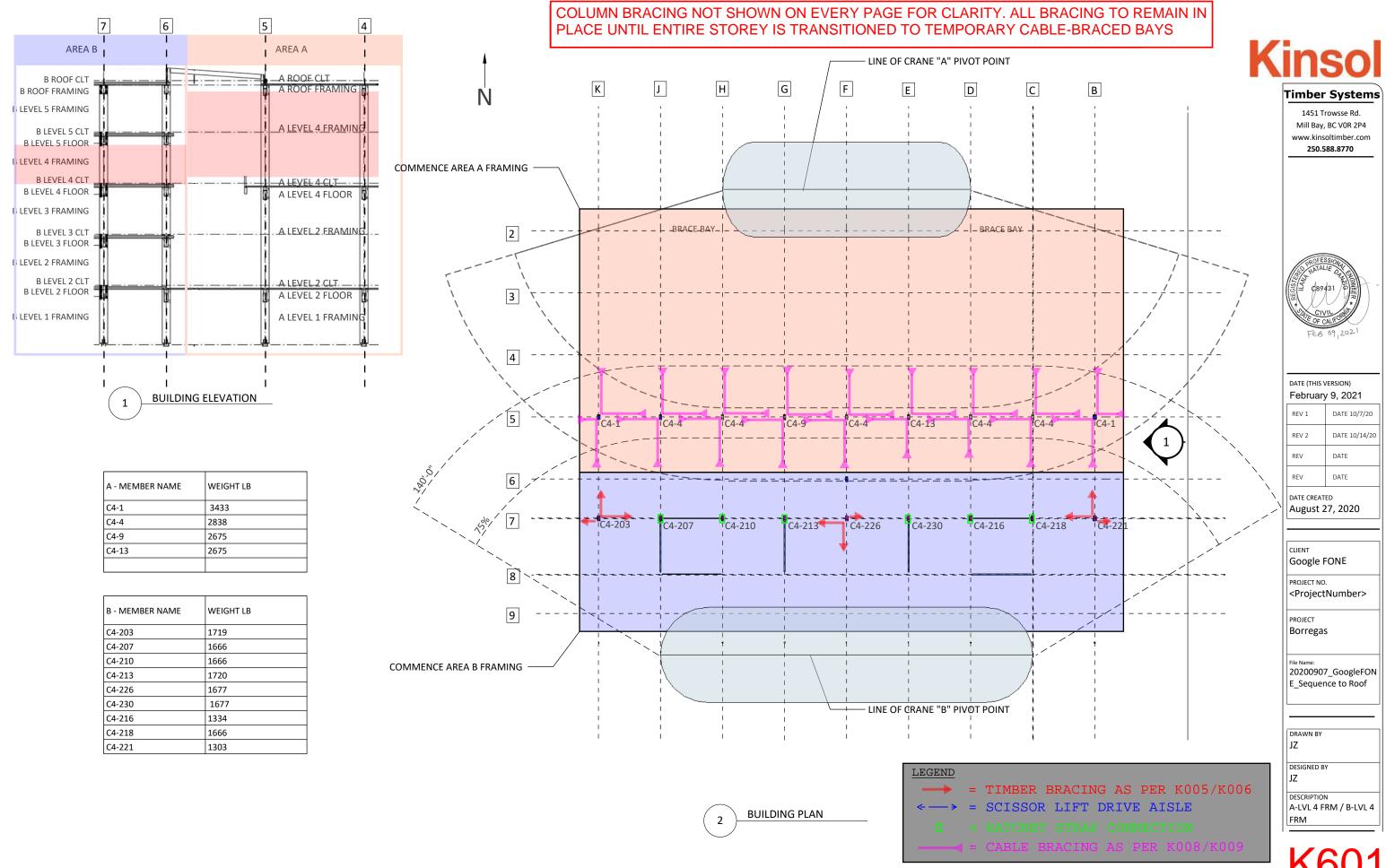
Erection Step 5 Bracing

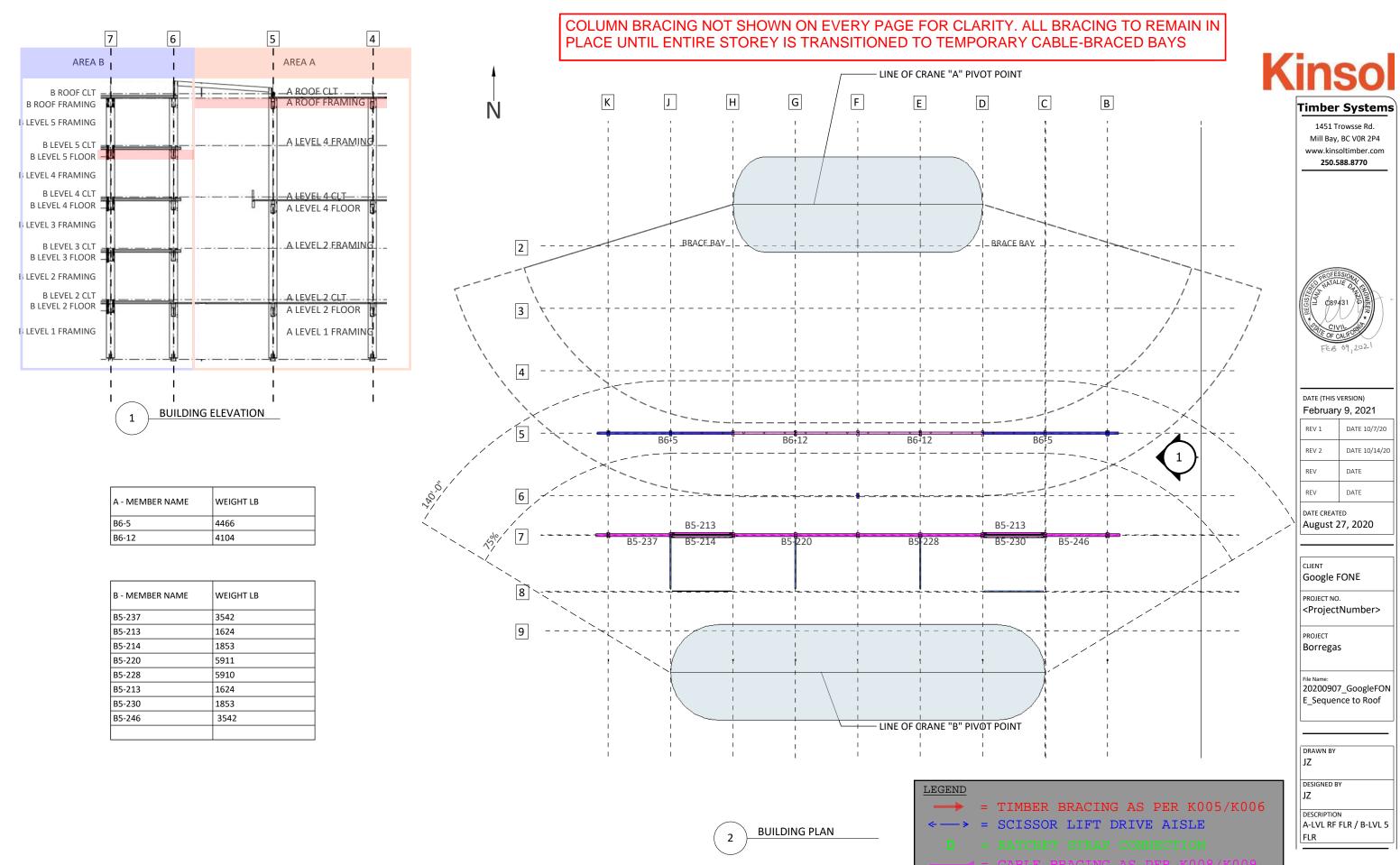
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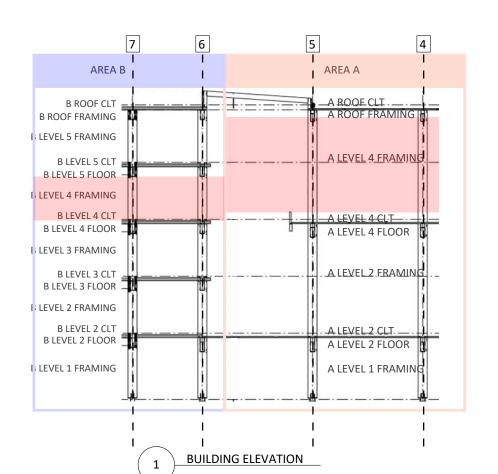
Designed: ME Checked: AG

ng No.: Revision No.:





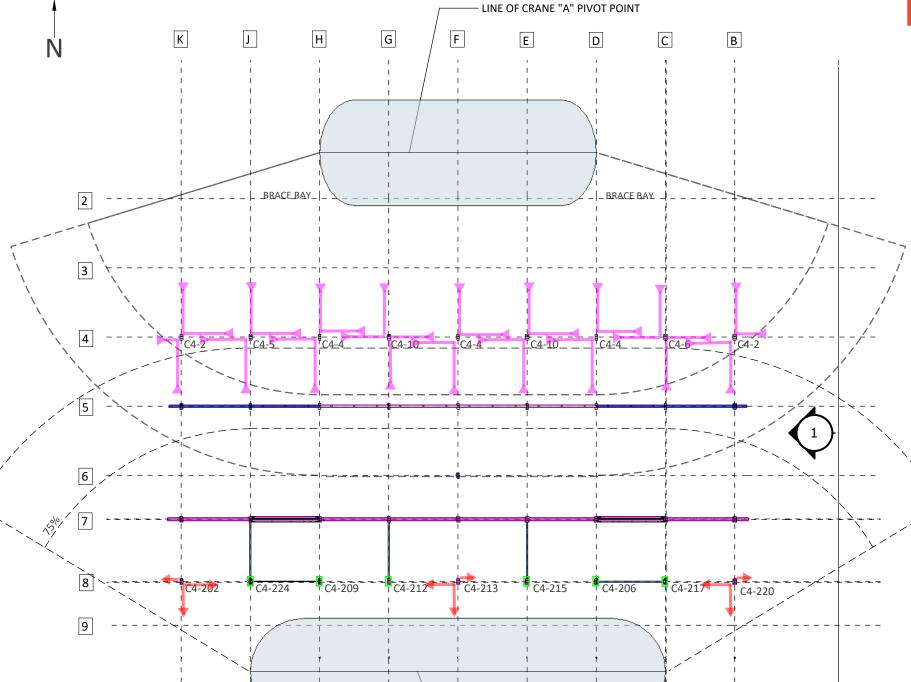




A - MEMBER NAME	WEIGHT LB
C4-2	3536
C4-5	3172
C4-4	2838
C4-10	3290
C4-6	3187

B - MEMBER NAME	WEIGHT LB
C4-202	1719
C4-224	1666
C4-209	1666
C4-212	1720
C4-213	1677
C4-215	1677
C4-206	1334
C4-217	1666
C4-220	1719

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LINE OF CRANE "B" PIVOT POINT

LEGEND

BUILDING PLAN

= TIMBER BRACING AS PER K005/K006

<----> = SCISSOR LIFT DRIVE AISLE

= RATCHET STRAP CONNECTION

= CABLE BRACING AS PER K008/K009

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REV	DATE
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DATE CREATED	

DATE CREATED
August 27, 2020

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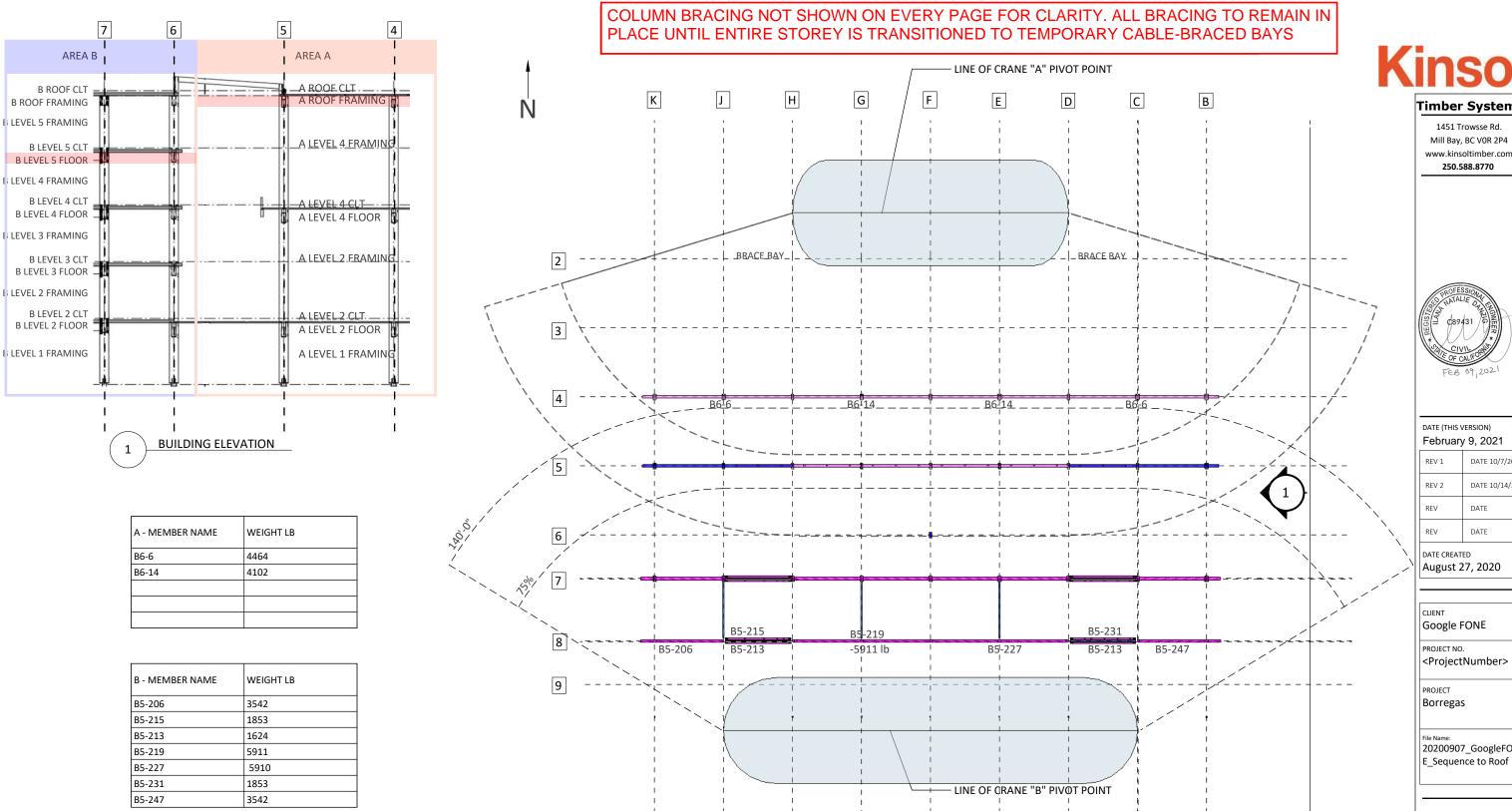
PROJECT Borregas

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JZ

JZ

DESCRIPTION
A-LVL 4 FRM / B-LVL 4
FRM



BUILDING PLAN

LEGEND = TIMBER BRACING AS PER K005/K006 = SCISSOR LIFT DRIVE AISLE

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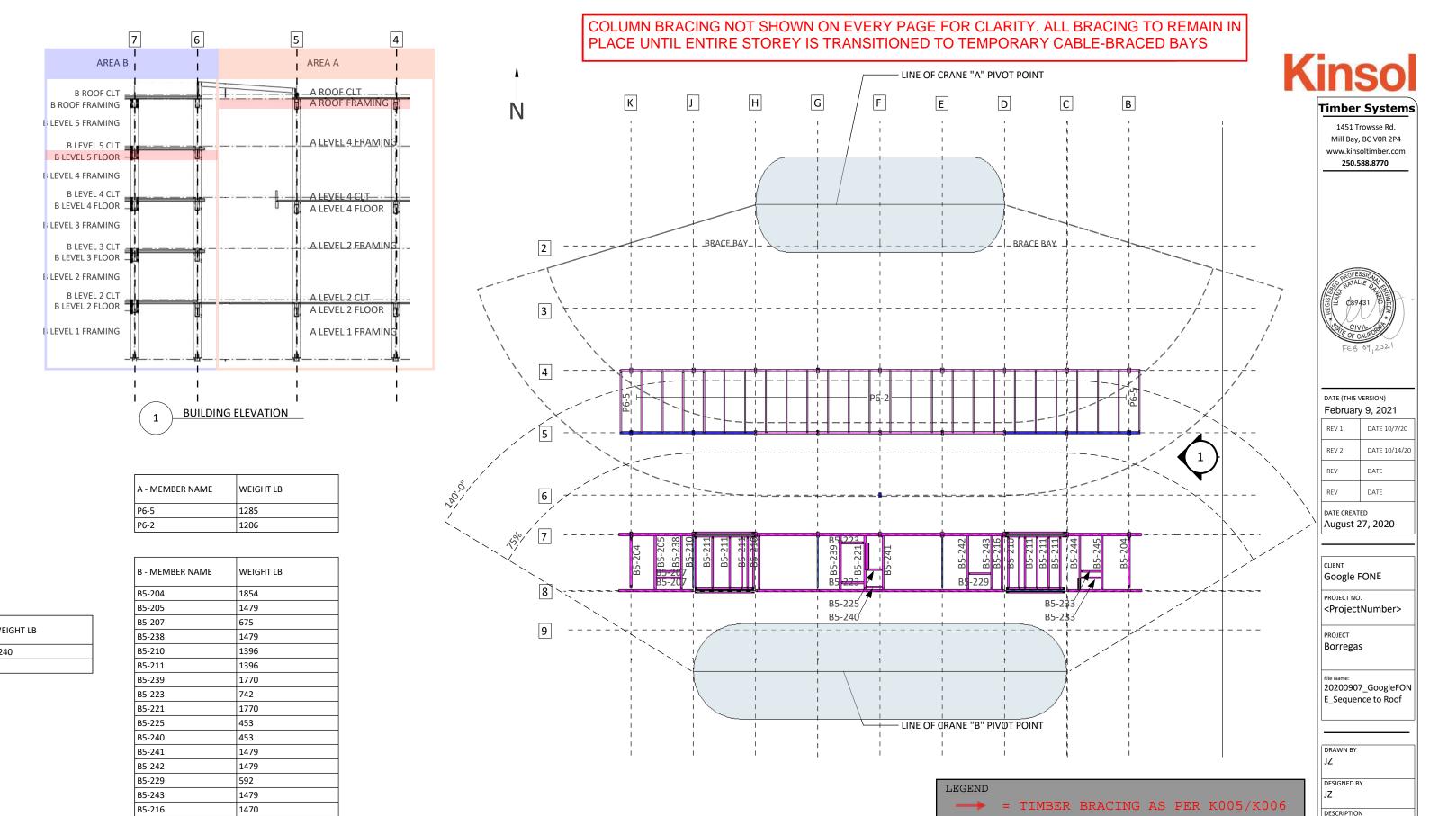
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BUILDING PLAN

K605

A-LVL RF FRM / B-LVL

SCISSOR LIFT DRIVE AISLE

B5-244

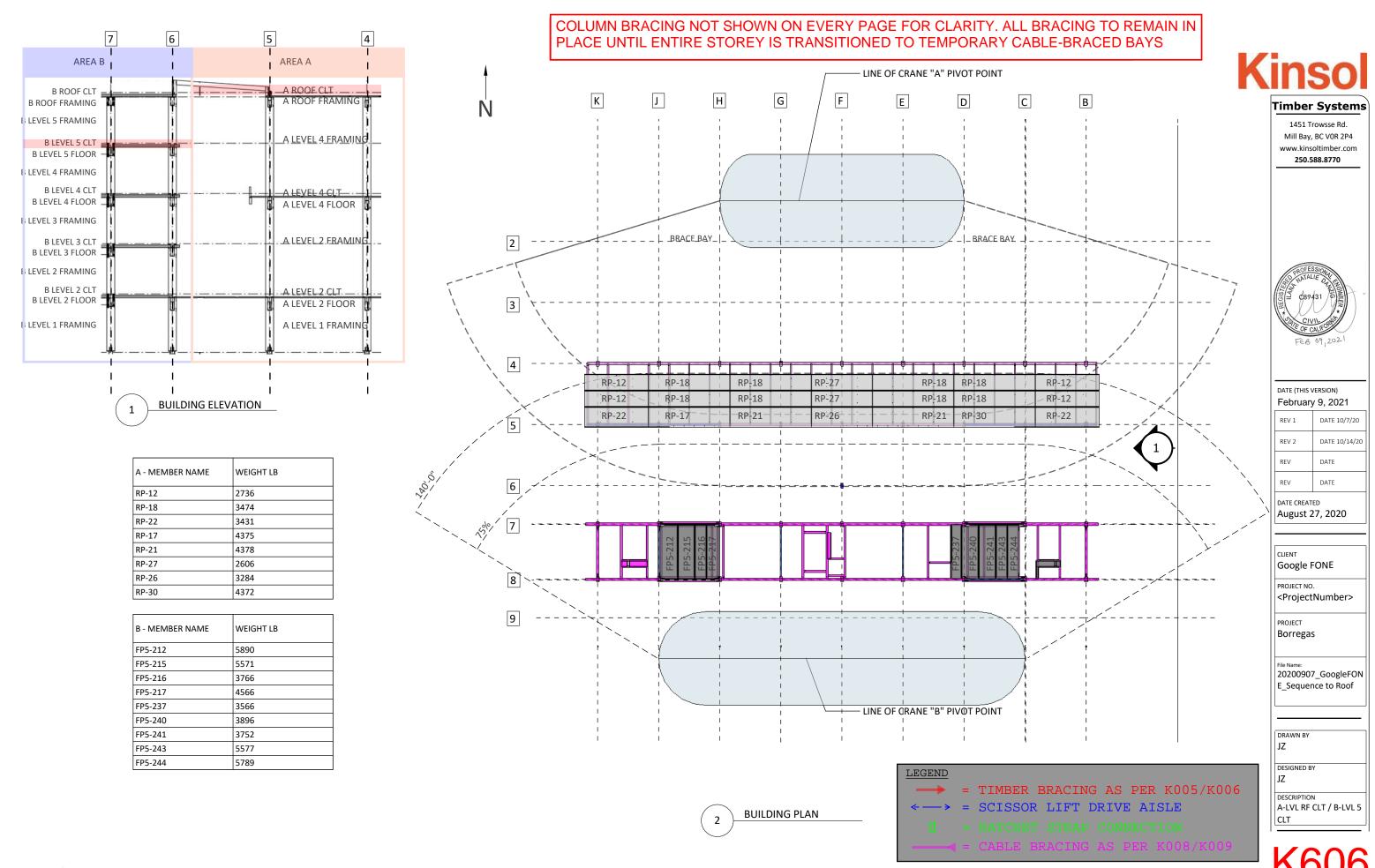
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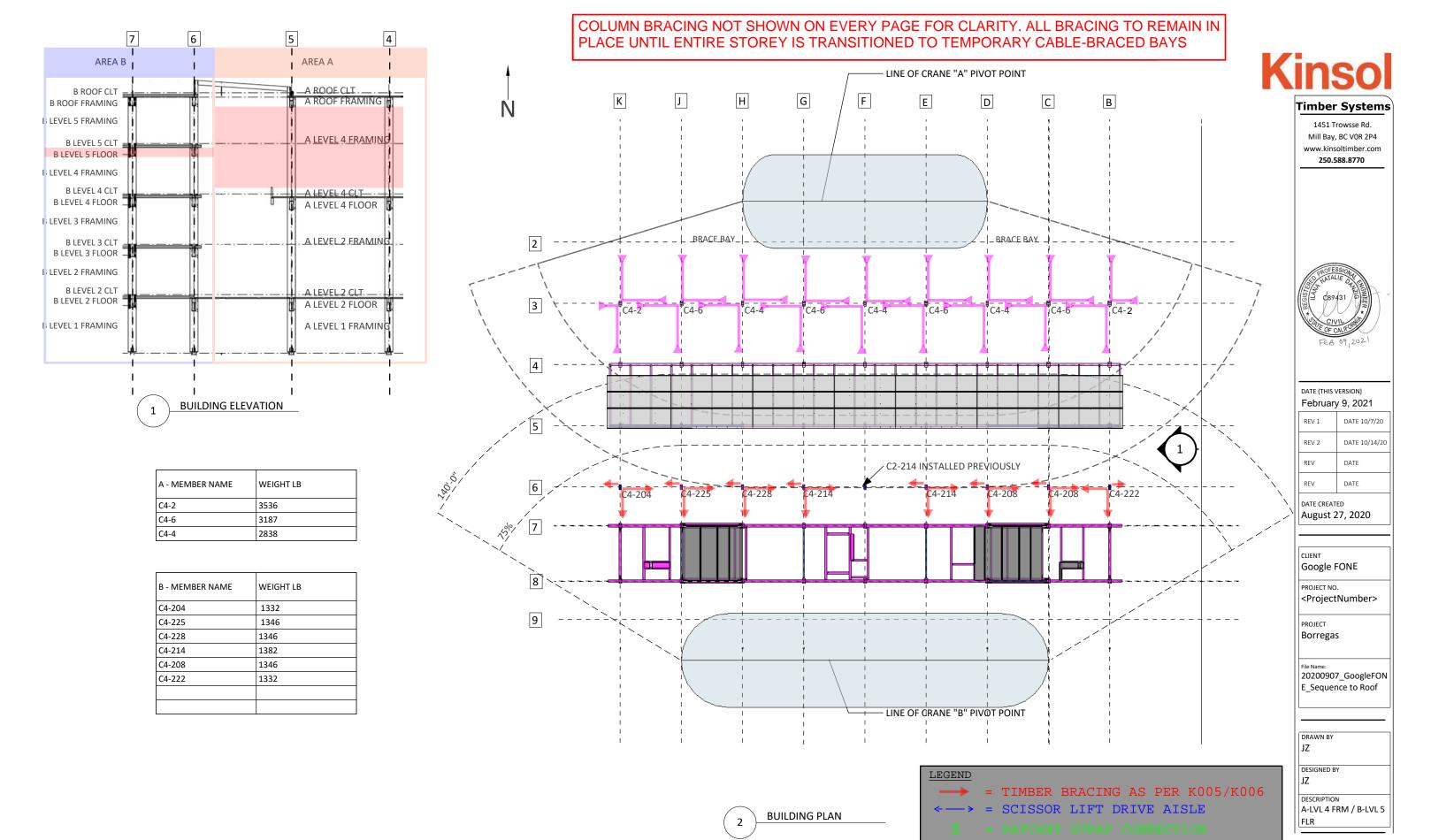
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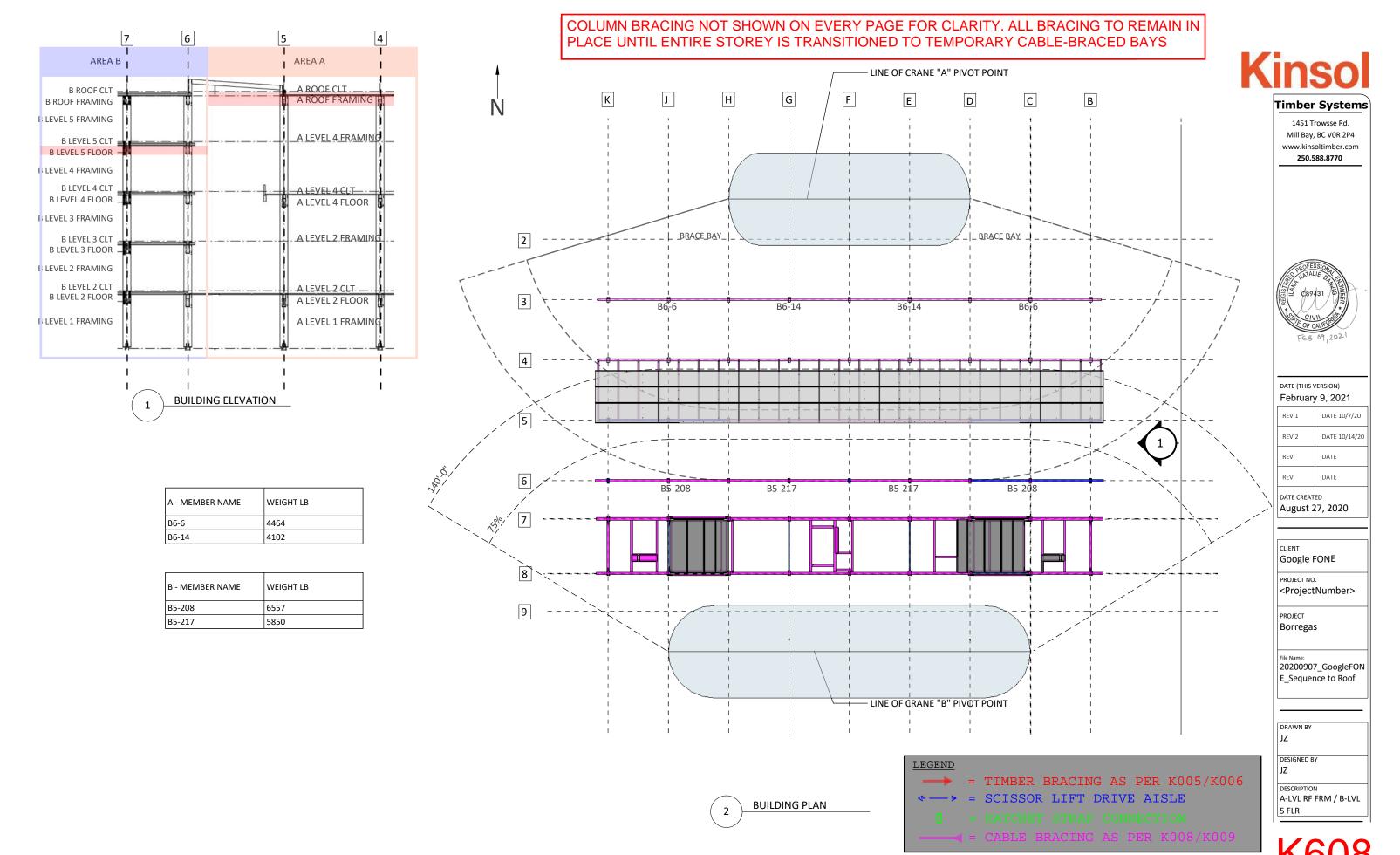
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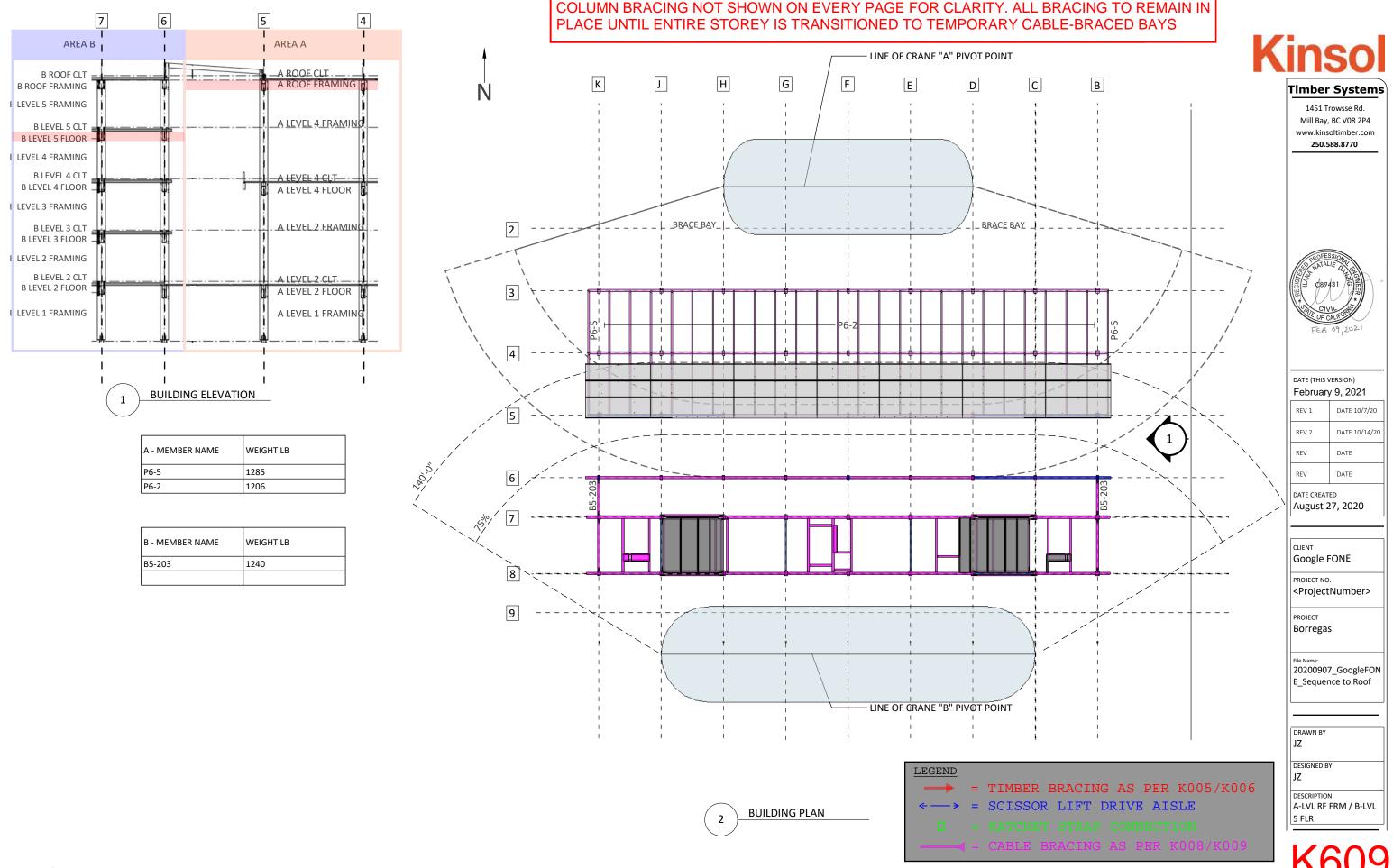
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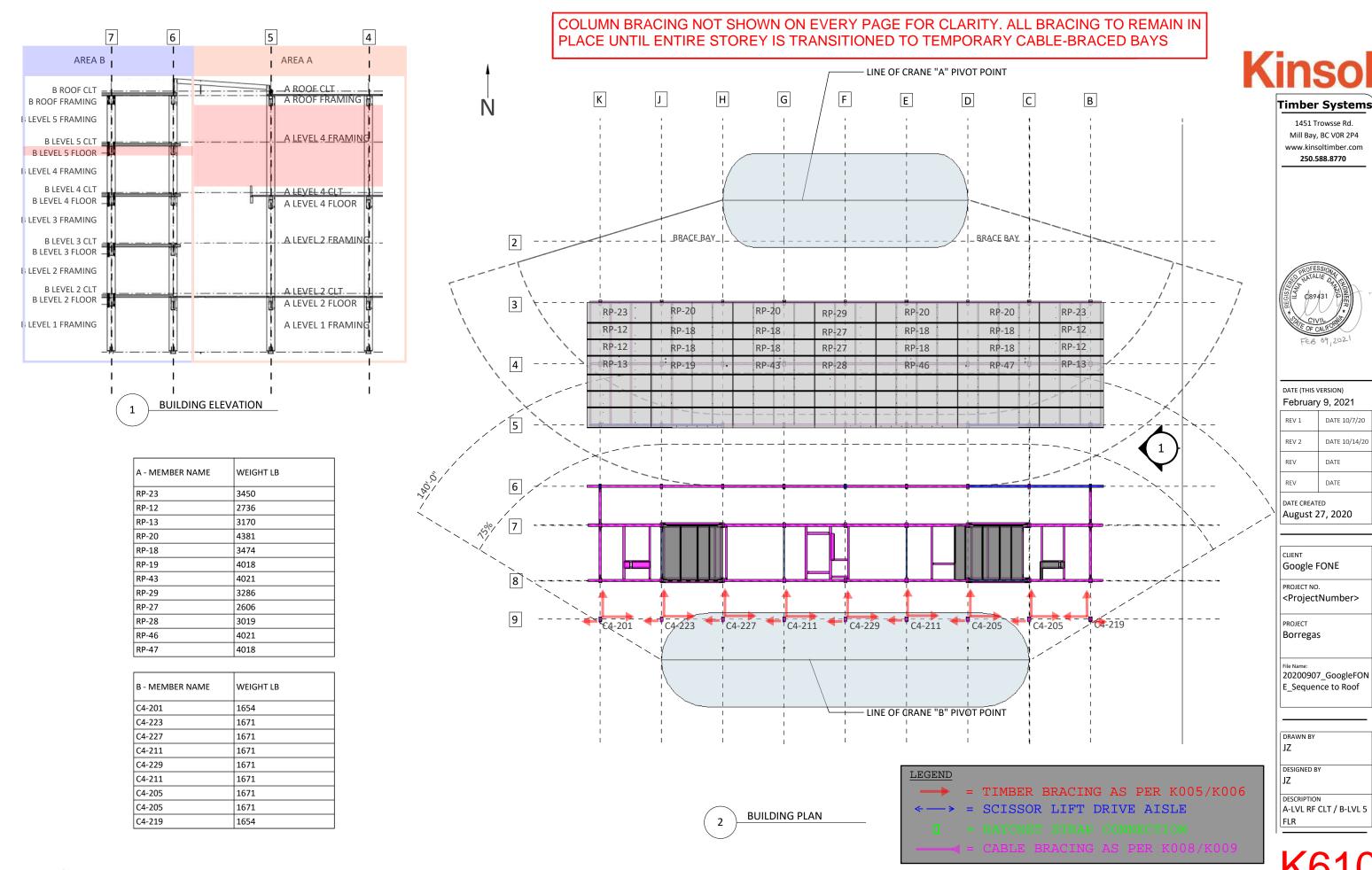
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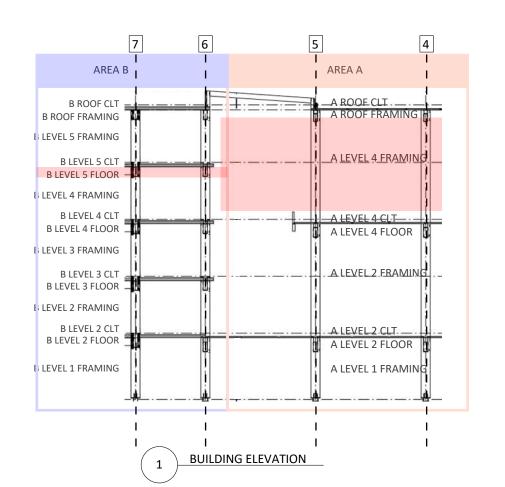




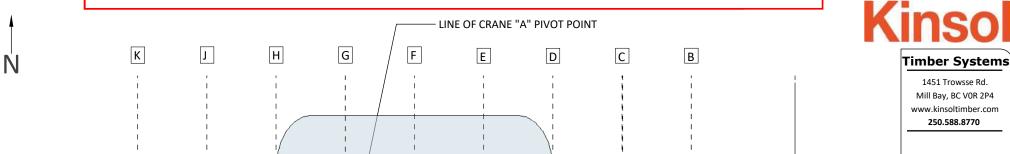








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REV	DATE
REV	DATE
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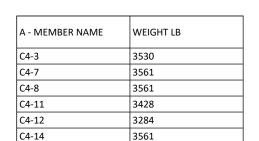
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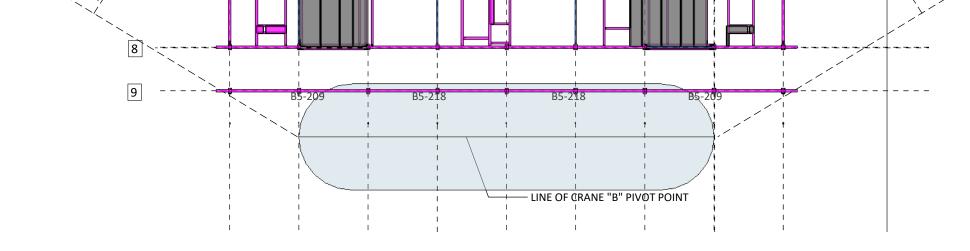
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DESCRIPTION A-LVL RF FRM / B-LVL



B - MEMBER NAME	WEIGHT LB
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R5-218	6196

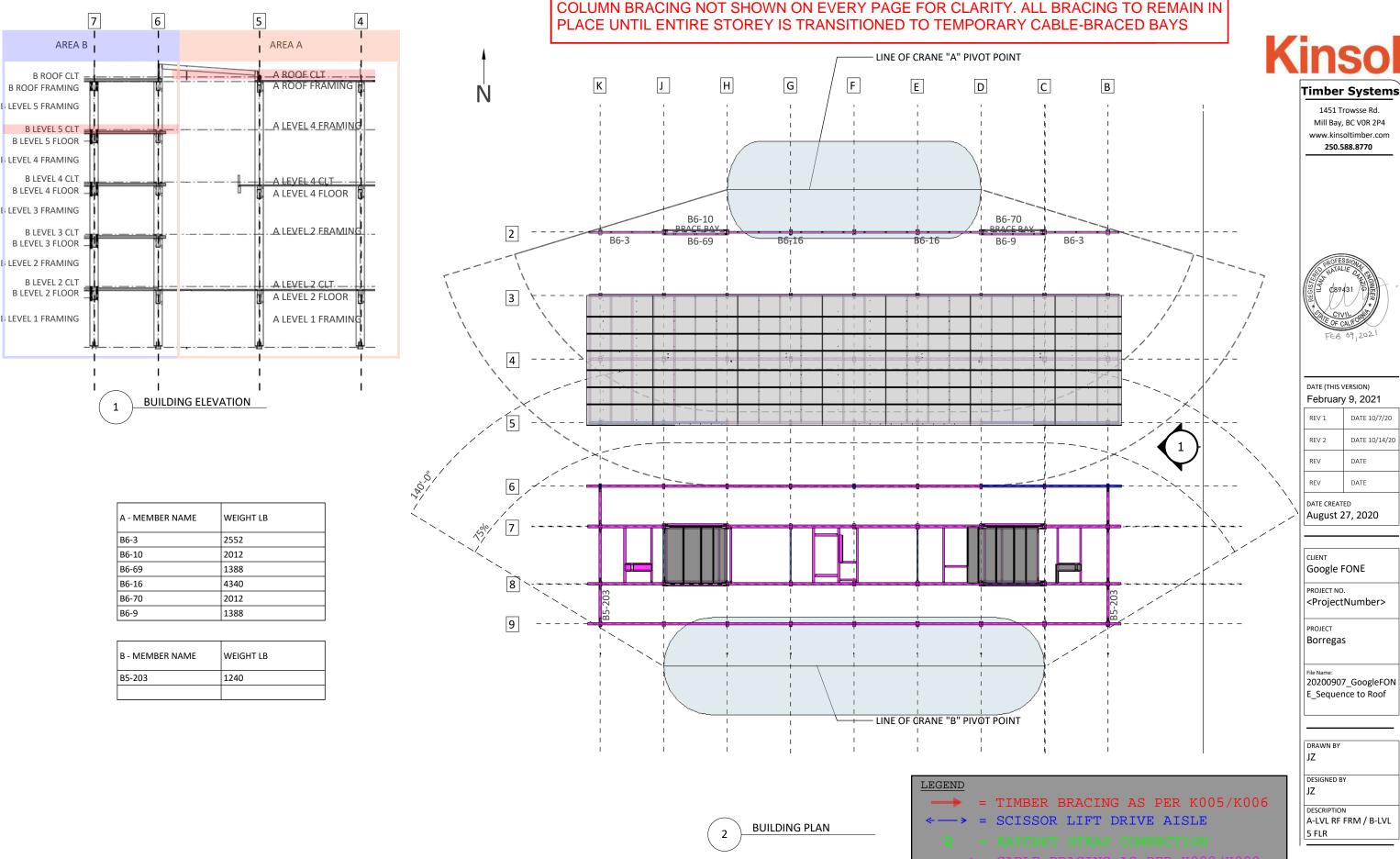


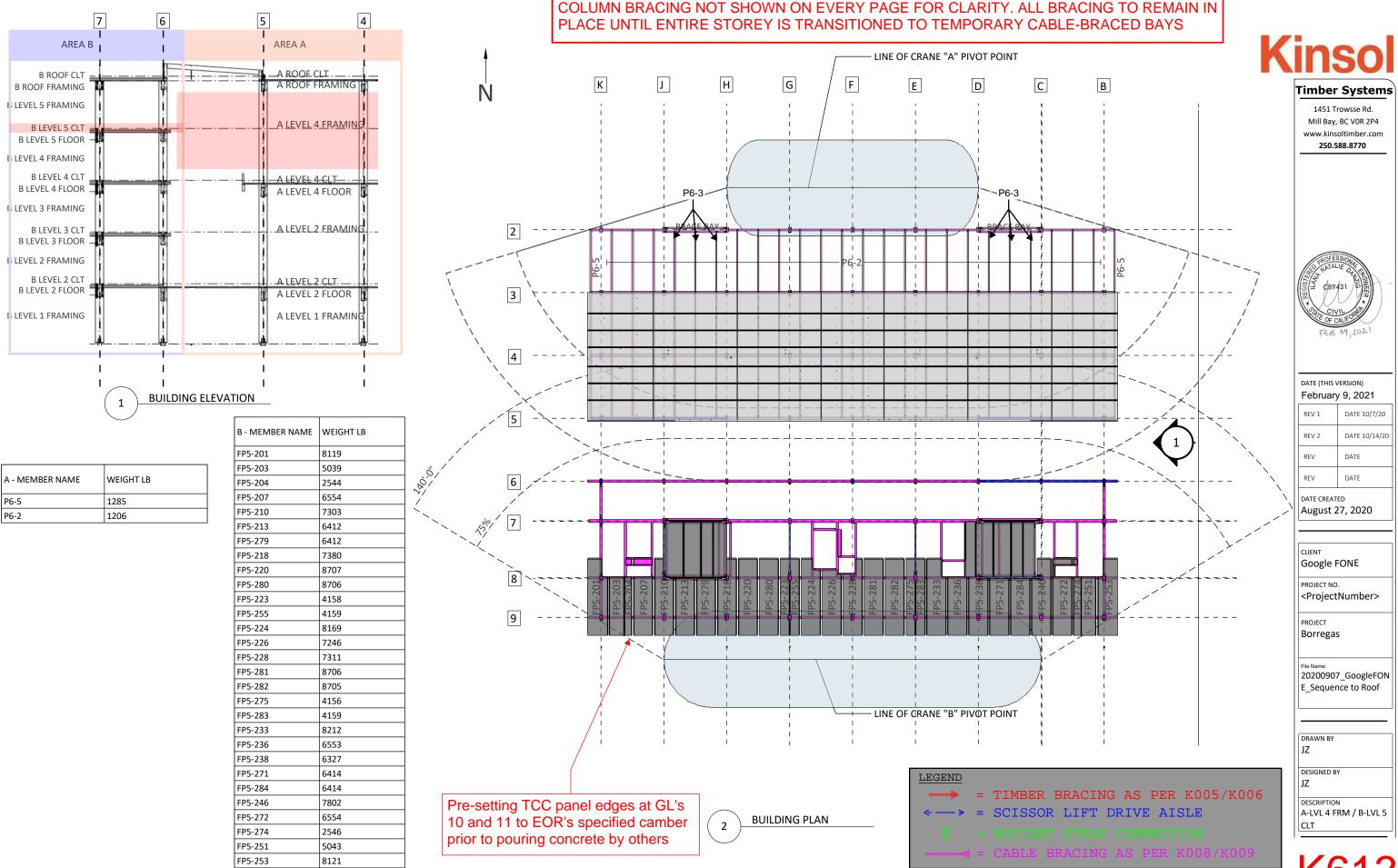
BUILDING PLAN

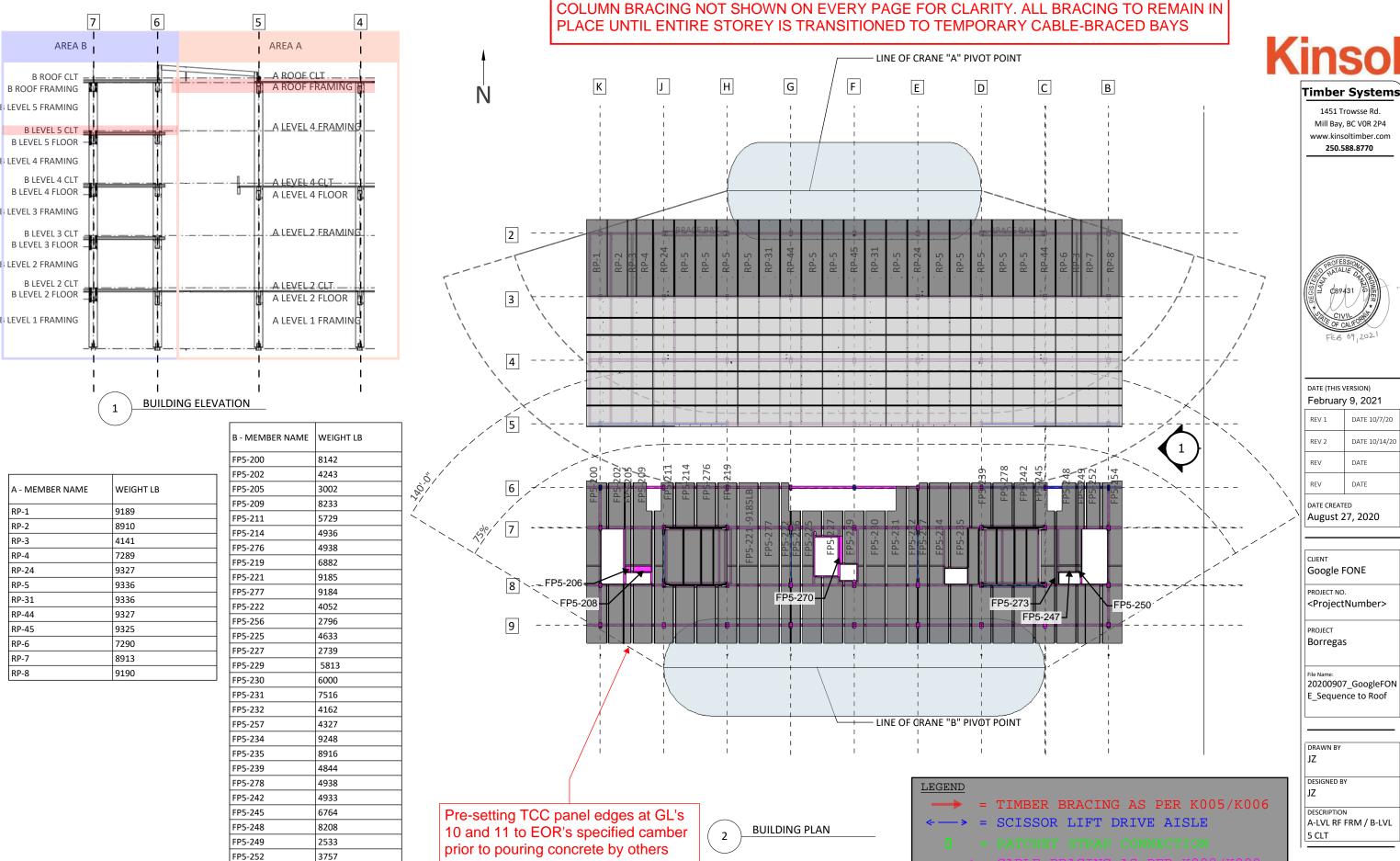
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4

7





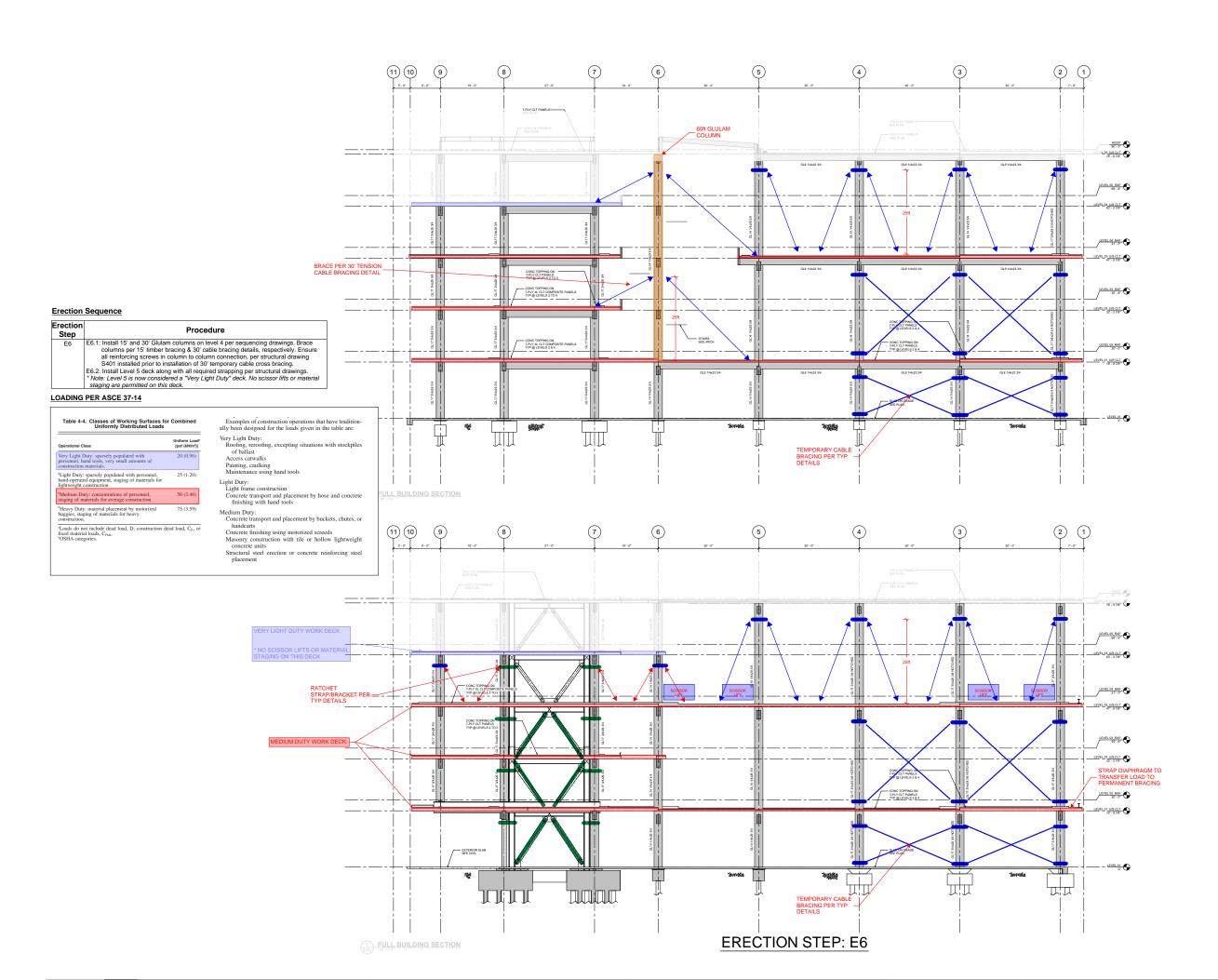


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2020-10-06	Issued for Coordination	
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2021-02-09	Issued for Construction	2

Plot Date:

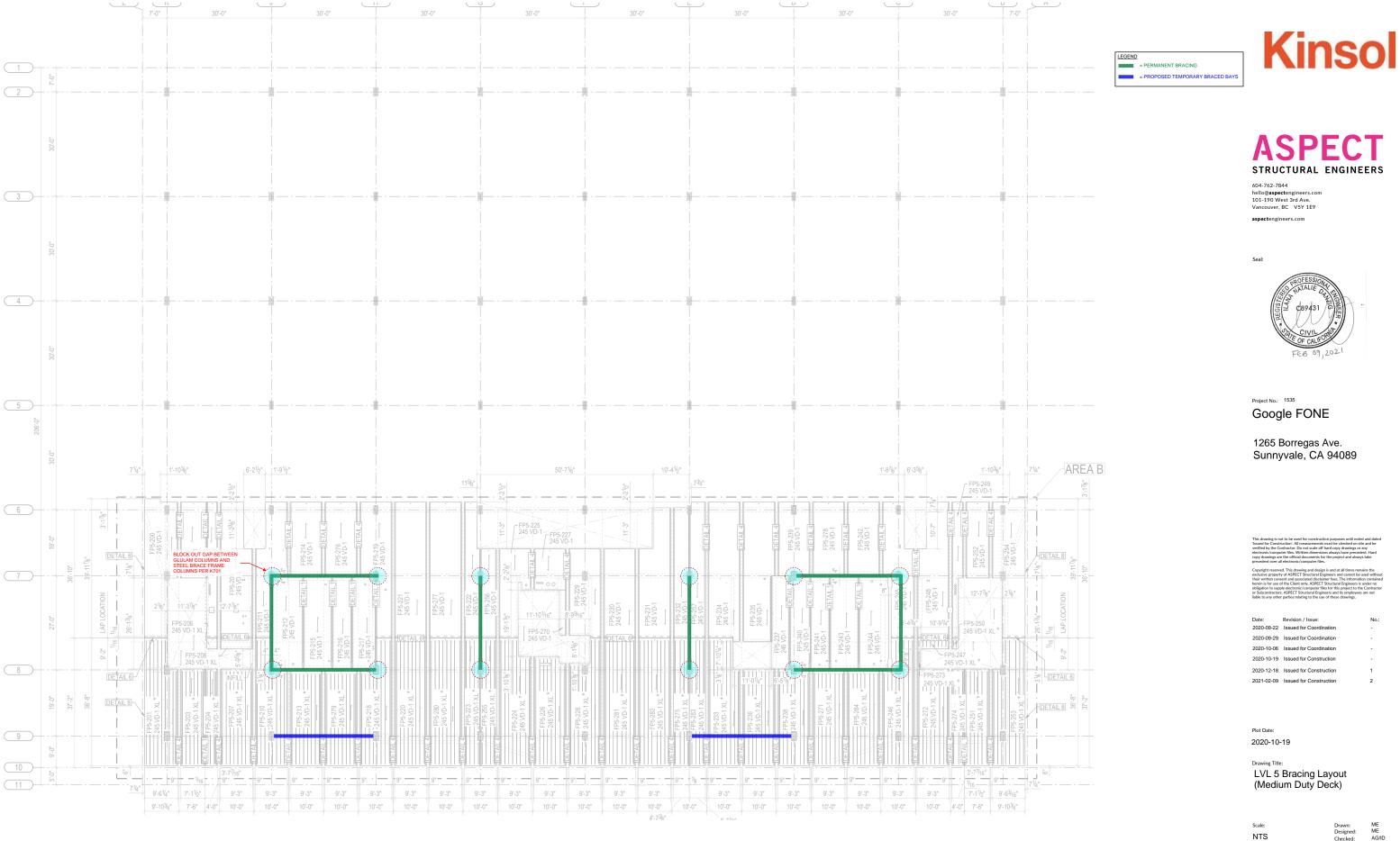
2020-10-19

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Scale: Drawn: ME
Designed: ME
NTS Checked: AC

awing No.: Revision No.

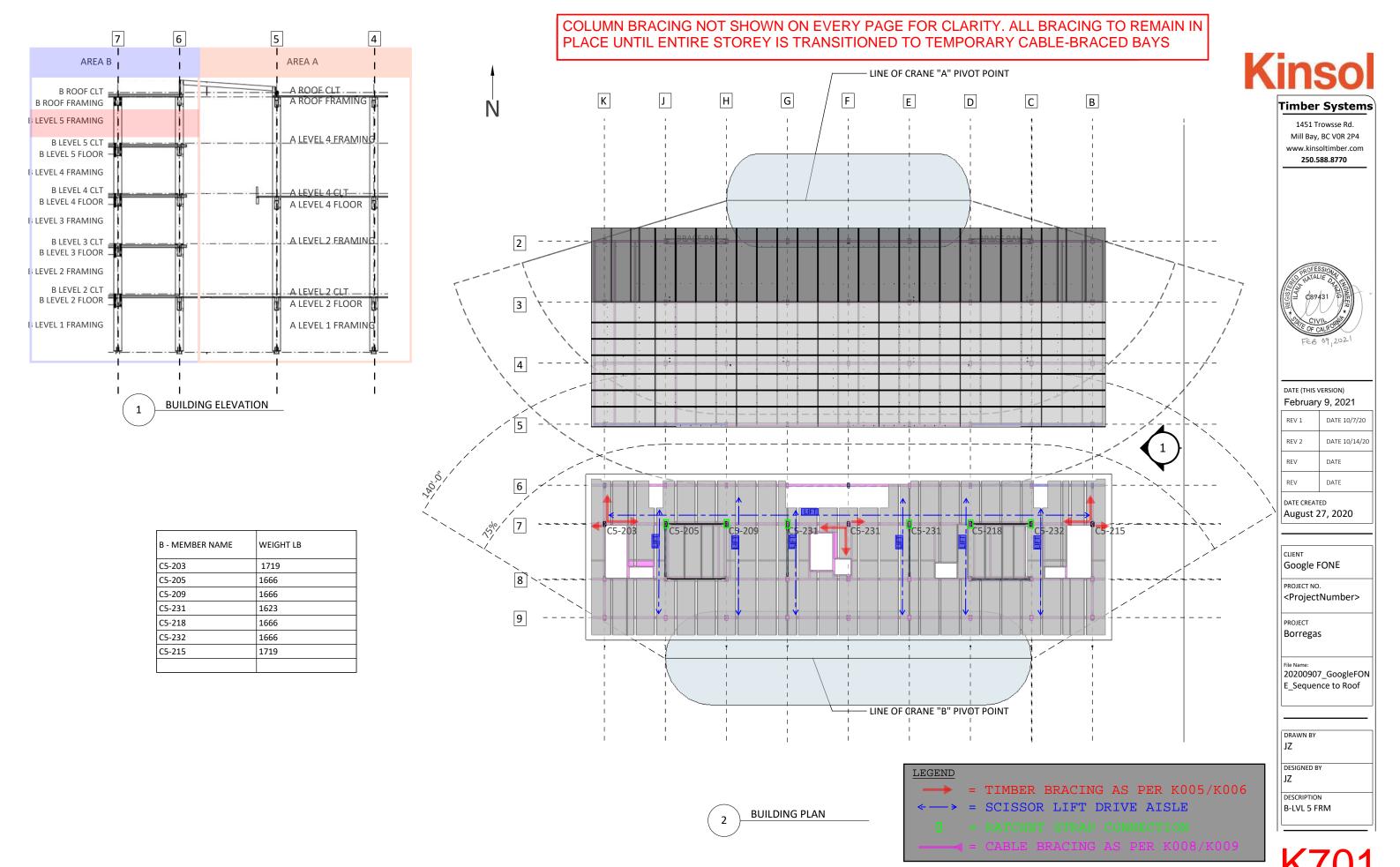


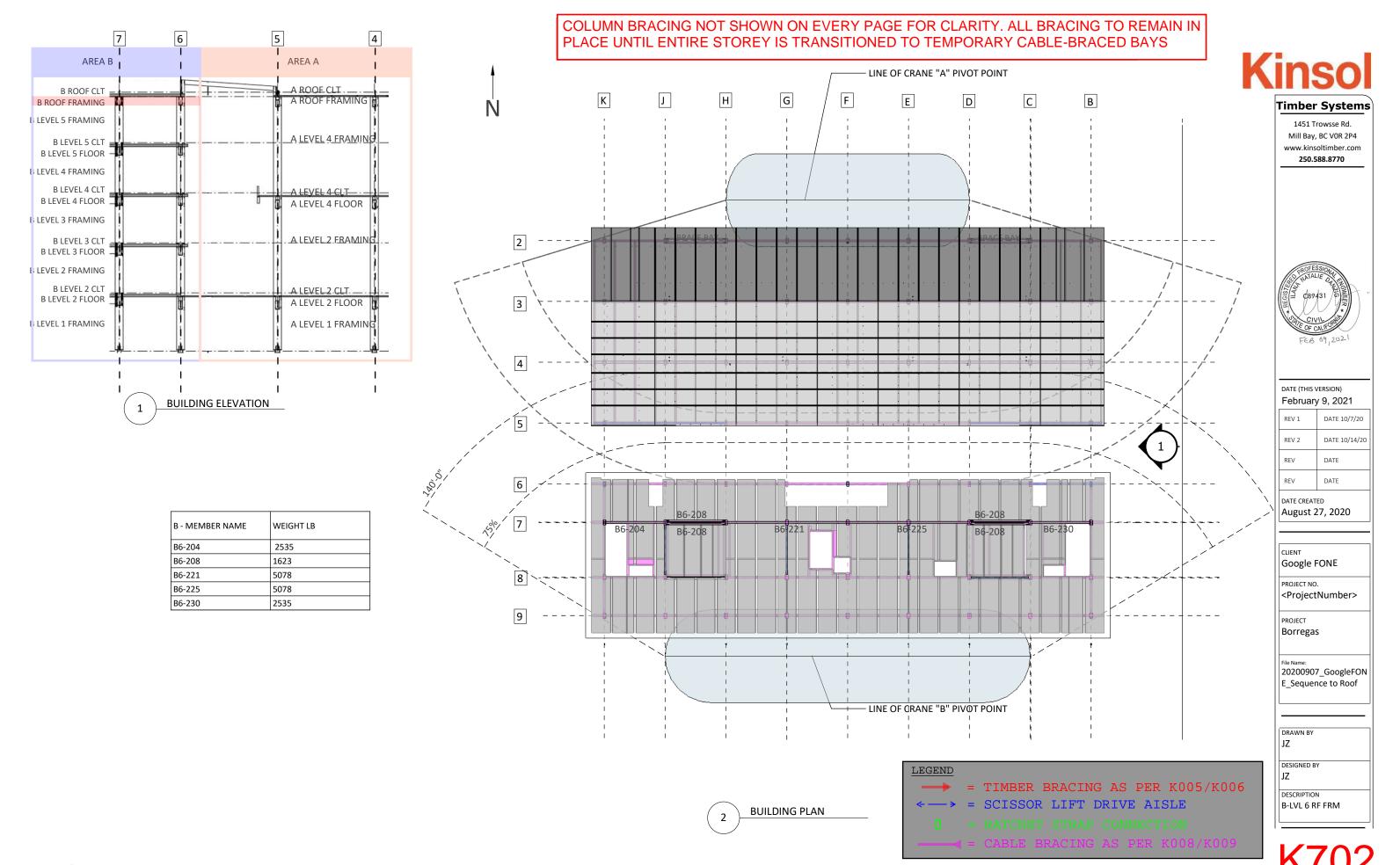
LVL 5 BRACING LAYOUT PLAN (MEDIUM DUTY DECK)

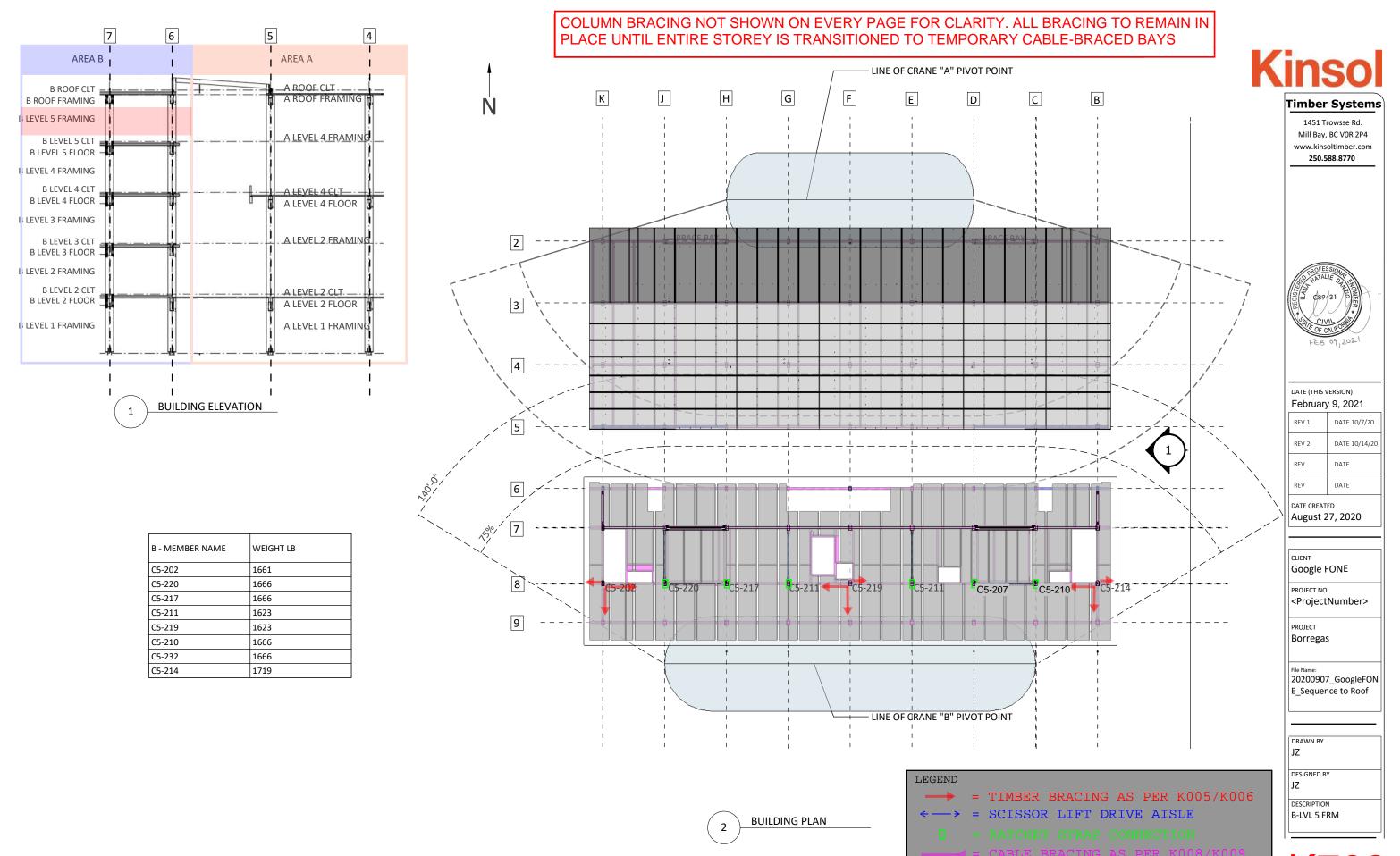
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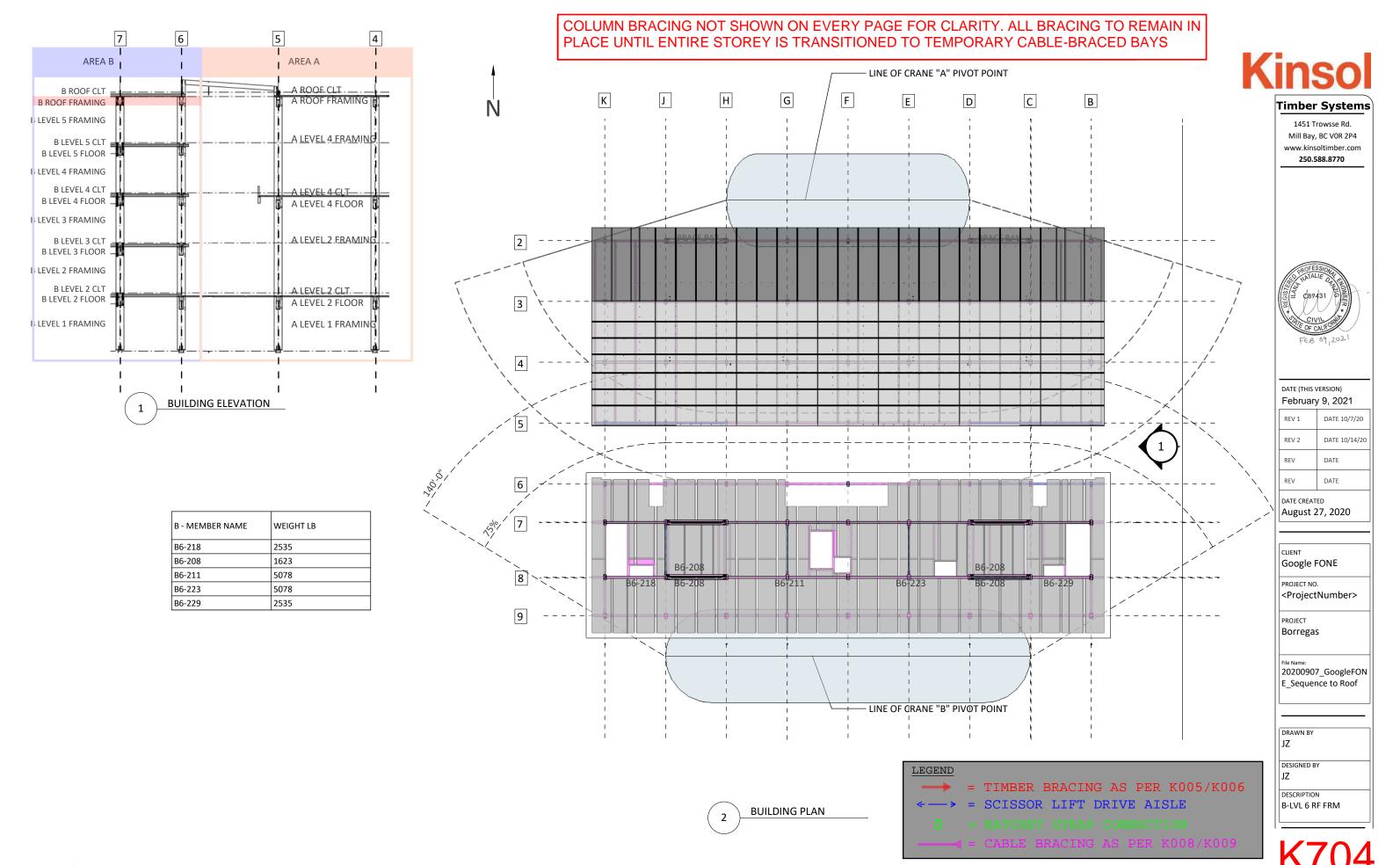
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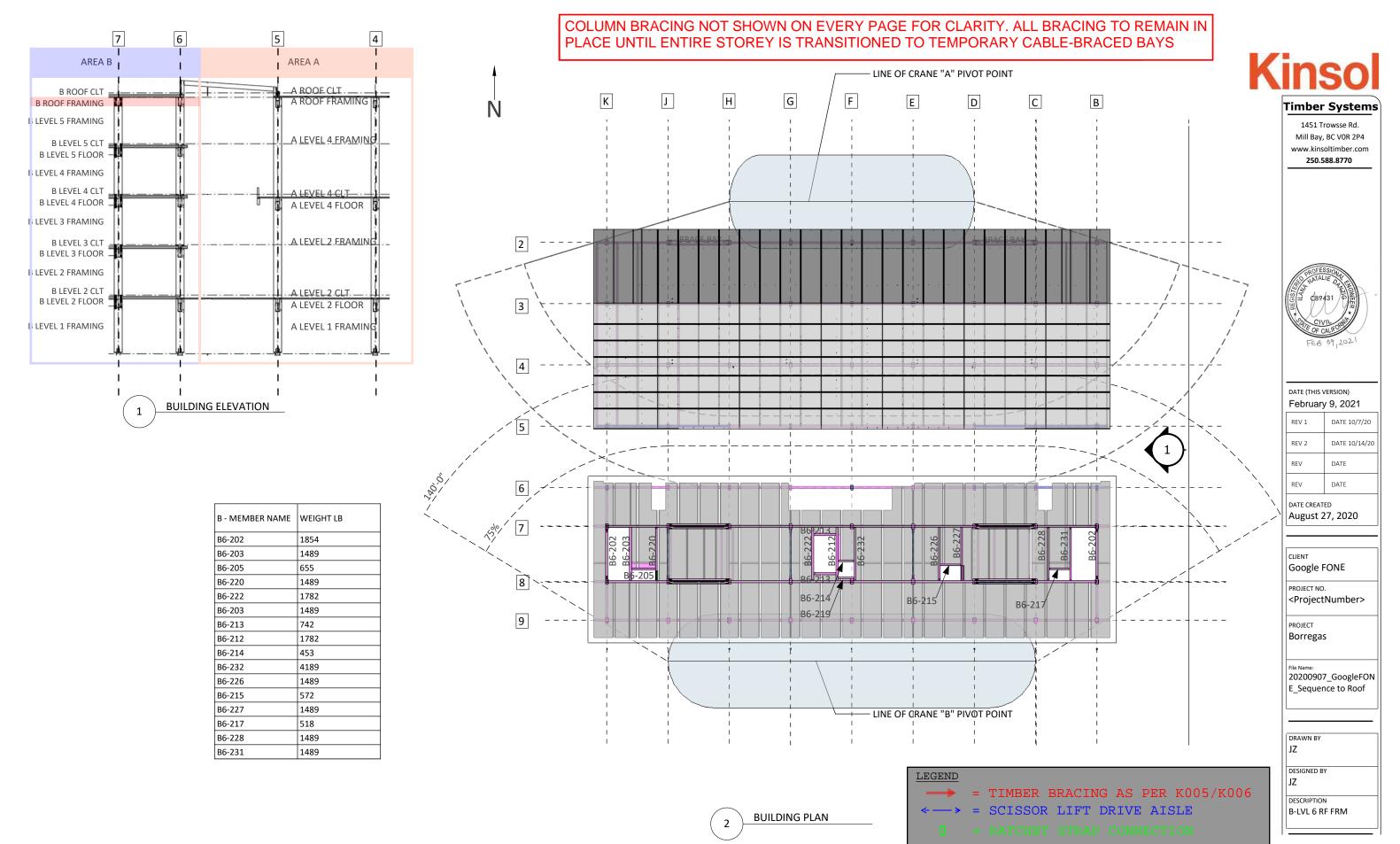
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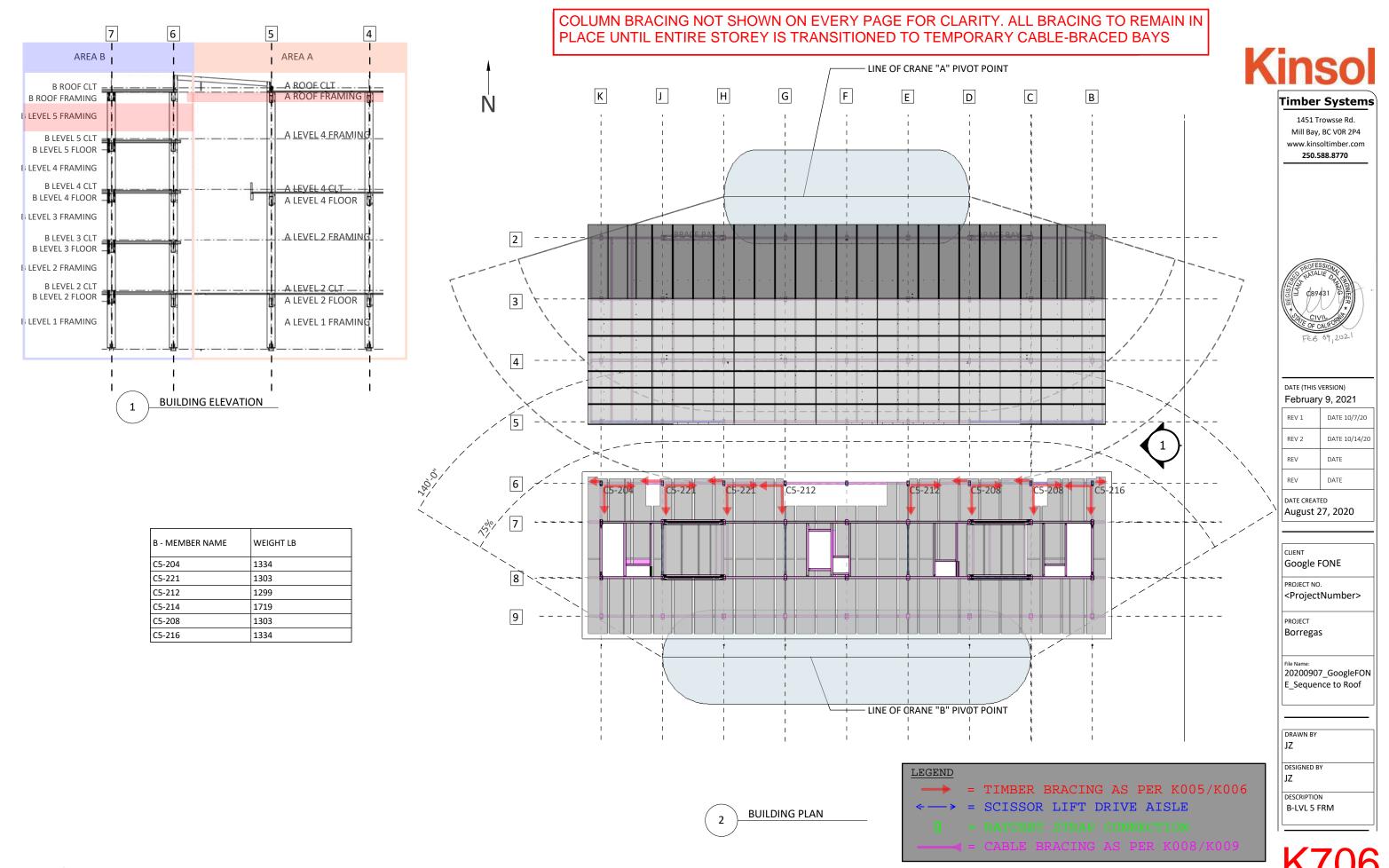


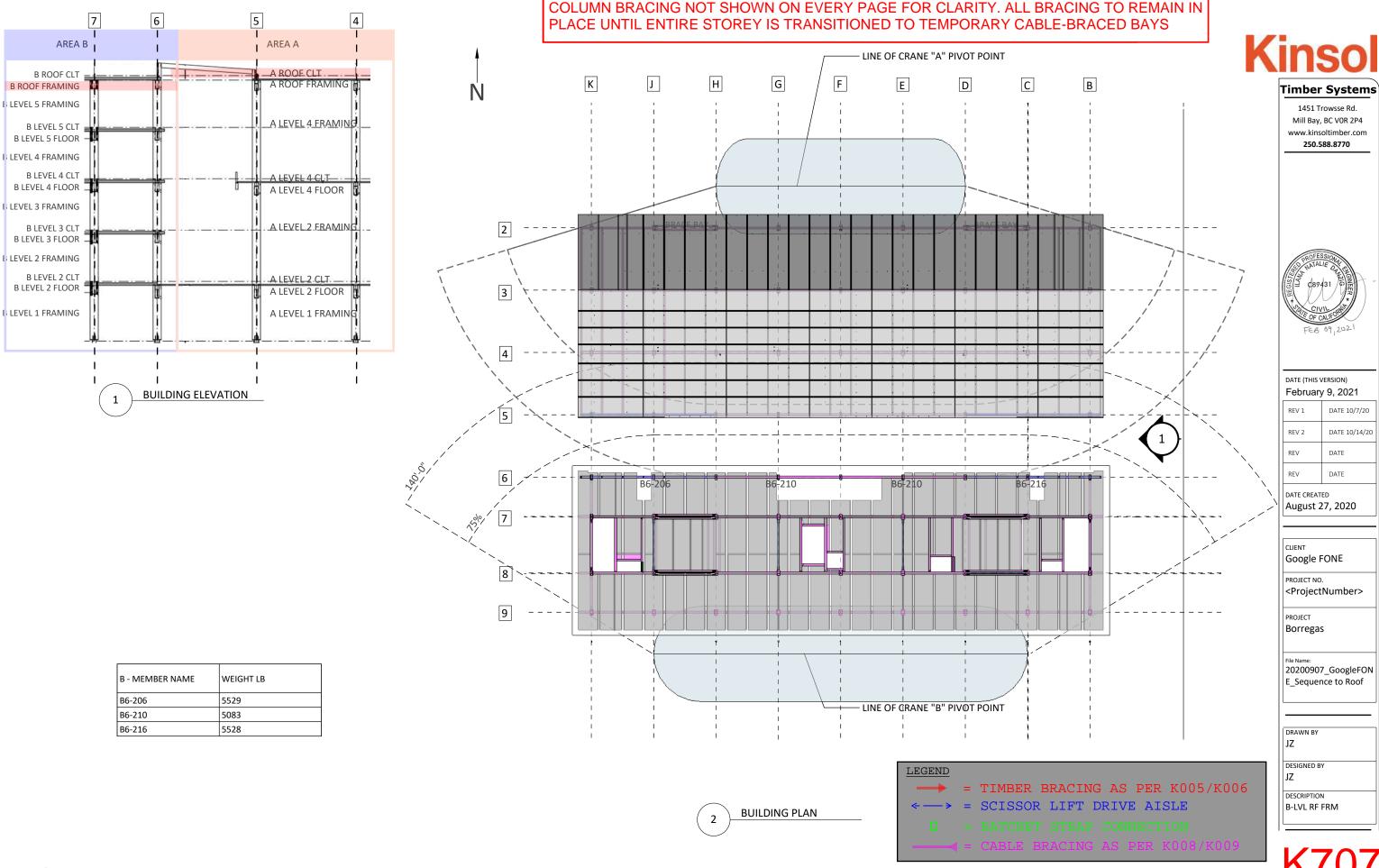


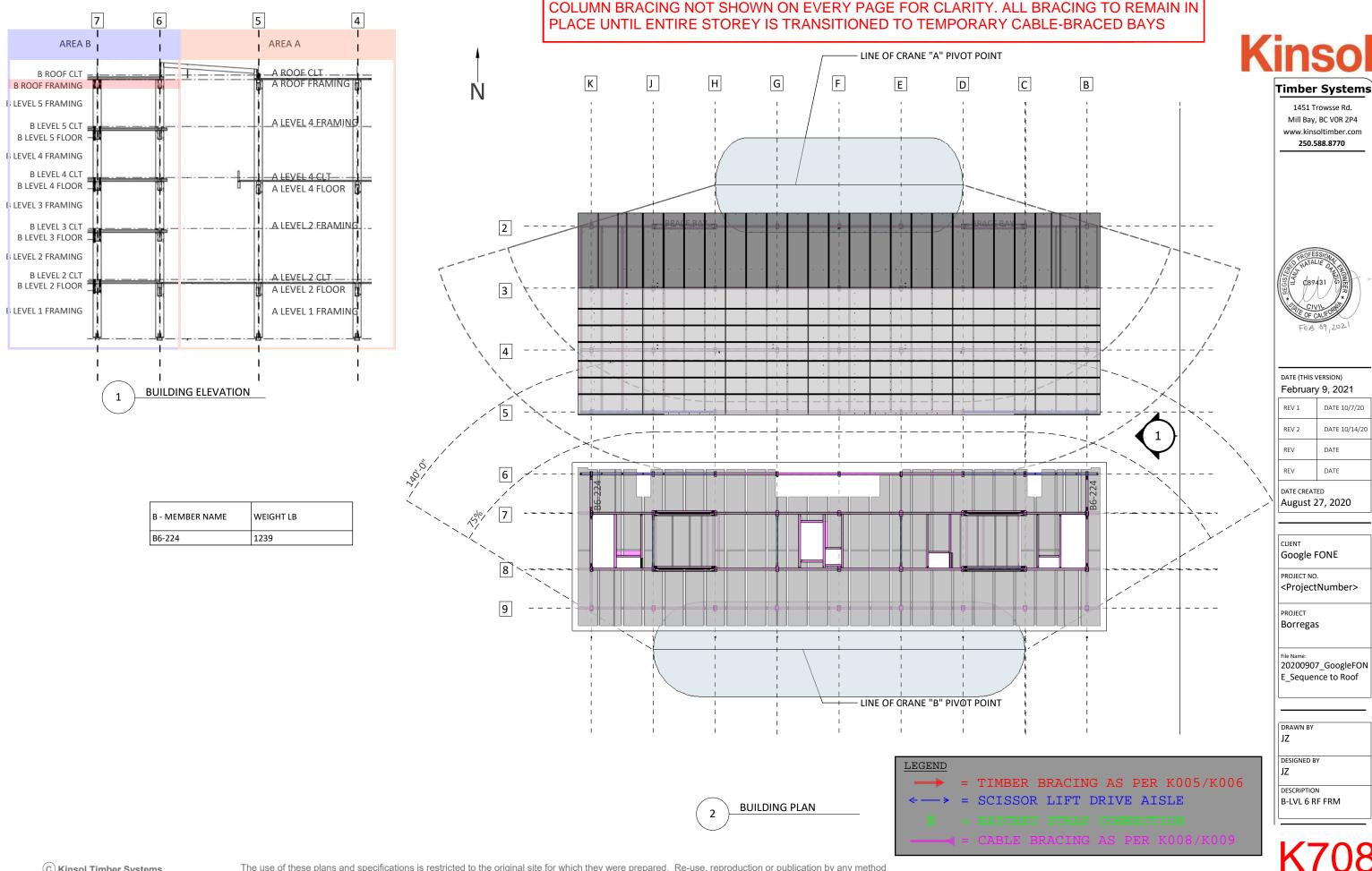


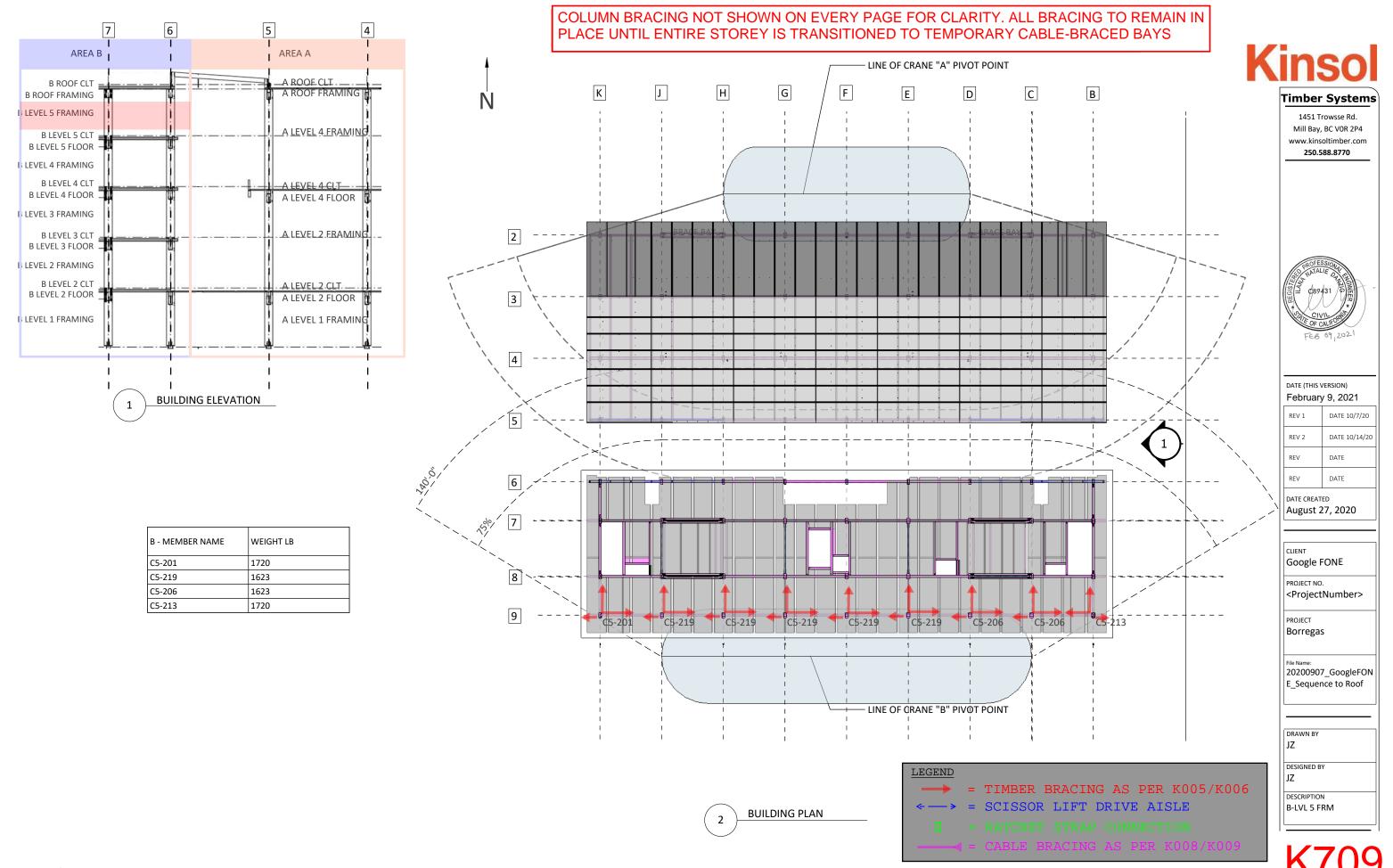


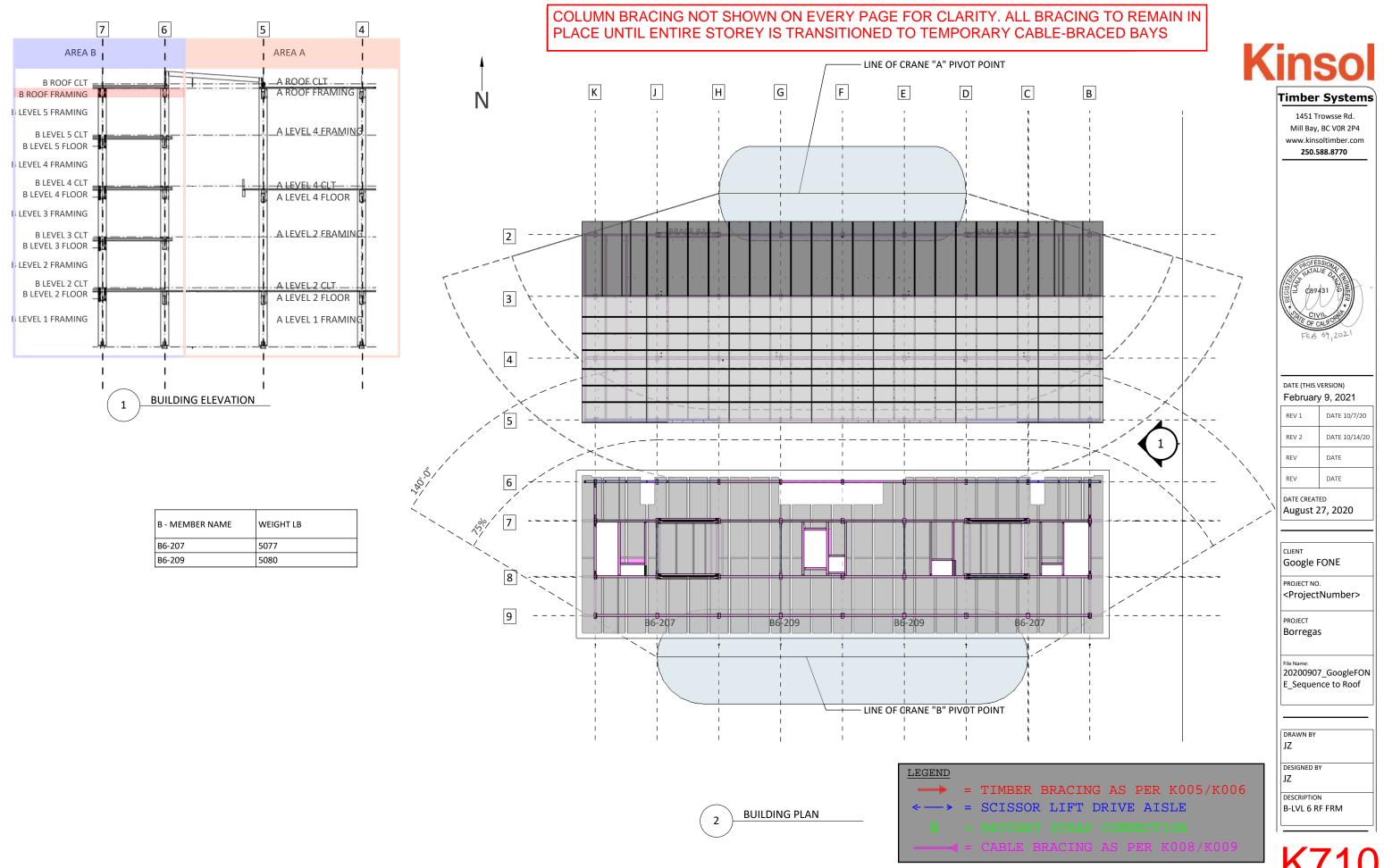


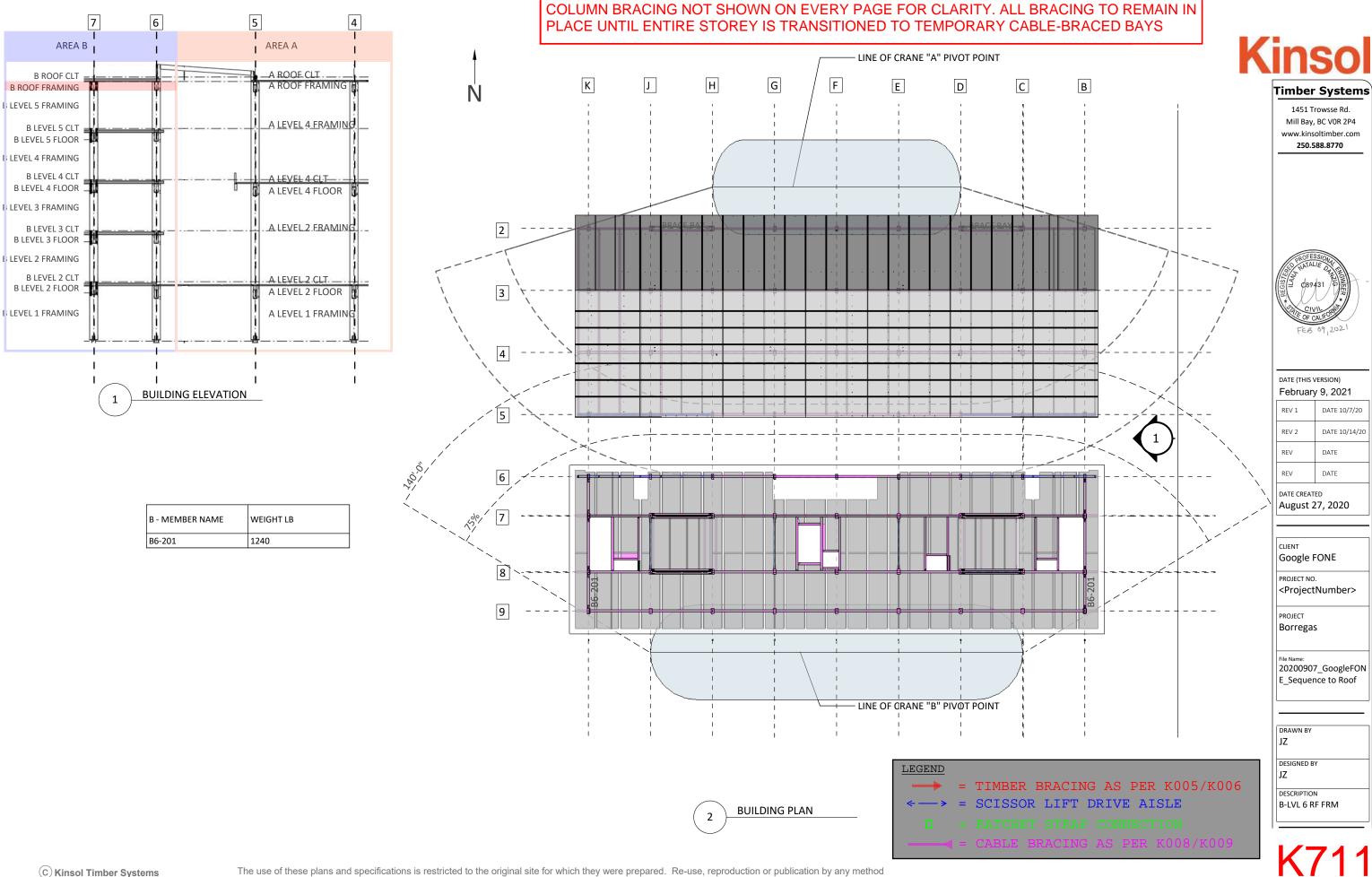


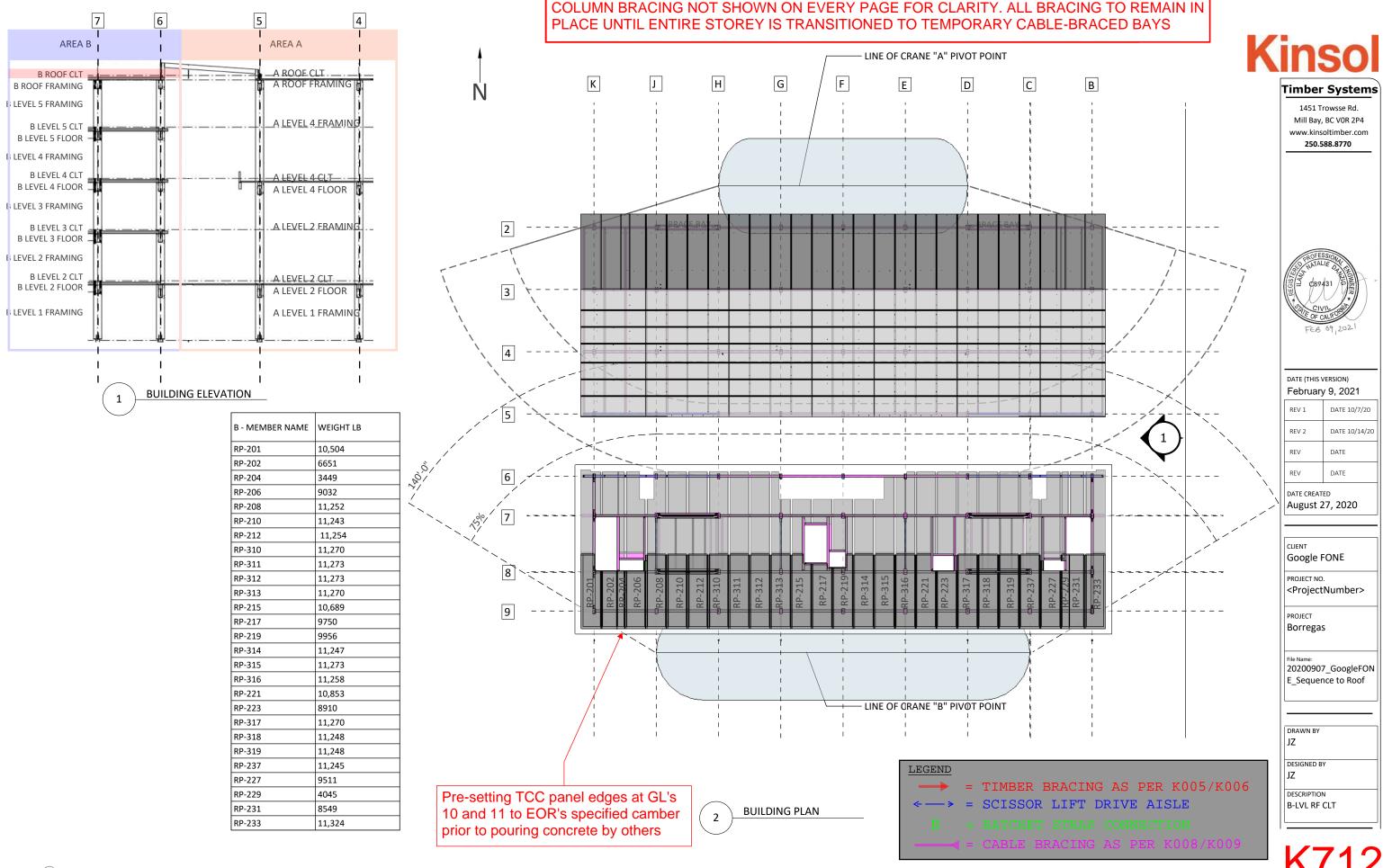


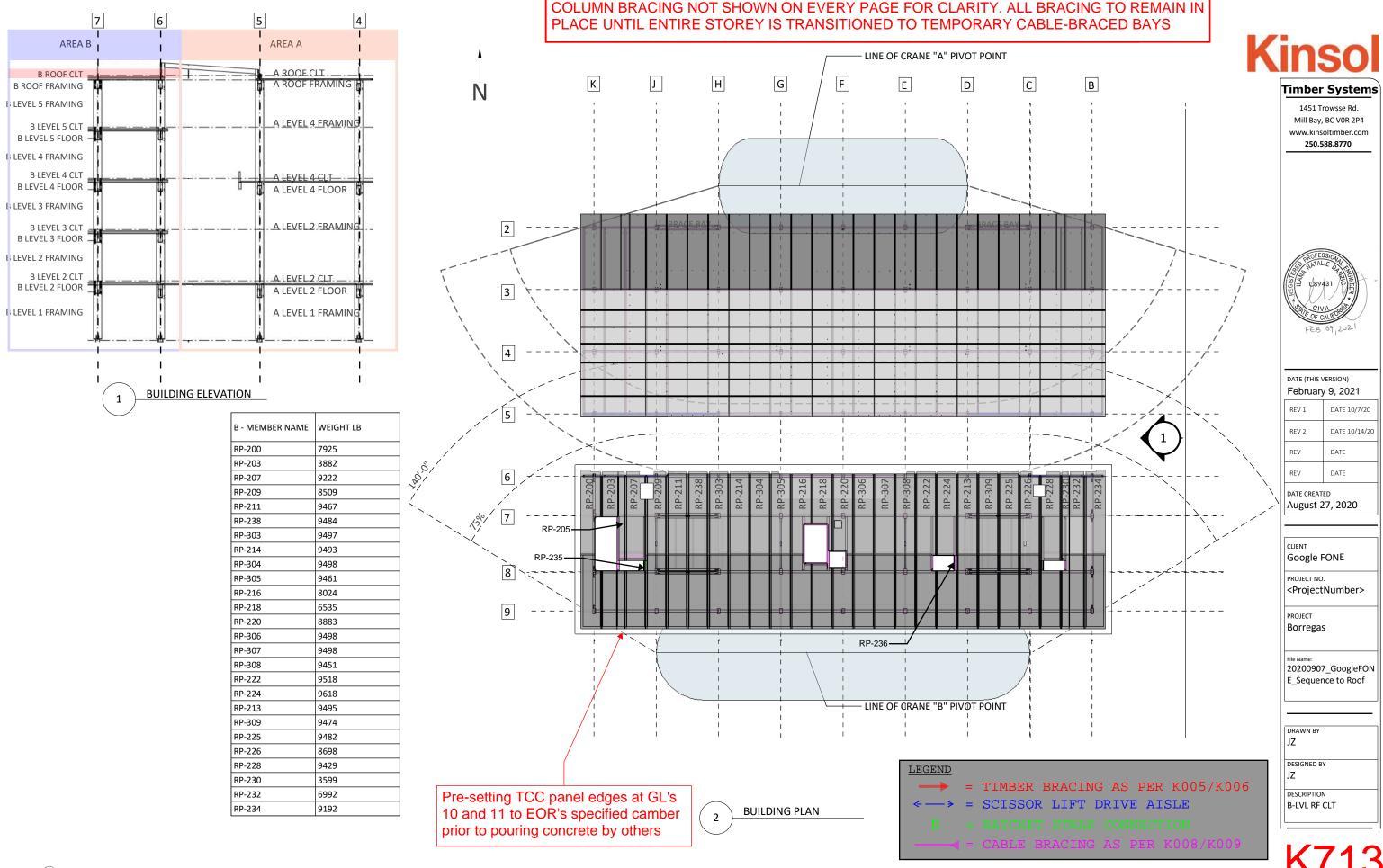


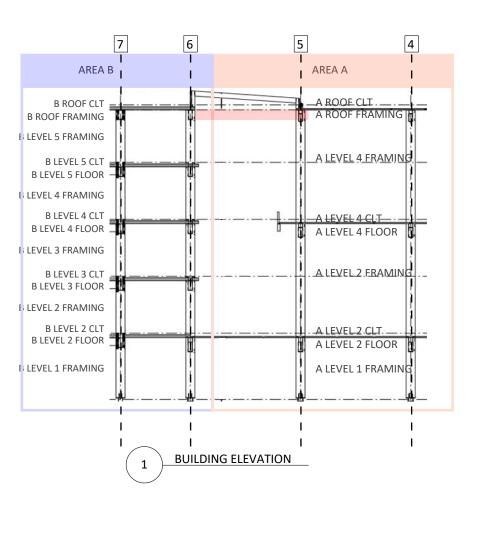






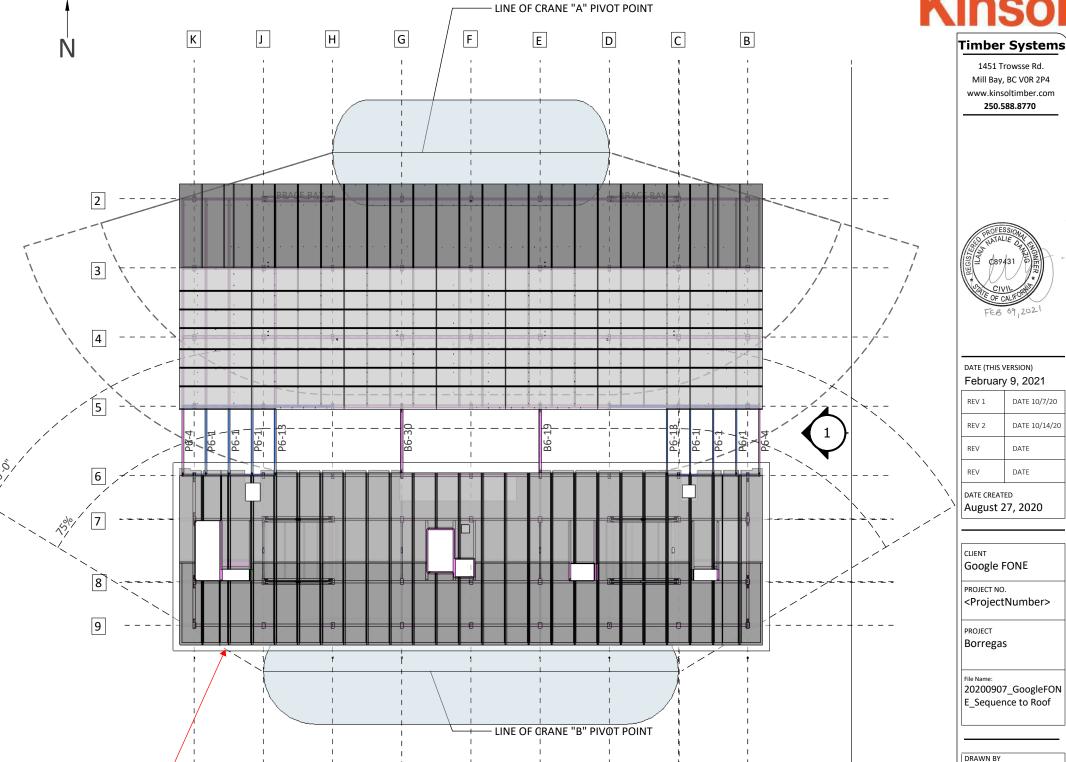






B - MEMBER NAME	WEIGHT LB
P6-4	1270
P6-1	1191
P6-13	1191
B6-30	2248
B6-19	2248

COLUMN BRACING NOT SHOWN ON EVERY PAGE FOR CLARITY. ALL BRACING TO REMAIN IN PLACE UNTIL ENTIRE STOREY IS TRANSITIONED TO TEMPORARY CABLE-BRACED BAYS



Pre-setting TCC panel edges at GL's 10 and 11 to EOR's specified camber prior to pouring concrete by others

BUILDING PLAN 2

= TIMBER BRACING AS PER K005/K006

SCISSOR LIFT DRIVE AISLE

LEGEND

1451 Trowsse Rd. Mill Bay, BC VOR 2P4

www.kinsoltimber.com

250.588.8770

DATE 10/7/20 DATE 10/14/20

DATE DATE

REV

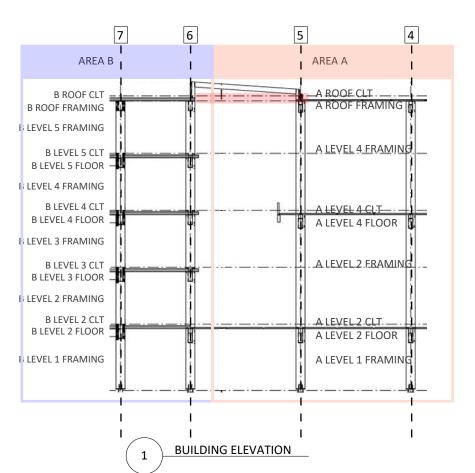
REV

JΖ

DESIGNED BY

DESCRIPTION

A/B LVL RF FRM



B - MEMBER NAME	WEIGHT LB
RP-11	2045
RP-10	2342
RP-9	2355
RP-16	1930
RP-15	2210
RP-14	2222
RP-50	2045
RP-49	2342
RP-48	2355
·	

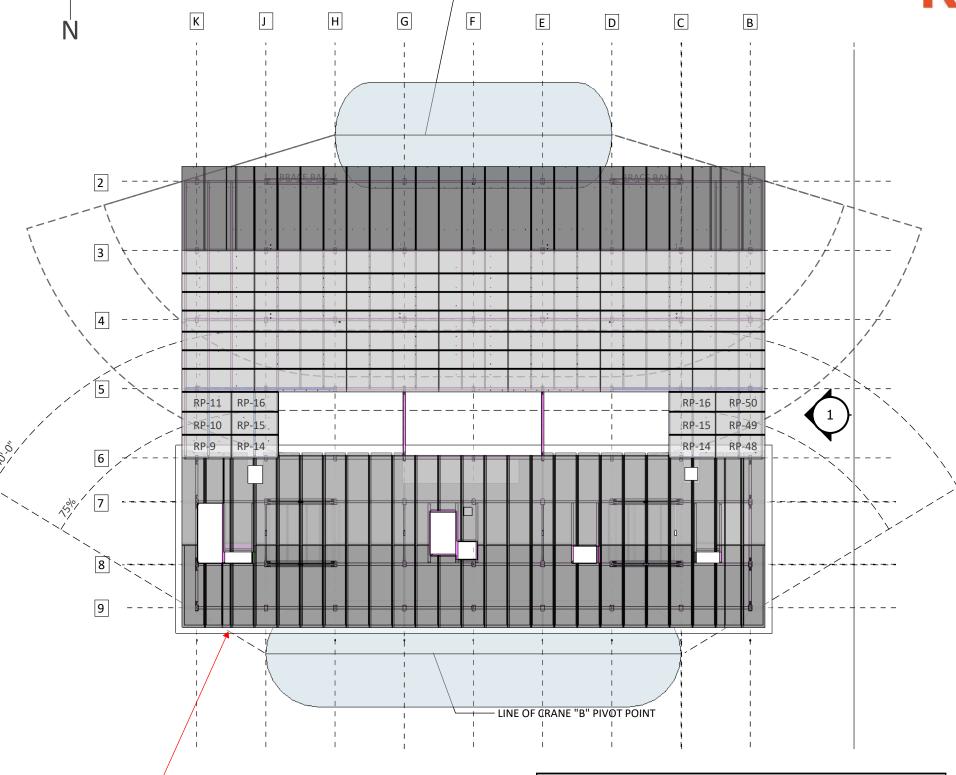
COLUMN BRACING NOT SHOWN ON EVERY PAGE FOR CLARITY. ALL BRACING TO REMAIN IN PLACE UNTIL ENTIRE STOREY IS TRANSITIONED TO TEMPORARY CABLE-BRACED BAYS

- LINE OF CRANE "A" PIVOT POINT

LEGEND

= TIMBER BRACING AS PER K005/K006

SCISSOR LIFT DRIVE AISLE



BUILDING PLAN

2

K715

Timber Systems

1451 Trowsse Rd. Mill Bay, BC VOR 2P4

www.kinsoltimber.com

250.588.8770

DATE (THIS VERSION)
February 9, 2021

REV

REV

DATE CREATED
August 27, 2020

Google FONE

PROJECT NO.

<ProjectNumber>

20200907_GoogleFON E_Sequence to Roof

PROJECT Borregas

DRAWN BY

DESIGNED BY

DESCRIPTION

A/B LVL RF CLT

DATE 10/7/20

DATE 10/14/20

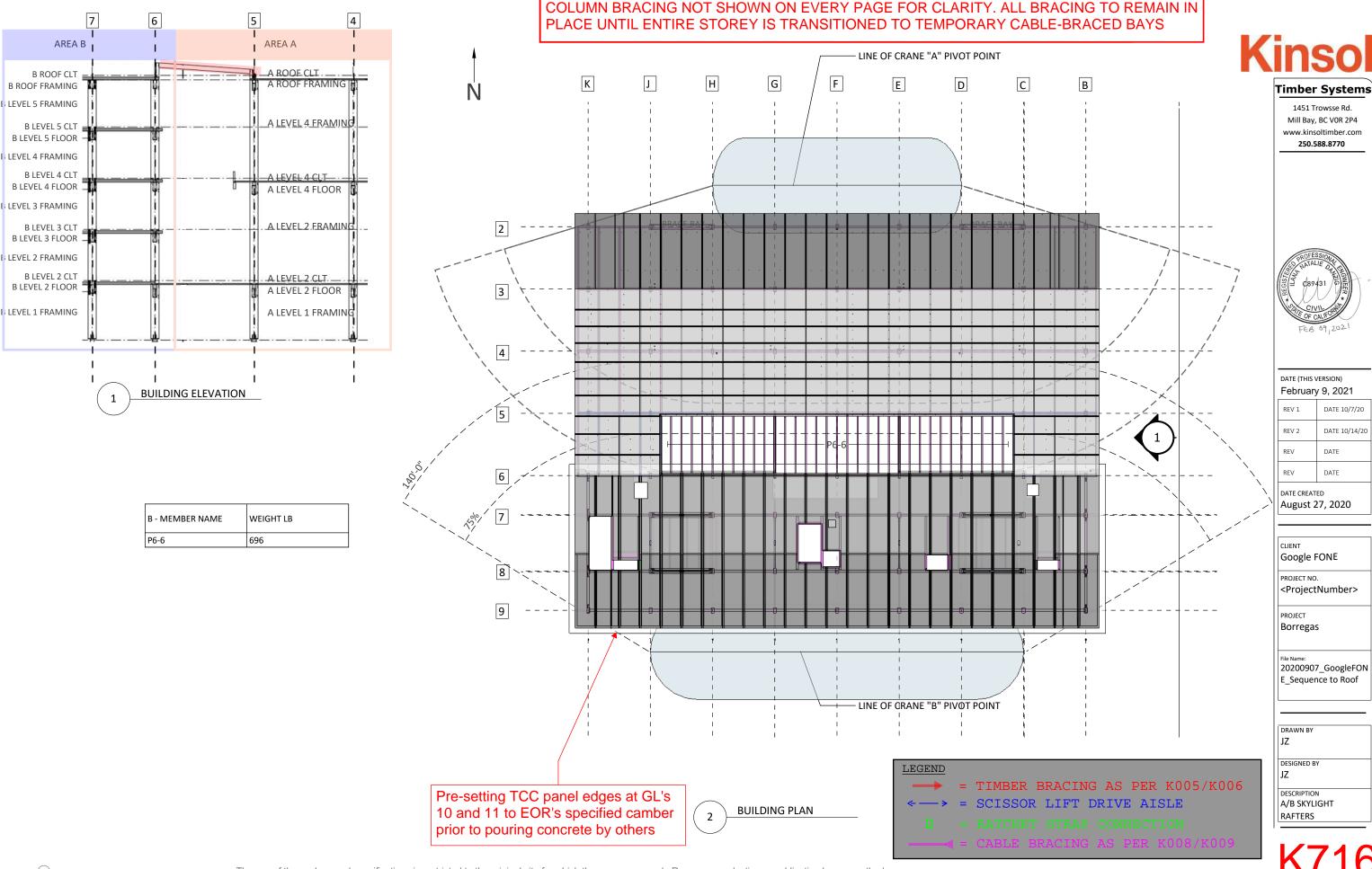
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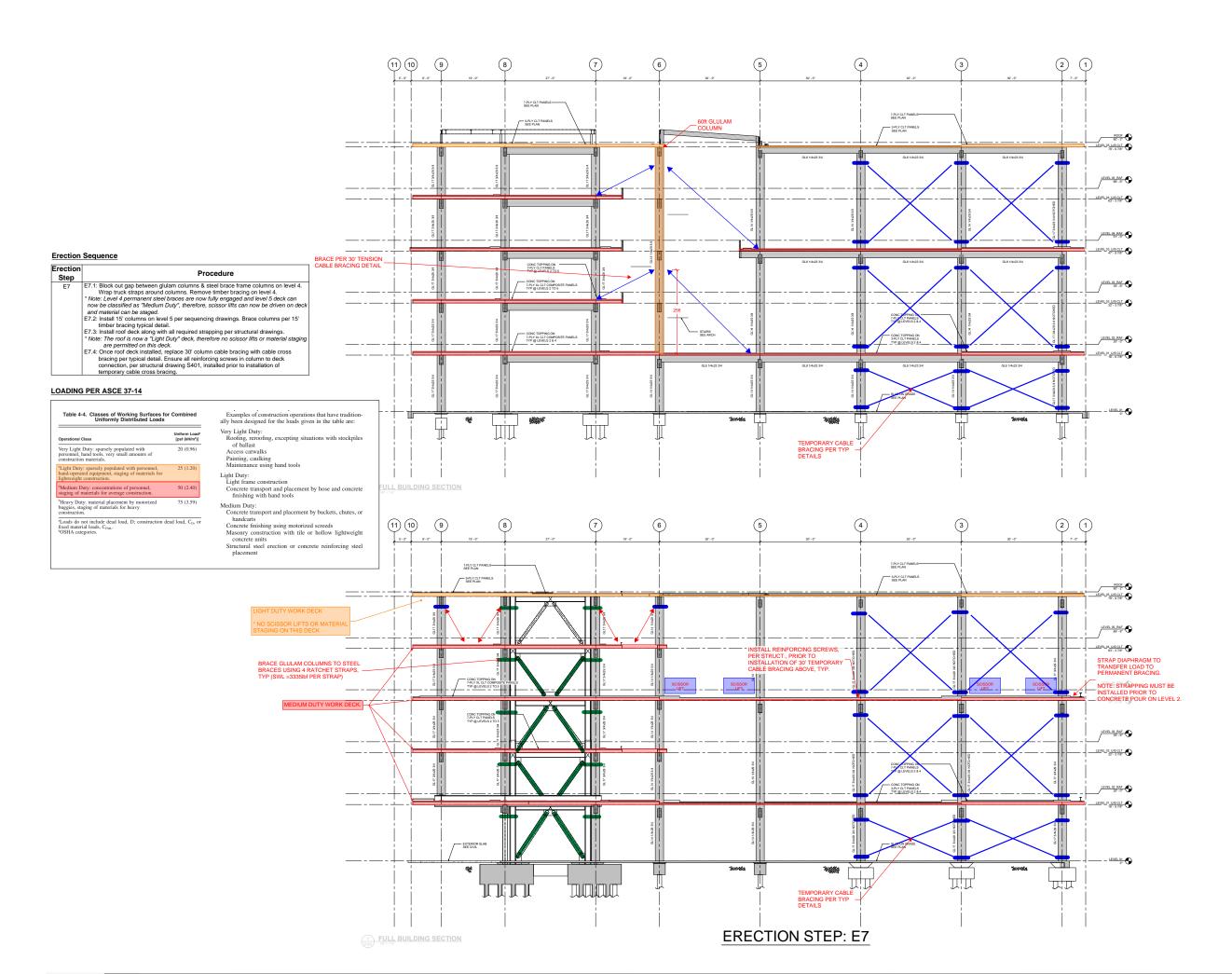
DATE

Pre-setting TCC panel edges at GL's

10 and 11 to EOR's specified camber

prior to pouring concrete by others







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2020-10-19	Issued for Construction	
2020-12-18	Issued for Construction	1
2021-02-09	Issued for Construction	2

Plot Date:

2020-10-19

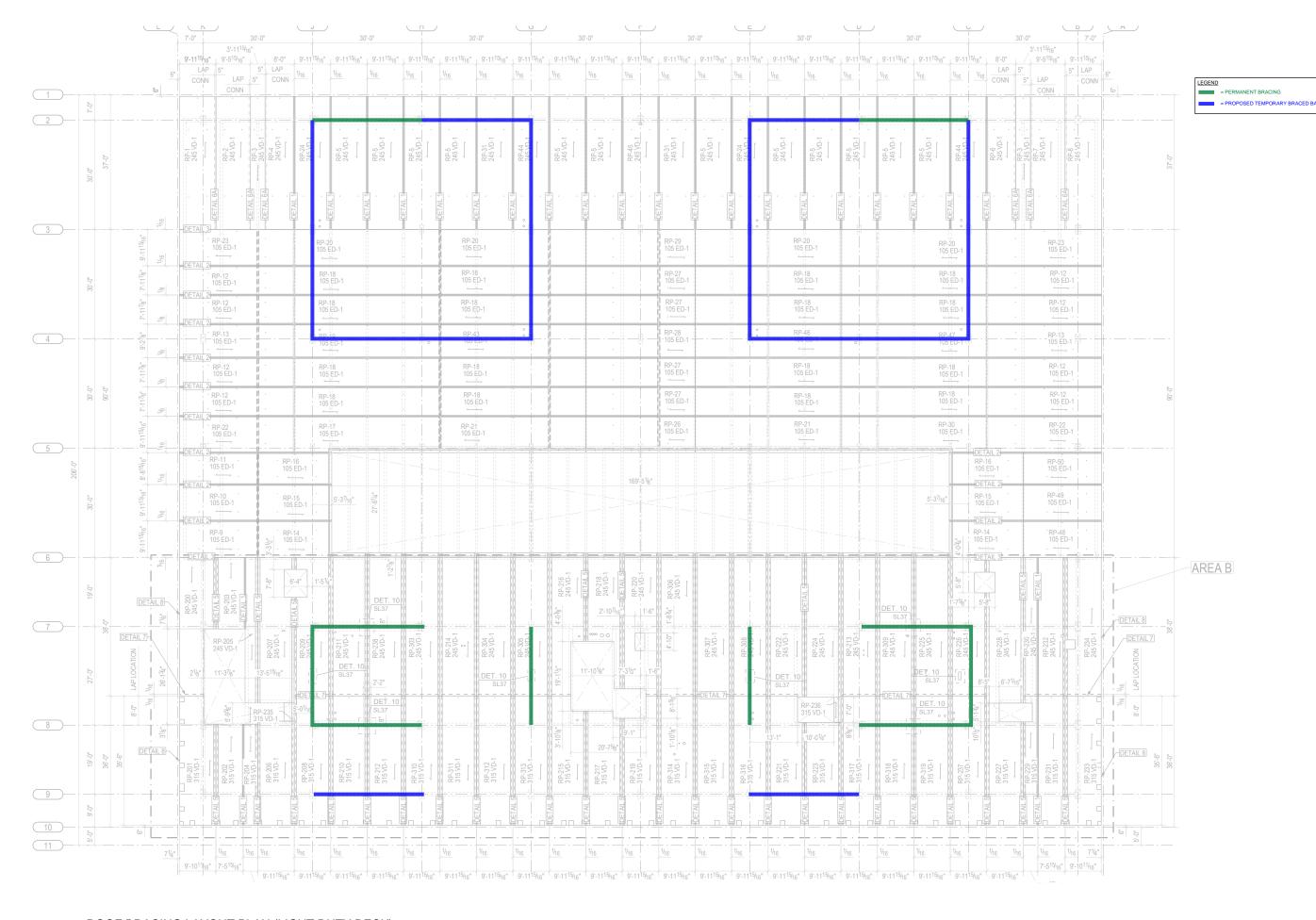
Drawing

Erection Step 7 Bracing

 Scale:
 Drawn:
 M

 NTS
 Checked:
 A

Drawing No.: Revision No.: -



ROOF BRACING LAYOUT PLAN (LIGHT DUTY DECK)

Scale: NTS

Kinsol

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2020-12-18	Issued for Construction	1
2021-02-09	Issued for Construction	2

Plot Date:

2020-10-19

Drawir

Roof Bracing Layout (Light Duty Deck)

Scale: NTS Drawn: Designed: Checked:

ving No.: Revision N

LOADING PER ASCE 37-14

Table 4-4. Classes of Working Surfaces for Combined Uniformly Distributed Loads

Operational Class	Uniform Load [psf (kN/m²)]
Very Light Duty: sparsely populated with personnel, hand tools, very small amounts of construction materials.	20 (0.96)
^b Light Duty: sparsely populated with personnel, hand-operated equipment, staging of materials for lightweight construction.	25 (1.20)
^b Medium Duty: concentrations of personnel, staging of materials for average construction.	50 (2.40)
^b Heavy Duty: material placement by motorized buggies, staging of materials for heavy construction.	75 (3.59)

 aLoads do not include dead load, D; construction dead load, $C_{\text{D}},$ or fixed material loads, $C_{\text{FML}}.$ bOSHA categories.

Examples of construction operations that have traditionally been designed for the loads given in the table are:

Very Light Duty: Roofing, reroofing, excepting situations with stockpiles of ballast

Access catwalks

Painting, caulking Maintenance using hand tools

Light Duty:

Light frame construction

Concrete transport and placement by hose and concrete finishing with hand tools

Medium Duty:

Concrete transport and placement by buckets, chutes, or handcarts

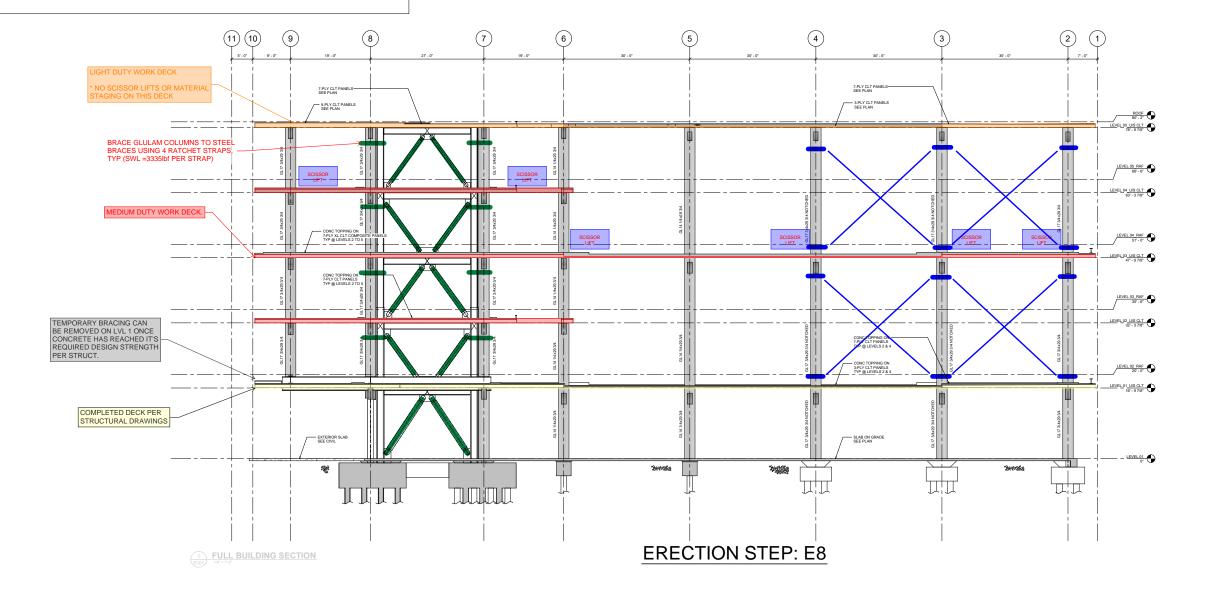
Concrete finishing using motorized screeds

Masonry construction with tile or hollow lightweight

concrete units Structural steel erection or concrete reinforcing steel placement

Erection Sequence

Erection Step	Procedure
E8	E8.1: Block out gap between glulam columns & steel brace frame columns on level 5. Wrap truck straps around columns. E8.2: Replace level 5 timber temporary bracing with cable bracing per typical detail. * Note: Scissor lifts or material staging are not permitted on the roof deck at any time. E8.3: Pour concrete topping on level 2 per structural drawings prior to removing temporary cable cross bracing on level 1.



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2020-10-19

Erection Step E8 Bracing

NTS

LOADING PER ASCE 37-14

Table 4-4. Classes of Working Surfaces for Combined

Operational Class	Uniform Load [psf (kN/m²)]
Very Light Duty: sparsely populated with personnel, hand tools, very small amounts of construction materials.	20 (0.96)
^b Light Duty: sparsely populated with personnel, hand-operated equipment, staging of materials for lightweight construction.	25 (1.20)
^b Medium Duty: concentrations of personnel, staging of materials for average construction.	50 (2.40)
bHeavy Duty: material placement by motorized buggies, staging of materials for heavy	75 (3.59)

 aLoads do not include dead load, D; construction dead load, $C_{\scriptscriptstyle D},$ or fixed material loads, $C_{\scriptscriptstyle FML}.$ bOSHA categories.

Examples of construction operations that have traditionally been designed for the loads given in the table are:

Very Light Duty: Roofing, reroofing, excepting situations with stockpiles of ballast

Access catwalks

Painting, caulking Maintenance using hand tools

Light Duty:

Light frame construction

Concrete transport and placement by hose and concrete finishing with hand tools

Medium Duty:

placement

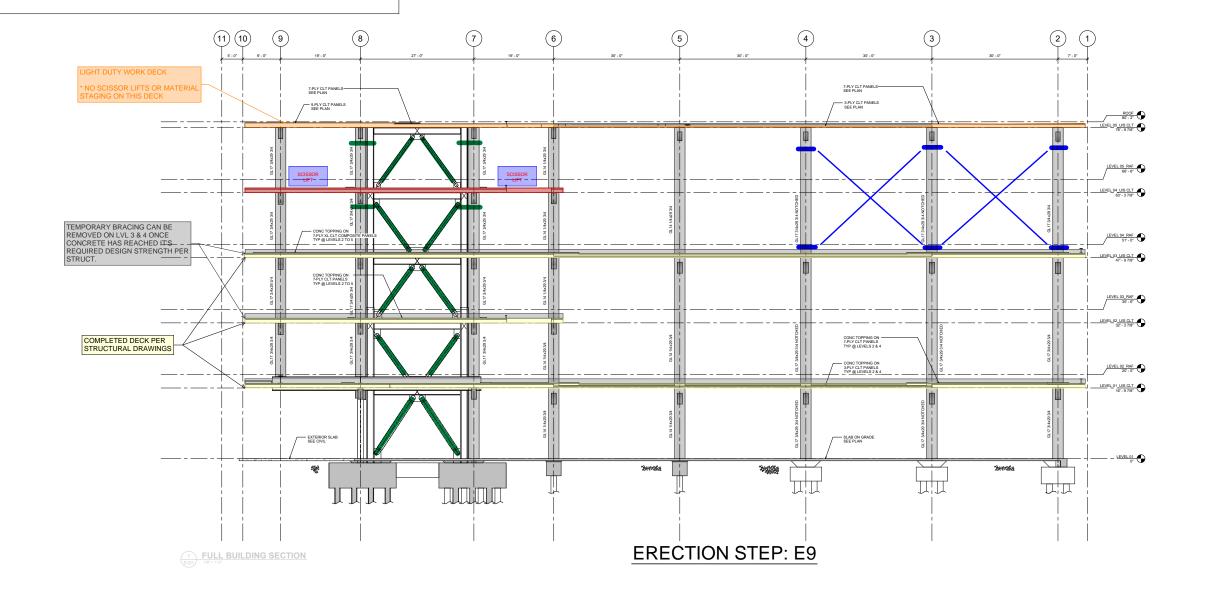
Concrete transport and placement by buckets, chutes, or handcarts

Concrete finishing using motorized screeds
Masonry construction with tile or hollow lightweight

concrete units Structural steel erection or concrete reinforcing steel

Erection Sequence

Erection Step	Procedure
E9	E9.1: Pour concrete topping on level 3 and 4 per structural drawings prior to removing temporary bracing on level 2.



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Erection Step E9 Bracing

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Drawing 1

Erection Step E10 Bracing

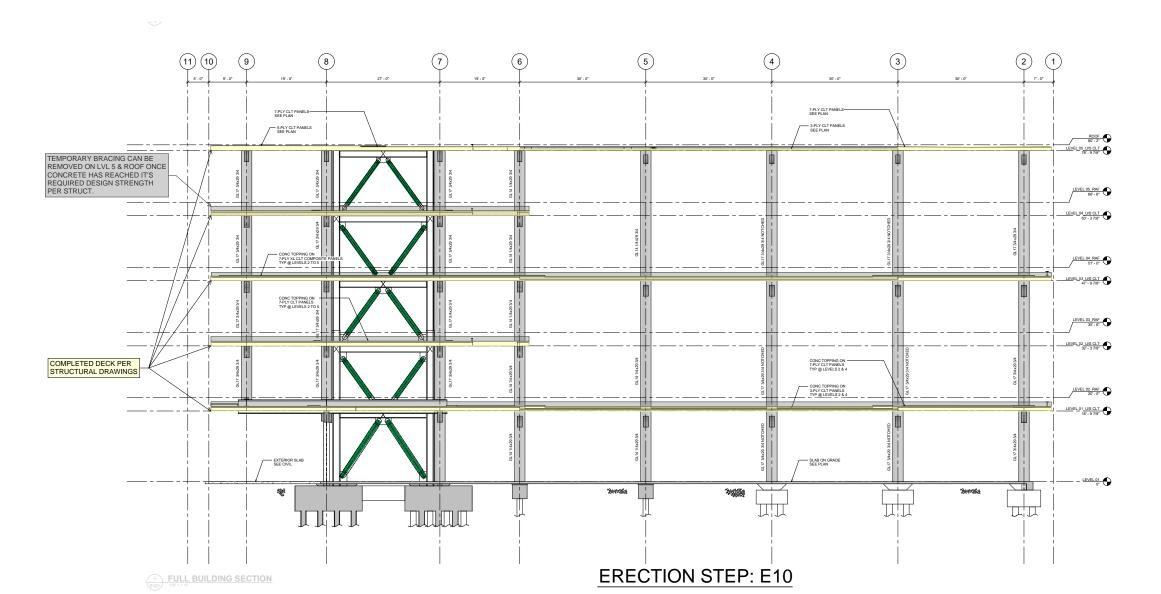
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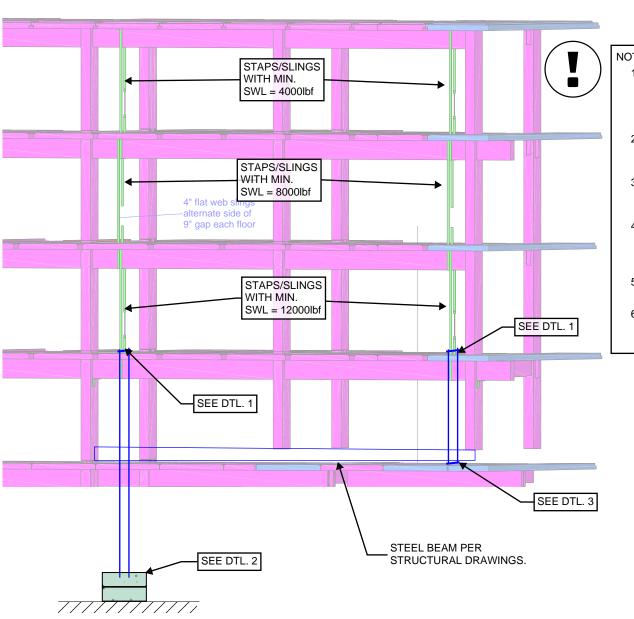
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Erection Sequence

Erection Step	Procedure	
E10	E10.1: Pour concrete topping on level 5 and roof per structural drawings prior to	1
	removing temporary bracing on level 4. Confirm with EOR prior to removal of	
	temporary bracing.	





- 1. THE APPROACH TO RESOLVING THE CAMBER INDUCED IN THE STEEL STRUCTURE IS TO CONFORM THE TIMBER FRAME TO THE INITIAL GEOMETRY OF THE STEEL TO ALLOW INSTALLATION TO PROCEED PRIOR TO THE CONCRETE BEING POURED.
- 2. THIS APPROACH ALLOWS THE CAMBER TO COME OUT AS MORE DEAD LOAD IS APPLIED TO THE
- 3. AS THE CAMBER COMES DOWN, ALL TEMPORARY LOADS SHOULD GO TO ZERO AND THE RIGGING CAN BE REMOVED.
- 4. SURVEY ELEVATIONS OF CAMBERED STEEL BEAMS TO CONFIRM TARGET CAMBERS ARE ACHIEVED. NOTIFY ASPECT OF DEVIATIONS > 1/16" ABOVE OR BELOW TARGET DEFLECTION
- 5. MONITOR FOR GAPS BETWEEN ELEMENTS DURING INSTALLATION AND NOTIFY ASPECT IF OBSERVED.
- 6. NOTIFY ASPECT WHEN TEMPORARY RIGGING GOES SLACK OR PRIOR TO REMOVAL OF TEMPORARY BRACING.

(2) 3/4" A307 RODS

STEEL BEAM

MÍN. SWL = 8000lbf EACH.

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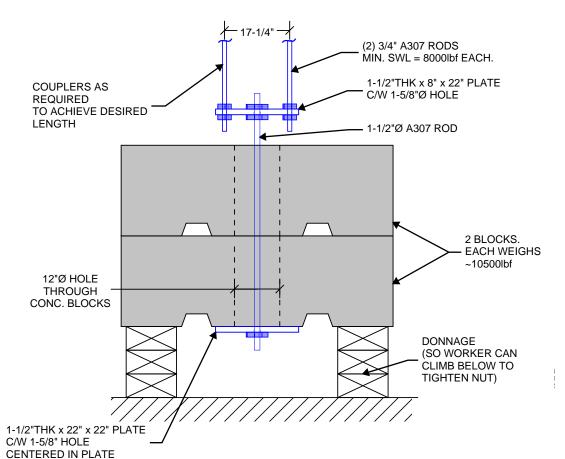
1265 Borregas Ave. Sunnyvale, CA 94089

Plot Date:

2021-02-12

Beam Camber Approach

– 17 1/4" – 4"x2"x1/4"HSS MIN. 22" LENGTH **GLULAM BEAM RODS TO GLULAM BEAM STRAP**



Scale: NTS

RODS TO STEEL BEAM STRAP AT GL. B-9 Scale: NTS

4"x2"x1/4"HSS

MIN. 22" LENGTH

CONCRETE BLOCK WEIGHT DETAIL AT GL. D-9

Scale: NTS